

22 June 2021

### 45 STATION ROAD, STAGE 1B HUAPAI

### **GEOTECHNICAL COMPLETION REPORT**

Cabra Developments Limited Ref: AKL2016\_0634AP Rev. 0

AKL2016_0634AP			
Date	Revision	Comments	
14 June 2021	А	Initial draft for internal review	
22 June 2021	0	Final issue to client	

	Name	Signature	Position
Prepared by	Jasmine Walden	A	Engineering Geologist
Reviewed by	Andrew Linton	At	Principal Geotechnical Engineer, CPEng
Authorised by	Richard Knowles	RT Knowles	Principal Geotechnical Engineer, CPEng



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#### 1. INTRODUCTION

In accordance with our instructions, this Geotechnical Completion Report has been prepared for Cabra Development Limited as part of the documentation to be submitted to Auckland Council following earthworks to form Stage 1B of the Huapai Triangle Sub Precinct A (45 Station Road, Huapai) Development. Construction of this residential subdivision has been undertaken in accordance with the Auckland Council Resource Consent number ENG 60068582 and SUB 60035794 and Engineering Approval letter dated 23 February 2017. Specific structures constructed during the civil works to create the subdivision include timber pole retaining walls and a keystone retaining wall.

This report contains our Suitability Statement, specific comments related to items raised in the Resource Consent, relevant test data and the Cato Bolam Consultants As-built plan set as provided in Appendix B.

This report covers the construction period December 2016 to March 2021 and is intended to be used for certification purposes for new lots (listed below) created from Lot 1 DP 540873 as follows:

- 27 new residential lots numbered 59 and 67 to 92;
  - Lots 67 and 68 are to be further subdivided into Lots 1 to 10 under Resource Consent number BUN 60372292;
  - Lots 86 and 87 are to be further subdivided into Lots 1 to 10 under Resource Consent number SUB 60374214;
  - Lot 92 is to be further subdivided into Lots 1 to 11 under Resource Consent number SUB 60368220.
- an extension to the existing Schoolside Road;
- 2 new avenues named Croatia Avenue and Podgora Avenue respectively;

This stage of the 45 Station Road Development is located off Station Road, Huapai. As can be seen from the As-built plans, 21 of the lots have been affected by filling as part of the earthworks operations to a maximum depth of approximately 6.1 metres.

#### 2. PROJECT BACKGROUND

The geotechnical investigations and design were undertaken by CMW Geosciences as presented in the following reports:

- Geotechnical Report for Huapai Development Stage 1, Huapai, prepared by CMW Geosciences referenced 2015\_1029AB Rev.0 dated 24 November 2014;
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced AKL2017\_0089AB Rev.0 dated 12 July 2017;
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced AKL2018-0195AA Rev.0 dated 12 December 2018;
- Stage 1A & 1B Retaining Wall Designs prepared by CMW Geosciences, referenced AKL2016\_0634AD Rev.0 dated 23 March 2017;
- Remaining Scope of Stage 1A & 1B Retaining Wall Designs prepared by CMW Geosciences, referenced AKL2016\_0634AH Rev.1 dated 19 March 2019;

#### 3. DESCRIPTION OF EARTHWORKS

Earthworks operations for the entire 45 Station Road Development began in early December 2016 with the installation of silt fences and other environmental controls, including formation of a sediment retention pond

in the southern end of Lot 92. Cut and fills for Stage 1B were conducted in conjunction with the other stages of this development and with neighbouring developments, all of which were under the observation of CMW Geosciences. The majority of this stage is in fill, with an area of cut required to form approximately 9 of the northern lots and a portion of Croatia Avenue, Schoolside Road and Podgora Avenue.

Bulk cut to fill was ongoing from December 2016 through to March 2017 within Stage 1B and the wider subdivision. A gully muck-out was completed in the southern portion of the stage, falling from west to east, and two subsoil drain coils were placed surrounded by scoria and Bidim cloth in the base of this muck-out. The subsoil drains from this undercut extend through from Stage 1A in the west, under retaining wall 11C and through Lot 69 towards the southeast.

Additional areas of soft and/or unsuitable materials were undercut below Lots 85 to 88 and the southern end of Lot 92. Subsoil drains were installed within the undercuts to capture groundwater springs and these drains were then connected into the main subsoil drain network.

Retaining wall construction for timber pole walls 13A, 13B and 13C began in May 2017. The retaining wall construction and earthworks within Stage 1B were then put on hold from the end of the 2017 earthworks season while the surrounding stages were developed.

In September 2020 the earthworks within Stage 1B was restarted with the muck out of the existing sediment retention pond in Lot 92 and filling beginning in early October 2020. The sediment retention pond was reduced in size through the earthworks operations, with a Decanting Earth Bund (DEB) left for sediment control during civil works operations. October also saw the construction of the remainder of the timber pole retaining walls within this stage.

Civil works including road construction also began in October 2020 and continued until completion of this stage in March 2021. A keystone retaining wall along the northern boundary of Lot 77 was constructed and completed in March 2021. The muck out and filling of the DEB within Lot 92 was the final area to be completed in Stage 1B and saw the completion of the subdivision in late March 2021.

### 4. GEOTECHNICAL QUALITY CONTROL

#### 4.1. Site Observations

During the earthworks, site visits were typically undertaken several times each week to assess compliance with NZS 4431 and specific design recommendations and specifications.

Site visits were carried out to observe and confirm compliance relating to:

- Adequate topsoil stripping;
- Fill areas prior to the placement of fill materials to ascertain that all mullock and soft inorganic subsoils had been removed;
- Installation and backfilling of subsoil drains;
- Excavation and backfilling of sewer and stormwater trenches;
- Construction of cantilever timber pole retaining walls including ground conditions, pile size, spacing and depth; and
- Construction of keystone walls including ground conditions, block placement, geogrid placement and hardfill backfill;
- Placement and compaction of engineered fills.

#### 4.2. Compaction Control

Compaction of engineered earth fills was controlled by undrained shear strength measured by handheld shear vane calibrated using the NZGS 2001 method and by air voids as defined by NZS4402.

The criteria for undrained shear strength were a minimum single value of 110 kPa and minimum average of any 10 consecutive tests of 140 kPa.

The criteria for air voids were a maximum single value of 12% and maximum average of any 10 consecutive tests of 10%.

Vane shear strength, water content and in situ density tests were carried out on all areas of the engineered filling to at least the frequency recommended by NZS 4431.

These tests showed on occasions that the contractor was struggling to achieve the required compaction standards with the prevailing site and soil conditions, but to the best of our knowledge, all areas of fill were re-worked as necessary. Subsequent testing confirmed compliance with the specification.

### 5. EVALUATION OF COMPLETED EARTHWORKS

#### 5.1. Natural Hazards

The appended as-built drawings depict the extents of a series of zones that contain limitations intended to ensure that future building and/ or earthworks on the lots is undertaken in a manner that does not lead to buildings being subject to any of the natural hazards described in Section 71(3) of the Building Act, i.e. erosion, falling debris, subsidence, slippage, and inundation. Consideration of the inundation hazard was outside the scope of CMW's brief and has been assessed by others. The applied zones on this subdivision are all **Specific Design Zones (retaining)** - intended to protect the subdivision retaining walls from overloading at the crest or undermining at the toe that could lead to instability.

Full descriptions of the restrictions associated with these zones are presented in the Suitability Statement (Appendix A). Additional information is also provided in some of the following sections.

#### 5.2. Natural Soils Geotechnical Assessment

Portions of lots within this stage of the subdivision are formed partially within natural soils, which are predominantly of alluvial origin.

Hand auger boreholes conducted in the centre of each lot as part of the post earthworks investigations did not encounter further lenses of organically stained soils but observed some lower strength natural subsoils.

While not expected, some discrete lenses of organic stained clay soils may still exist within the natural soils beneath lot areas. We do not consider that liquefaction and/or settlement due to discrete lenses of organically stained clay soils are a significant geotechnical risk for development or future residential dwellings built in general accordance with NZS 3604.

#### 5.3. Land Stability and Erosion Control

The subdivision scheme layout includes a series of batters to form the slope above some of the subdivision retaining walls. The batters are part of the residential lots with maximum gradients of 1(v) in 2.5(h) as depicted on the as-built drawings.

Design of the works to provide appropriate stability conditions that meet regulatory requirements for the land within this stage has led to the construction of deep subsoil drainage, cantilever pole retaining walls and keystone walls.

Building and landscape designers must ensure that all runoff from solid surfaces is directed into the stormwater system. It is also important that care is paid to the disposal of stormwater during construction so that concentrated discharges (e.g. from unconnected spouting) are not directed towards steep ground.

Depths of mulch and topsoil applied to sloping areas should be limited to less than 150mm to minimise the risks of saturation leading to localised slumping on batter faces. Wherever practical on such land, and particularly on steep batters, existing vegetation and grass cover should be well maintained. Any vegetation cleared beyond the immediate area of building platforms for temporary construction purposes should be replanted or replaced as soon as possible. The roots of an established vegetation cover can serve to bind the surface soils while the foliage can reduce rain infiltration and soil saturation, resulting in better resistance to erosion and shallow slumping.

#### 5.4. Retaining Walls

Cantilever timber pole and keystone retaining walls have been constructed in the locations shown on the appended Cato Bolam Limited As-built Plans. These walls reach a maximum height of approximately 1.5 metres and were designed by CMW Geosciences, and the construction was also observed by this consultancy. Copies of the Producer Statements - Construction Review (PS4) are provided in Appendix E.

The timber pole retaining walls along the western boundary of Lots 59 and 67 to 73 were constructed as part of the Stage 1A development under the Building Consent number BCO10092414-1-B and the associated PS4 is also provided in Appendix E.

Descriptions of the building and earthworks restrictions within the vicinity of these walls (Specific Design Zones – retaining) are contained in the Suitability Statement in Appendix A. Lots containing these zones include lots 59 and 67 to 91 inclusive.

#### 5.5. Fill Induced Settlement

On the basis of the elapsed time since fill has been placed across this stage of the subdivision, we consider that remaining post-construction settlements will be within code limits.

#### 5.6. Service Line Trenches

As part of the civil works, sanitary sewer and stormwater services were trenched throughout the development as shown on the appended Cato Bolam Consultants Stormwater and Wastewater As-built Plans.

As is normal on all subdivisions, building developments involving foundations within a 45 degree zone of influence from pipe inverts will require engineering input. The Auckland Council drawing referenced SW22 provided in Appendix B extracted from Chapter 4 of the Auckland Council Code of Practice for Land Development and Subdivision depicts their requirements for stormwater pipes. Details for water and wastewater pipes are available in the Watercare COP1 - General Requirements and Procedures. The majority of lots are known to have service trenches within the lots as shown on the appended stormwater and wastewater as-built plans, with the relevant zone of influence related to the stormwater pipes indicated on the Cato Bolam drawing titled Stormwater Zone of Influence As Built Plan Stage 1B, reference 42358-DR-SU-9304. The resulting restrictions are presented in the Suitability Statement below.

#### 5.7. Subsoil Drains

The appended Cato Bolam Consultants Cut/Fill As-built Plans show the positions of counterfort drains which were constructed in the natural ground during the earthworks operations. The drains were installed to help control groundwater levels and are either linked to the reticulated storm water system or extend to formed outlets within bush reserve areas. The ongoing operation of these drains is important to the overall stability conditions of the site.

Typical trench excavations extended into natural ground and lie between 2.8 and 6.1 metres below the finished surface. Accordingly, they are predominantly beyond the depths of anticipated foundations.

Descriptions of the restrictions related to these subsoil drains are contained in the appended Suitability Statement.

#### 5.8. Road Subgrades

Penetration resistance testing was carried out on the road subgrades during construction and the results of this testing were forwarded to Cato Bolam Consultants for pavement remedial design. Where soft ground with low equivalent CBR values was identified it was generally undercut and replaced with engineered fill, including placement of geogrid and geotextile cloth. All road subgrade areas were subsequently lime/ cement stabilised to achieve appropriate CBR values.

#### 5.9. Design of Shallow Foundations

#### 5.9.1. Bearing Capacity

Once bulk earthworks and top-soiling of the building platforms had been completed, our staff drilled hand auger boreholes on platforms in natural ground to determine representative finished ground conditions and hence evaluate likely foundation options for future building development. Our assessments of bearing capacity for the design of shallow foundations on each building platform are contained in the appended Suitability Statement.

At current subgrade levels Lots 59 and 67 to 92 inclusive have been assessed as having a geotechnical ultimate bearing capacity of 300kPa within the influence of conventional shallow residential building foundation loads.

If higher geotechnical ultimate bearing capacities are required, further specific site investigation and design of foundations should be carried out prior to Building Consent application.

#### 5.9.2. Seismic Site Subsoil Category

Based on the ground conditions observed during earthworks testing, combined with experience working in the surrounding area, the seismic site subsoil category for all lots within Stage 1B is assessed as being Class C (shallow soil site) in accordance with NZS 1170.5.

#### 5.9.3. Foundation Settlements

At the bearing pressures specified above, and subject to the design requirements for soil expansiveness provided below, differential settlement of shallow foundations for buildings designed in accordance with NZS 3604 (including the 600mm subfloor fill depth limit) should be within code limits.

#### 5.9.4. Soil Expansiveness Classification

Seasonal shrinking and swelling results in vertical surface ground movement which can cause significant cracking of floor slabs and walls. NZS 3604:2011<sup>1</sup> excludes from the definition of 'good ground', soils with a liquid limit of more than 50% and a linear shrinkage of more than 15% due to their potential to shrink and swell as a result of seasonal fluctuations in water content. For soils exceeding these limits, NZS 3604 has historically referenced AS 2870<sup>2</sup>. for foundation design advice. However, the November 2019 update of Acceptable Solution B1/AS1<sup>3</sup> provides amendments to NZS 3604 that define a method for testing and

<sup>&</sup>lt;sup>1</sup> Standards New Zealand (2011) Timber-framed buildings, NZS 3604:2011, NZ Standard

<sup>&</sup>lt;sup>2</sup> Standards Australia Limited (2011) Residential slabs and footings, AS 2870-2011, Australian Standard, NSW

<sup>&</sup>lt;sup>3</sup> Ministry of Business, Innovation and Employment (2019) Acceptable Solutions and Verification Methods for NZ Building Code Clause B1 Structure, B1/AS1, Amendment 19

classifying the soils and provides foundation designs for specific, simple house configurations across the range of expansive soil conditions.

Nevertheless, there is evidence<sup>4</sup> indicating that the use of the B1/AS1 method of assessment of expansiveness may be inaccurate.

8 sets of soil tests were carried out on samples taken from likely foundation level on lots within this stage of the development. Testing was carried out in accordance with NZS 4402, "Methods of Testing Soils for Civil Engineering Purposes" test 2.2 and 2.6 and were used in conjunction with visual-tactile assessment of the site soils and BRANZ Report SR120A<sup>5</sup> to determine expansive site Classes as defined in AS 2870, "Residential Slabs and Footings – Construction". All test results are appended.

The expansive soil hazard is addressed by a combination of appropriate foundation design, careful site preparation and diligent maintenance of plantings near the foundations.

#### Foundation Design

We have assessed the AS 2870 Site Class for lots on this stage of the development to be as detailed below:

- M (Medium) Lots 59, 67 to 76 inclusive and Lot 92,
- H1 (High) Lots 79 to 84 inclusive and Lots 88 to 91 inclusive,
- E (Extreme) Lots 77, 78 and 85 to 87 inclusive.

Details of foundation options for these Classes are contained in the appended Suitability Statement.

#### Site Preparation

There have been instances of concrete floors and/ or foundations that have been poured on dry, desiccated subgrades in summer months on expansive soils and have undergone heaving and cracking requiring extensive repairs or re-building once the soil moisture contents have returned to higher levels. In some instances, perimeter foundations have been appropriately treated but floor slabs have been poured on dry ground. Infiltration of moisture via pipe bedding has then occurred.

Foundation contractors need to be made aware of the extreme damage potentially caused by these circumstances and the need to maintain appropriate moisture contents in the footings <u>and</u> building platform subgrade between the time of excavation and the pouring of concrete.

Remedial actions that may be appropriate include platform protection with a hard fill layer, pouring of a blinding layer of concrete in footing bases and soaking of the building platform with sprinklers for an extended period.

#### Site Maintenance

Landowners must be mindful that either the <u>planting or removal</u> of high water demand plants where their roots may extend close to footings (i.e. within a lateral distance of 1.5 times the mature tree height) can cause settlement or heave damage.

#### 5.10. Topsoil Depths

Topsoil depths have been checked by the drilling of a borehole in the approximate centre of the building platform on each lot. The results are considered indicative for each lot, but may be subject to variations. Topsoil depths range between 100 and 300mm on these stages of the development.

<sup>&</sup>lt;sup>4</sup> Rogers, N., McDougall, N., Twose, G., Teal, J. & Smith, T. (2020) The Shrink Swell Test: A Critical Analysis, *NZ Geomechanics News*, Issue 99, pages 66-80.

<sup>&</sup>lt;sup>5</sup> Fraser Thomas Limited (2008) - Addendum Study Report (BRANZ SR120A), Soil Expansivity in the Auckland Region – Final Report

Site specific findings are contained in the appended Suitability Statement Summary (Appendix A). However, it is possible that further levelling works have been undertaken since our investigations and accordingly, we strongly recommend that lot purchasers complete their own checks of topsoil depths.

#### 6. CLOSURE

The appended Statement of Professional Opinion is provided to the Auckland Council and Cabra Developments Limited for their purposes alone on the express condition that it will not be relied upon by any other person. It is important that prospective purchasers satisfy themselves as to any specific conditions pertaining to their particular land interest.

Although regular site visits have been undertaken for observation, for providing guidance and instruction and for testing purposes, the geotechnical services scope did not include full time site presence. To this end, our appended Suitability Statement also relies on the Contractors' work practices and assumes that when we have not been present to observe the work, it has been completed to high standards and in accordance with the drawings, instructions and consent conditions provided to them.

Similarly it assumes that all as-built information and other details provided to the Client and/or CMW by other members of the project team are accurate and correct in all respects.

Appendix A: Statement of Professional Opinion as to the Suitability of Land for Building Development

#### STATEMENT OF PROFESSIONAL OPINION AS TO THE SUITABILITY OF LAND FOR BUILDING DEVELOPMENT

I, Richard Knowles, of CMW Geosciences (NZ) Limited, Auckland, hereby confirm that:

- As a Chartered Professional Engineer experienced in the field of geotechnical engineering, I am a Geoprofessional as defined in Section 1.2.2 of NZS 4404 and was retained by the Developer as the Geotechnical Engineer on Stage 1B of the Huapai Triangle Sub Precinct A (45 Station Road, Huapai) Development.
- 2. The extent of preliminary investigations carried out to date are described in:
- Geotechnical Report for Huapai Development Stage 1, Huapai, prepared by CMW Geosciences referenced 2015\_1029AB Rev.0 dated 24 November 2014;
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced AKL2017\_0089AB Rev.0 dated 12 July 2017;
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced AKL2018-0195AA Rev.0 dated 12 December 2018;

The conclusions and recommendations of these documents have been re-evaluated in the preparation of this report. The results of all tests carried out are also appended.

- 3. In my professional opinion, not to be construed as a guarantee, I consider that:
  - (a) The earth fills shown on the appended Cato Bolam Consultants Cut/Fill As-built Plans have been placed in compliance with NZS 4431, the Auckland Council Unitary Plans and related documents.
  - (b) Specific Design Zone (Retaining) areas have been applied on lots 59 and 67 to 91 inclusive for the protection of the function of the retaining walls. The retaining walls on, or adjacent to, this stage of the development were designed for the following limits:
    - Timber Pole Wall 9 and 10 (Lot 59) Maximum of 1(V):2.5(H) slope behind the wall, 5° tope slope and 12kPa surcharge;
    - Timber Pole Wall 11A (Lots 67 to 73) Maximum of 1(V):2.5(H) slope behind the wall, 5° tope slope and 12kPa surcharge;
    - Timber Pole Wall 11B (Lots 74 to 76) Maximum of 5° slope behind the wall, 5° toe slope and 12kPa surcharge;
    - Timber Pole Wall 11C (Lot 76) Maximum of 5° slope behind the wall, 5° toe slope and 12kPa surcharge;
    - Timber Pole Wall 13B (Lots 84 to 91) Maximum of 1(V):2.5(H) slope behind the wall, 5° toe slope and 12kPa surcharge;
    - Timber Pole Wall 13C (Lot 91) Maximum of 5° slope behind the wall, 5° toe slope and 12kPa surcharge;
    - Keystone Retaining Wall 12 (Lot 77) Maximum of 1(V):2.5(H) slope behind the wall, 0° toe slope and 12kPa surcharge;

No building construction <u>and</u> no earthworks (i.e. cut or fills) should take place within the Specific Design Zone areas that exceed these design limits on the walls unless endorsed by a Chartered Professional Engineer experienced in geomechanics and familiar with the contents of this report who has considered the stability implications of the earthworks and/ or building proposals on the retaining walls.

- (c) The function of the subsoil drains installed beneath lots 68, 69, 85 to 87 and 92 inclusive must not be impaired by any building development or landscaping works. Any bored or driven piles must be positioned to avoid damaging the draincoils. Where any subsoil drain is intercepted by building works, it must be reinstated under the direction of a Chartered Professional Engineer to ensure the integrity of the subsoil drainage system.
- (d) A geotechnical ultimate bearing capacity of 300kPa may be assumed for shallow foundation design on the building platforms of Lots 59 and 67 to 92 inclusive.

If for any reason higher geotechnical bearing capacities are required, further specific site investigation and design of foundations should be carried out prior to Building Consent application.

- (e) The expansive site Class for all lots in Stage 1B has been assessed as the following from AS2870:
  - M (Medium) Lots 59, 67 to 76 inclusive and Lot 92,
  - H1 (High) Lots 79 to 84 inclusive and Lots 88 to 91 inclusive,
  - E (Extreme) Lots 77, 78 and 85 to 87 inclusive.
- (f) The backfilling and compaction of the stormwater and sanitary sewer trenches on this subdivision has been carried out to appropriate standards having regard for the prevailing ground conditions and associated compaction induced pipe loadings.

However, no building development should take place within the 45 degree zone of influence of drain inverts unless endorsed by specific design and by construction inspections undertaken by a Chartered Professional Engineer experienced in geomechanics to ensure that lateral stability and differential settlement issues are addressed and that building loads are transferred beyond the influence of the pipe and trench backfill. A copy of drawing SW22 extracted from Chapter 4 of the Auckland Council Code of Practice for Land development and Subdivision this document is provided in Appendix B for clarification. Details for water and wastewater pipes are available in the Watercare COP1 - General Requirements and Procedures. Cato Bolam have also provided a plan within their As-Built set showing the general extent of the zone of influence of these pipes.

- (g) Subject to the geotechnical limitations, restrictions and recommendations contained in clauses 3(b), 3(c), 3(d), 3(e) and 3(f) above:
  - (i) The filled and natural ground is generally suitable for residential buildings constructed in accordance with NZS 3604 and the requirements of AS2870 for the appropriate expansive soil class.
  - (ii) Where shallow foundations are appropriate, design may be carried out in accordance with AS2870 (soil classifications for lots within this stage are detailed in clause (e) above) or alternately, a specific foundation and structural design may be undertaken by a Chartered Professional Engineer.
- 4. Road subgrades have been formed with appropriate regard for slope stability and settlement risks.

The following table summarises the conditions on each of each residential lot.

#### For and on behalf of CMW Geosciences

lef knowles

Richard Knowles Principal Geotechnical Engineer, CPEng.

Condition	Specific Design Zone (retaining)	Subsoil Drains Present	Geotechnical Ultimate Bearing Capacity (kPa)	AS2870 Expansive Class	Service Lines Restrictions	Indicative Topsoil Depth (mm)
GCR SOPO Clause	3(b)	3(c)	3(d)	3(e)	3(f)	
Lot number						
59	•		300	М	•	300
67	•		300	М	•	300
68	•	•	300	М	•	300
69	•	•	300	М	•	300
70	•		300	М	•	300
71	•		300	М	•	200
72	•		300	М	•	300
73	•		300	М	•	300
74	•		300	М	•	300
75	•		300	М	•	300
76	•		300	М	•	300
77	•		300	Е	•	200
78	•		300	Е	•	200
79	•		300	H1	•	300
80	•		300	H1	•	300
81	•		300	H1	•	250
82	•		300	H1	•	300
83	•		300	H1	•	200
84	•		300	H1	•	200

Condition	Specific Design Zone (retaining)	Subsoil Drains Present	Geotechnical Ultimate Bearing Capacity (kPa)	AS2870 Expansive Class	Service Lines Restrictions	Indicative Topsoil Depth (mm)
GCR SOPO Clause	3(b)	3(c)	3(d)	3(e)	3(f)	
85	•	•	300	Е	•	100
86	•	•	300	Е	•	300
87	•	•	300	Е	•	200
88	•		300	H1	•	300
89	•		300	H1	•	200
90	•		300	H1	•	300
91	•		300	H1	•	250
92		•	300	М		200

## **Appendix B: Drawings**

Title	Reference No.	Date	Revision
Cato Bolam Consultants Lot Number Reference Asbuilt Plans – Stage 1B	42358-DR-SU-0001	May 2021	0
Cato Bolam Consultants Final Contours and Retaining Walls Asbuilt Plans – Stage 1B (Sheets 1 to 3)	42358-DR-SU-9000 to 9002	May 2021	0
Cato Bolam Consultants Retaining Wall Specific Design Asbuilt Plans – Stage 1B (Sheets 1 to 2)	42358-DR-SU-9005 to 9006	May 2021	0
Cato Bolam Consultants Retaining Wall Cut/Fill Asbuilt Plan – Stage 1B (Sheets 1 to 3)	42358-DR-SU-9010 to 9012	May 2021	0
Cato Bolam Consultants Wastewater Asbuilt Plans – Stage 1B (Sheets 1 to 3)	42358-DR-SU-9200 to 9202	May 2021	0
Cato Bolam Consultants Stormwater Asbuilt Plans – Stage 1B (Sheets 1 to 3)	42358-DR-SU-9300 to 9302	May 2021	1
Cato Bolam Consultants Stormwater Connections Asbuilt Plan – Stage 1B	42358-DR-SU-9303	May 2021	0
Cato Bolam Consultants Stormwater Zone of Influence Asbuilt Plan – Stage 1B	42358-DR-SU-9304	May 2021	0
Auckland Council Stormwater Pipe and Manhole Construction Clearance Requirements	ACSD SW22	September 2013	1

# Cabra Developments Ltd - 45 Station Road, Huapai - Stage 1B

# For Completion



LOCATION DIAGRAM Scale 1:10,000

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an No	Rev	Plan Litle	Plan No	Rev	Plar
		General			Sto
9800	0	Lot Number Reference As Built Plan Stage 1B	9300	0	Stor
			9301	0	Stor
		Earthworks	9302	0	Stor
9000	0	Final Contours & Retaining Walls As Built Plan Stage	9303	0	Stor
		1B (Sheet 1 of 3)	9304	0	Stor
9001	0	Final Contours & Retaining Walls As Built Plan Stage 1B (Sheet 2 of 3)		C	
9002	0	Final Contours & Retaining Walls As Built Plan Stage			Wa
	-	1B (Sheet 3 of 3)	9400	1	Wat
9005	0	Retaining Wall Specific Design As Built Plan Stage 1B	9401	0	Wat
		(Sheet 1 of 2)	9402	0	Wat
9006	0	Retaining Wall Specific Design As Built Plan Stage 1B (Sheet 2 of 2)	9403	0	Wat
9010	0	Cut Fill As Built Plan Stage 1B (Sheet 1 of 3)			
9011	0	Cut Fill As Built Plan Stage 1B (Sheet 2 of 3)			
9012	0	Cut Fill As Built Plan Stage 1B (Sheet 3 of 3)			

### Roading

9100 9101 9102

9103

9200

9201

9202

0	Roading As Built Plan Stage 1B (Sheet 1 of 4)
0	Roading As Built Plan Stage 1B (Sheet 2 of 4)

- 0 Roading As Built Plan Stage 1B (Sheet 3 of 4)
- 0 Roading As Built Plan Stage 1B (Sheet 4 of 4)

#### Wastewater

1 Wastewater As Built Plan Stage 1B (Sheet 1 of 3)

- 0 Wastewater As Built Plan Stage 1B (Sheet 2 of 3)
- 0 Wastewater As Built Plan Stage 1B (Sheet 3 of 3)



PLANNERS | SURVEYORS | ENGINEERS ARCHITECTS | ENVIRONMENTAL

#### n Title

ormwater

rmwater As Built Plan Stage 1B (Sheet 1 of 3) rmwater As Built Plan Stage 1B (Sheet 2 of 3) rmwater As Built Plan Stage 1B (Sheet 3 of 3) rmwater Connections As Built Plan Stage 1B rmwater Zone of Influence As Built Plan Stage 1B

#### ater

ter Supply As Built Plan Stage 1B (Sheet 1 of 4) ter Supply As Built Plan Stage 1B (Sheet 2 of 4) ter Supply As Built Plan Stage 1B (Sheet 3 of 4) ter Supply As Built Plan Stage 1B (Sheet 4 of 4)

DATE	May 2021	ORIGINAL SIZE	A3
DRAWING NO.			

42358-DR-SU-0001-0















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45 Station Road, Huapai

As Built Plan - Stage 1B (Sheet 2 of 3)



	DAIL	ORIGINAL SCALL	OKION	
	16/03/21	1:500		A3
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DATE 24/01/2021	SURVEYED DESIGNED DRAWN DATE 16/03/21	ORIGINAL SCALE 1:500	NAM OPIE KM AA ORIGII	E ( S ( 1 NAL SI)	DATE 2021 09/2016 6/03/21 ZE A3
DATE 24/01/2021	SURVEYED DESIGNED DRAWN DATF	ORIGINAL SCALE	NAM Opie Km AA	E ( 5 ( 1 )	DATE 2021 09/2016 6/03/21 7F
DATE 24/01/2021	SURVEYED		NAM Opie	E S	DATE 2021
retaini	ng wall drainage has	been provided by Opi	es.		
2. Coord	tion regarding the loca	ation and dimentions a	associat	ed	
GENER 1. Levels	AL s are in terms of LINZ dinates are in terms of	Z Datum 1946.			
NOTE	S				
Pr Er	none : (09) 427 0072 nail : catobolam@cato	obolam.co.nz			
Na	ame: Tom Lemon				
Da	ate: 31/05/2021				
Si	gned:Regist	ered Professional Surve	eyor		
		uatum), anu are within ±	- 1011111.		
*	NZGD(2000), and are The levels (Z) are in t	e within ± 50mm. erms of the Auckland 19	946 (MS	L)	
l c the *	ertify that these As-Bui e works undertaken and The Coordinates (X	ilt Plans are an accurate d that: () are in terms of NZTM	e record	of	
					- 7
		(Refer to CMW Ge Geotechnical Con	eoscie Poscie	nces n Rei	Ltd port)
		Retaining Walls s	pecific	desi	gn
//-	//	Durapanel Titan S unless shown othe	afety l erwise	-enci	ing
38	3.5	Final Contours Mi	nor		
o	8 0	Einal Contours Mr	aior		
<del>- 49 - 49</del>	<del> </del>	Top of Bank			
		Stage Boundary			
$\square$	/	Private SW catchp	oit and	pipe	for
	1.5	Keystone Retainir with height of wall	ng Wal	ĺ	
	(1.5)	with height of wall	vvailS		
	<u>/ 13 DU</u>	Ilt Legend	Malla		
		<u>As Bu</u> (1.5)	As Built Legend         1.5       Timber Retaining with height of wall         (1.5)       Keystone Retaining with height of wall	As Built Legend         1.5       Timber Retaining Walls with height of wall         (1.5)       Keystone Retaining Wall with height of wall	As Built Legend         (1.5)       Timber Retaining Walls with height of wall         (1.5)       Keystone Retaining Wall with height of wall



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45 Station Road, Huapai

As Built Plan - Stage 1B (Sheet 2 of 2)



Ν		42	358-DR-SU-90	)06	0
		16/03/21 Drawing No.	1:500		A3 REVISION
		DATE	ORIGINAL SCALE	ORIGI	NAL SIZE
		DESIGNED		KM	09/201
NAME DAT TL 24/02/	TE 2021	SURVEYED		NAM Opie	E DATE S 2021
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/	retaini	ng wan urainage has	веен ргомаеа ву Ор	ICS.	
/ /	nformat	ion regarding the loca	ation and dimentions a	associat	ted
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/   ?	1. Level	AL s are in terms of LINZ	2 Datum 1946.		
/   .		\$			
]					
$\setminus$	Er	nail : catobolam@cato	bolam.co.nz		
	Pŕ	none : (09) 427 0072			
	Na	ame: Tom Lemon			
	Da	ate: 31/05/2021			
		Registe	ered Professional Surve	eyor	
	Si	gned:			
		LINZ datum (DOSLI d	patum), and are within :	E 10mm.	
	*	The levels (Z) are in to	$\pm$ within $\pm$ 50mm. erms of the Auckland 1	946 (MS	iL)
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	l c	ertify that these As-Bui	It Plans are an accurate d that:	e record	of
			Geotechnical Con	npletio	n Report)
			(Refer to CMW G	eoscie	nces Ltd
			Retaining Walls s	pecific ions to	design boundary
_		//	Durapanel Titan S	Safety	Fencing
	38	9.5	Final Contours Mi	nor	
-	3	8.0	Final Contours Ma	ajor	
			Bottom of Bank		
	- 11 - 11	<del>- 11 - 11 - 11 -</del>	Top of Bank		
			Stage Boundary		
			wall drainage		
		/	Private SW catch	pit and	pipe for
		(1.5)	with height of wal	l	
,,   _			Keystone Retainir	ng Wal	I
◀   -		(1.5)	with height of wal	vvalis I	
				Wells.	
		As Ru	ilt I egend		
		Retai	ning Wall		
		Retai	ning Wall		







Cabra Developments Ltd 45 Station Road, Huapai Cut Fill As Built Plan Stage 1B (Sheet 1 of 3)



No. REVISION (DESCRIPTIONS)
O Issued for completion

		1
/	Cut / Fill	Legend
	<u> </u>	0 Contour
		Cut Contours & Area
	0.5	Fill Contours & Area
		Extent of Stage
	<u> </u>	Subsoil Drainage (150Ø) with depth Existing Subsoil Drainage
		Timber Retaining Wall
		Keystone Retaining Wall
		Existing Retaining Wall
	I certify that these As-Built F the works undertaken and t * The Coordinates (X, Y) NZGD(2000), and are w * The levels (Z) are in terr LINZ datum (DOSLI dat Signed:	Plans are an accurate record of hat: are in terms of NZTM on <i>ithin</i> ± 50mm. ns of the Auckland 1946 (MSL) tum), and are within ± 10mm. de Professional Surveyor
7		
	ENG60068582	2 / SUB60035794
NAME DATE TL 24/05/2021	SURVEYED DESIGNED DRAWN DATE 16/03/21	NAME         DATE           HB         01/03/21           KM         09/2016           AA         16/03/21           NAL SCALE         ORIGINAL SIZE           1:1000         A3
ON	DRAWING NO.	
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Cabra Developments Ltd 45 Station Road, Huapai Cut Fill As Built Plan Stage 1B (Sheet 2 of 3)



	Cut / Fill L	_egend
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		Extent of Stage
	<u> </u>	Subsoil Drainage (150Ø) with depth Existing Subsoil Drainage
		Timber Retaining Wall
		Keystone Retaining Wall
		Existing Retaining Wall
· · · · ■■■ D N P E	certify that these As-Built Plans are an ie works undertaken and that: * The Coordinates (X, Y) are in terms NZGD(2000), and are within ± 50m * The levels (Z) are in terms of the Au LINZ datum (DOSLI datum), and an igned: Registered Profession ate: 31/05/2021 ame: 	a accurate record of of NZTM on m. ickland 1946 (MSL) e within ± 10mm. nal Surveyor



ENG60068582 / SUB60035794

ION		DRAWING NO.			REVISION
		16/03/21	1:500		A3
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		DRAWN		AA	16/03/21
		DESIGNED		KM	09/2016
TL 2	24/05/2021	SURVEYED		HB	01/03/21
NAME	DATE			NAME	DATE







Cabra Developments Ltd 45 Station Road, Huapai

Cut Fill As Built Plan Stage 1B (Sheet 3 of 3)



· · · ·					
0.0		Cut / Fill L	_egend		
/ /			0 Contour		
			Cut Contou	rs & Area	
		0.5	Fill Contour	s & Area	
F			Extent of St	tage	
1		<u> </u>	Subsoil Dra with depth Existing Su	iinage (15) bsoil Drair	0Ø) nage
			Timber Ret	aining Wa	I
1.			Keystone R	tetaining V	Vall
20			Existing Re	taining Wa	all
490 M					
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		I certify that these As-Built Plan the works undertaken and that: * The Coordinates (X, Y) are NZGD(2000), and are withi * The levels (Z) are in terms of LINZ datum (DOSLI datum) Signed: Registered F	is are an accu in terms of NZ n ± 50mm. of the Aucklan ), and are with 	urate record ZTM on Id 1946 (Ms inin ± 10mm urveyor	l of SL) 
		Date: 31/05/2021			
		Name:Tom Lemon			
		Phone : (09) 427 0072 Email : catobolam@catobolan	n.co.nz		
		ENG60068582 / SUB6	0035794		
NAME	DATE			NAME	DATE
TL	24/05/2021	SURVEYED DESIGNED		HB KM	01/03/21 09/2016

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16/03/21	1:500		A3
DATE	ORIGINAL SCALE	ORIGIN	IAL SIZE
DRAWN		AA	16/03/21
DESIGNED		KM	09/2016
SURVEYED		HB	01/03/21







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Cabra Developments Ltd 45 Station Road, Huapai

Wastewater As Built Plan Stage 1B (Sheet 1 of 3)





I certify that these As-Built Plans are an accurate record of the works undertaken and that:

- \* The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within ± 50mm.
- \* The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.

Signed:	Ø	
	Registered Professional Surveyor	

- 31/05/2021 Date<sup>.</sup>
- Name: Tom Lemon

S

Phone : (09) 427 0072 Email: catobolam@catobolam.co.nz

#### NOTES

#### GENERAL

- Levels are in terms of LINZ Datum 1946.
- Coordinates are in terms of NZTM.
- All infrastructure is public unless otherwise shown.

#### WASTEWATER RETICULATION

- Cover for wastewater reticulation. Mains under grass berms and footpaths 600mm Mains under road carriageway 900mm Wastewatermain 75, 63, 50 & 400D PE100 PN16
- Stainless Steel bolts and nuts used for flanged
- connections.
- Metallic Detector Tape provided above all wastewatermains and ridermains

ENG60068582 / SUB60035794

ΩN		DRAWING NO.	מפה ווס מת הי	<b>n</b>	REVISION
		17/02/2021	1:1000		A3
		DATE	ORIGINAL SCALE	ORIGIN	AL SIZE
		DRAWN		GH	17/02/2021
TL	24/05/2021	DESIGNED		KM	11/2016
DL	25/02/2021	SURVEYED		OPIES	03/2021
NAME	DATE			NAME	DATE



Sianed:	
5	Registered Professional Surveyor
Date:	31/05/2021
Name:	Tom Lemon
Dhana	(00) 407 0070



Legend -	- Wastewater	· As Built
Existing	New - Cor Stage 1B	nstructed under
90 — <u>110</u> —	PWC Sewer	40
M	Isolation Valve	M
	Reducer	
FP	Flushing Pit	FP
	Boundary Kit	ВК

Signed:	Registered Professional Surveyor
Date:	31/05/2021
Name:	Tom Lemon



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Stage 1B (Sheet 1 of 3)







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Stage 1B (Sheet 2 of 3)



ON		DRAWING NO.	01	REVISION	
		18/05/2021	1:500		A3
		DATE	ORIGINAL SCALE	ORIGINA	AL SIZE
		DRAWN		KJM	18/05/2021
		DESIGNED		KM	09/2016
TL	24/05/2021	SURVEYED		HB	01/03/21
NAME	DATE			NAME	DATE



4	Drainage As Built Le	gend	
	Existing	Nev	v -As
	- Stormwater (Public)	Con	structed
		۲	— — —
			······
3	s Rock Rip Rap		<u>s —</u>
368220	Rain Garden	c	
T — — — — —		[	<u></u> 1~
	Syphoned catchpit with 100Ø uPVC pipe	<b>—</b>	P SW
Lot 1	Lot connections 100Ø uPVC unless otherwise shown	_	+
e 🕴	Extent of Works		
RJ Class 4 8% Rock Rip-Rap	NOTES GENERAL 1. Levels are in terms of LINZ Datum 1946. 2. Coordinates are in terms of NZTM. 3. All infrastructure is public unless otherwis STORMWATER 1. All pipes are Class 4 reinforced concrete jointed (RCRRJ), unless otherwise show	se show rubber i n.	n. ring
all Outlet UU.1	<ol> <li>All manholes are 1050mmØ concrete fla riser, unless otherwise shown</li> </ol>	nge bas	e and
1 1 00 m5	<ol> <li>Bedding is H2 type unless otherwise stat</li> </ol>	ed.	
8.36 mN	4. All catchpits are 675mm x 450mm cycle	friendly	grate
	<ol> <li>For house connection boundary offsets, s</li> <li>House connections 100Ø uPVC SN16</li> <li>All Private retaining wall outlets are 100u unless otherwise shown</li> </ol>	see She	et 9303. 116
102			
	I certify that these As-Built Plans are an accurate the works undertaken and that: * The Coordinates (X, Y) are in terms of NZTM ( NZGD(2000), and are within ± 50mm. * The levels (Z) are in terms of the Auckland 19 LINZ datum (DOSLI datum), and are within ± Signed: Registered Professional Survey 31/05/2021	record of on 46 (MSL) 10mm. vor	
	Date:		
-	Name: Tom Lemon		
ļ	Phone : (09) 427 0072 Email : catobolam@catobolam.co.nz		
	ENG60068582 / SUB60035794		
NAME DATE	_	NAME	DATE
TL 24/05/202	1 SURVEYED DESIGNED	НВ	01/03/21
	DRAWN	KJM	18/05/202
	DATE ORIGINAL SCALE	ORIGIN	IAL SIZE
		<u> </u>	
	DIAMINU NU.		<b>VEADOIN</b>

I certify that these As-Built Plans are an accurate record of

- the works undertaken and that:
- $^{*}$  The Coordinates (X, Y) are in terms of NZTM on
- NZGD(2000), and are within ± 50mm. \* The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.
- Signed

Registered Professional Surveyor

31/05/2021 Date:

Name: Tom Lemon

Phone : (09) 427 0072 Email: catobolam@catobolam.co.nz

#### NOTES

GENERAL

- Levels are in terms of LINZ Datum 1946. 1
- Coordinates are in terms of NZTM. 2.
- 3. All infrastructure is public unless otherwise shown. STORMWATER
- 1. All pipes are Class 4 reinforced concrete rubber ring jointed (RCRRJ), unless otherwise shown.
- All manholes are 1050mmØ concrete flange base and 2. riser, unless otherwise shown.
- Bedding is H2 type unless otherwise stated. 3
- All catchpits are 675mm x 450mm cycle friendly grate 4 lids semi recessed unless otherwise shown.
- House connections 100Ø uPVC SN16 5.
- All Private retaining wall outlets are 100uPVC SN16 6. unless otherwise shown.









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45 Station Road,



I certify that these As-Built Plans are an accurate record of the works undertaken and that:  $^{\ast}\,$  The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within ± 50mm. \* The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm. \_\_\_\_\_ s Signed Registered Professional Surveyor 8 31/05/2021 Date: Name: Tom Lemon Phone : (09) 427 0072 Email: catobolam@catobolam.co.nz NOTES GENERAL Levels are in terms of LINZ Datum 1946. 1 Coordinates are in terms of NZTM. 2. 3. All infrastructure is public unless otherwise shown. STORMWATER 1. All pipes are Class 4 reinforced concrete rubber ring jointed (RCRRJ), unless otherwise shown. All manholes are 1050mmØ concrete flange base and 2. riser, unless otherwise shown. Bedding is H2 type unless otherwise stated. 3 All catchpits are 675mm x 450mm cycle friendly grate 4

- lids semi recessed unless otherwise shown. For house connection boundary offsets, see Sheet 9303. 5.
- House connections 100Ø uPVC SN16 6.
- All Private retaining wall outlets are 100uPVC SN16 7. unless otherwise shown.



### Zone of Influence Legend

Pipeline 45° zone of influence is from 500mm below invert of Stormwater Pipes.







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Cabra Developments Ltd 45 Station Road,





### "BUILD CLOSE" NOTES:

- 1. Specific approval is required from Auckland Council if building adjacent to pipes, larger than 375mm internal diameter, or greater than 3.0m of depth.
- 2. Building to be outside all overland flow paths and floodplains.
- 3. Pile constructed to a depth of 1.0m below influence line.
- 4. Outside zone of influence, normal foundation requirements apply.

### "BUILD OVER" NOTES:

- 1. Applies to stormwater pipes 375mm nominal diameter or less.
- 2. Bridging over pipes larger than 375mm nominal diameter is NOT allowed under any circumstances.
- 3. Pile constructed to a depth of 1.0m below influence line.
- 4. Outside zone of influence, normal foundation requirements apply.
- 5. Bridging is NOT allowed over pipes where clear vertical seperation distance from top of pipe to underside of bridging beam is less than 1.5m



STORMWATER ENGINEERING STANDARD DETAILS ISSUE/REVISION: 1

DATE: 30 September 2013 CAD FILE: AC-STD-SW22

### $C \odot U \mathbb{N} C \square L$ AUGKLAND.

STORMWATER PIPE AND MANHOLE CONSTRUCTION CLEARANCE REQUIREMENTS MANHOLES NEAR BUILDINGS AND BUILDING CLOSE OR OVER PIPES

1. The information on this page is intended to show examples of typical scenarios and should be used for general guidance purposes only. Significant variations on a site by site basis are to be expected and it is in no way implied that meeting any of the above will guarantee approval. 2. Requirements for foundation design etc. apply to both sides of pipe. 3. No driven piles are permitted within 10m of brick Stormwater Structures, or within 5m of all other Stormwater Structures. 4. Specific approval is required from Auckland Council for driven piles in partially drilled holes, within the 5m/10m zone. 5. Piles that may be required to resist horizontal forces will require specific design. 6. Pile/Footing location point must be below 45° "Zone of Influence". 7. All Manholes shall have 24 hours unobstructed access. 8. Manholes in basements, or where sufficient clearance is unavailable, are not permitted. 9. All pipe buildovers will require approval by Auckland Council. 10. Refer to section 4.3.21 for pipe buildover 5m MIN. ADJACENT CLEARANCE BUILDING 3.2m MIN. 1.0m MIN. MANHOLE CONSTRUCTION CLEARANCE ORIGINAL SCALE: AS<u>NOTED</u> ENVIRONMENTAL-SW DRAWING No. REV ACSD Auckland OF Council SW22

## Appendix C: Laboratory Test Data



#### DETERMINATION OF THE WATER CONTENT, LIQUID LIMIT & LINEAR SHRINKAGE TEST METHOD NZS 4402 : 1986 TEST 2.1, 2.2 & 2.6

Project Name :	45 Station Road, Huapai					
Client : Address : Attention :	CMW Geosciences Ltd PO Box 300206 Albany, Auckland 0754 Jasmine Walden	Project No : Page : Date of Orde Sample Meth Sample Date Sampled By	er : nod : e :	20 0322 03 1 of 1 25.11.20 Hand auger 25.11.20		
Test Details :	Test performed on		Whole Sa			
	History :		Natural	mpie		
Sample No.	Location	Depth (m)	Liquid Limit	Linear Shrinkage	Natural Water Content (%)	
915L	Lot 67	0.4 to 0.8	91	17	39.1	
916L	Lot 71	0.4 to 0.9	78	18	36.9	
917L	Lot 74	0.4 to 0.10	76	18	34.9	
918L	Lot 77	0.4 to 0.11	132	26	54.4	
919L	Lot 82	0.4 to 0.12	101	16	39.9	
920L	Lot 86	0.4 to 0.13	123	28	35.4	
921L	Lot 90	0.4 to 0.14	79	20	42.7	
922L	Lot 92	0.4 to 0.15	62	15	33.6	

Comments :

 Tested By:
 HC
 Date :
 27.11.20 to 04.12.20

 Calculated By :
 ZH
 Date :
 09.12.20

 Checked By :
 ZH
 Date :
 10.12.20

## **Appendix D: Field Test Data**

	ton Woodward	LF11 Rev 4 Soil	Field Density NDN	/l Dire	ect Tr	ansn	nissio	on w	ith VSS	Report		Auckland Labo CMW Geoscie Building C, 9 F PO Box 30020 Phone: +64 (0	pratory Inces (NZ) Lim Hermark Drive 6, Albany, Aud 9) 4144 632	iited e, Rosedale, NZ ckland, NZ 0752	0632 2			
Project:		45 Station Road, Huapai										Test Method	ls:		Notes:			
Project No:		AKL2016_0634										NZS 4402.2.	1:1986		Solid Densi	tv:		Assumed
Location:		Huapai										NZS 4407.4.2	2.2:2015		Testing Loc	ations Select	ed By:	CMW Field Staff
Report No:		AKL2016_0634LAA Rev.0										NZGS:Augus	t 2001		, 60 tin B 20 0			
Report Date:		25/01/2017									-							
Client:		Cabra Developments Limited												Tes	ts indicated as	s	Measurement	s marked * are not accredite
Client Addres	5:												<b>0</b>	the	scope of the	e outside	and are outsid	e the scope of the laboratorie
Client Referer	ice:											ACCR	EDITED LABO	RATORY labo	oratory's accre	ditation		accreditation
				lr	1-situ Va	ne Shear	Strength	5				Field and	laboratory T	esting Data				
Date Sampled	Sample No.	Test Location	Soil Description	Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m³)	Gauge Dry Density (t/m³)	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth	Oven Water Content (%)	Solid Density (t/m³) *	Oven Dry Density (t/m³)	Calculated Air Voids (%) *	Comments
7/12/2016	N1	Lot 97	CLAY	189	189	>189	>189	>189	1.8543	1.3865	33.7	1.76	300	29.3	2.7	1.44	4.9	
	N2	Lot 99	CLAY	>189	>189	>189	>189	>189	1.8482	1.3592	36.0	0.65	300	37.4	2.7	1.34	-0.1	
14/12/2016	N3	Refer to site plan	CLAY	UTP	UTP	UTP	UTP	UTP	1.7574	1.3213	33.0	7.35	300	29.6	2.7	1.36	9.6	
	N4	Refer to site plan	CLAY	173	178	189	189	182	1.8379	1.3691	34.2	2.31	300	35.0	2.7	1.36	1.9	
16/12/2016	N5	Lot 93	CLAY	175	173	189	189	182	1.7953	1.2722	41.1	0.45	250	31.0	2.7	1.38	6.8	
	N6	Lot 97	CLAY	UTP	UTP	UTP	UTP	UTP	1.7946	1.3099	37.0	2.91	300	33.4	2.7	1.34	5.2	
	N7	Refer to site plan	CLAY	>189	>189	UTP	UTP	>189	1.8157	1.3169	37.9	1.24	250	30.8	2.7	1.38	5.9	
	N8	Lot 86	CLAY	UTP	UTP	UTP	UTP	UTP	1.8583	1.3689	35.7	0.25	300	38.6	2.7	1.34	-1.4	
20/12/2016	N9	Lot 93	CLAY	UTP	UTP	>189	>189	>189	1.8210	1.3141	38.6	0.52	300	34.9	2.7	1.34	2.9	
	N10	Lot 95	CLAY	UTP	UTP	>189	>189	>189	1.8331	1.3237	38.5	-0.08	300	33.5	2.7	1.38	3.2	
22/12/2016	N11	Road	CLAY	189	186	183	159	179	1.7840	1.3012	37.7	2.30	300	36.3	2.7	1.30	4.0	
30/12/2016	N12	Lot 44	Silty CLAY	>194	>194	193	141	>181	1.7770	1.2843	38.4	3.05	300	48.3	2.7	1.20	-2.2	Failed
	N13	Lot 45	Clayey SILT	UTP	151	158	148	>162	1.7915	1.3219	35.5	3.98	300	38.7	2.7	1.30	2.2	
	N14	Lot 47	Silty CLAY	>194	>194	>194	>194	>194	1.8021	1.3070	37.9	1.97	300	39.5	2.7	1.30	1.1	
	N15	Lot 46	Silty CLAY	>194	>194	>194	>194	>194	1.7969	1.3130	36.8	2.88	300	40.3	2.7	1.28	1.0	
4/01/2017	N16	Lot 62	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8617	1.3590	37.0	-0.72	300	31.6	2.7	1.42	2.9	
	N17	Lot 63	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8246	1.3219	38.0	0.66	300	36.7	2.7	1.34	1.6	
	N18	Lot 44	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8530	1.3908	33.2	2.17	300	37.4	2.7	1.34	-0.4	Retest of N12
5/01/2017	N19	Lot 64	Silty CLAY	188	151	154	157	163	1.7956	1.2963	38.5	1.95	300	54.4	2.7	1.16	-6.3	Failed
	N20	Road	Silty CLAY	>194	>194	>194	148	>182	1.7699	1.2558	40.9	1.96	300	28.7	2.7	1.38	9.6	
6/01/2017	N21	Lot 44	CLAY	UTP	UTP	UTP	UTP	UTP	1.8059	1.3199	36.8	2.44	300	44.8	2.7	1.24	-2.0	
	N22	Lot 42	CLAY	>194	>194	191	>194	>193	1.8125	1.3097	38.7	1.09	300	32.9	2.7	1.36	4.6	
	N23	Lot 64	CLAY	UTP	UTP	UTP	UTP	UTP	1.8148	1.3314	36.3	2.25	300	43.2	2.7	1.26	-1.7	Retest of N19
10/01/2017	N24	Lot 43	CLAY	189	189	>189	>189	>189	1.7905	1.3118	36.5	3.44	300	36.3	2.7	1.32	3.6	
	N25	Lot 19	CLAY	178	189	189	>189	>186	1.7855	1.2864	38.8	2.33	300	38.5	2.7	1.28	2.6	
12/01/2017	N26	Lot 14	CLAY	>189	>189	UTP	UTP	>189	1.7964	1.2794	40.4	0.80	300	41.3	2.7	1.28	0.4	
	N27	Lot 17	CLAY	>189	>189	UTP	UTP	>189	1.7357	1.2292	41.2	3.71	300	36.9	2.7	1.26	6.3	
i his report sl Created By: Checked By: Authorised Si	TG TG TG ignatory: <b>4</b>	we many hand	Date: Date: Date:	8/12 25/01	2/2016 L/2017													Page: 1 of 2
Nachonseu S	ignatury.	ner Miner-1-	υαιε.	63/01	12017	and the second												Page: 1 of 2



CM	WGeosci	LF11 Rev 5 Soil F	ield Density NDN	1 Dire	ect Ti	ransr	nissio	on w	ith VSS	Report		Auckland Lab CMW Geosci Building C, 9 PO Box 3002 Phone: +64 (1	oratory ences Piermark Drive 06, Albany, Au 09) 4144 632	e, Rosedale, NZ ckland, NZ 075	2 0632 52			
Project:		45 Station Road, Huapai										Test Metho	ds:		Notes:			
Project No:		AKL2016_0634										NZS 4402.2	1:1986		Solid Densit	y:		Assumed
Location:		Huapai										NZS 4407.4.2.2:2015 Testing Locations Selected By: CMW F				CMW Field Staff		
Report No:		AKL2016_0634LAF Rev.0										NZGS:Augu	st 2001					
Report Date:		17/07/2018														12403		
Client:		Cabra Developments Limited											AN		lests indicated not accredited a	as ire outside	Measurement	s marked * are not accredited
Client Address	:														the scope of the	aditation	anu are outsiu	accreditation
Client Referen	ce:											ACC	REDITED LAE	ORATORY	aboratory's acci	editation		
				li	n-situ Va	ne Shear	Strength	าร				Field and	Laboratory T	esting Data				
Date Sampled	Sample No.	Test Location	Soil Description	Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m <sup>3</sup> ) **	Gauge Dry Density (t/m <sup>3</sup> )	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth	Oven Water Content (%)	Solid Density (t/m <sup>3</sup> ) *	Oven Dry Density (t/m <sup>3</sup> )	Calculated Air Voids (%) *	Comments
8/02/2018	N65	Refer to site plan	CLAY	155	155	UTP	UTP	155+	1.8720	1.3980	30.0	7.5	300	28.9	2.7	1.46	4.2	
26/02/2018	N66	Refer to site plan	CLAY	UTP	UTP	UTP	120	120+	1.8590	1.3508	34.0	1.0	300	31.8	3 2.7	1.42	2.9	
	N67	Refer to site plan	CLAY	UTP	180	140	180	167+	1.8680	1.3681	36.0	2.6	300	31.3	3 2.7	1.42	2.8	
	N68	Refer to site plan	CLAY	90	109	180	168	137	1.8010	1.2510	38.0	9.0	300					No Sample taken
	N69	Refer to site plan	CLAY	180	140	180	190	173	1.8460	1.3480	36.0	3.4	300	20.9	2.7	1.52	12	Retest of N68
5/03/2018	N70	Refer to site plan	CLAY	137	171	155	218+	170+	1.8410	1.3420	35.7	2.3	300	32.5	5 2.7	1.38	3.4	
	N71	Refer to site plan	CLAY	171	218	218	155	191	1.8520	1.3501	35.1	2.1	. 300	30.4	2.7	1.42	4.2	
6/03/2018	N72	Refer to site plan	CLAY	UTP	UTP	UTP	UTP	UTP	1.8920	1.4590	29.1	2.8	300	28.3	3 2.7	1.48	3.6	
	N73	Refer to site plan	CLAY	UTP	UTP	215	UTP	215+	1.8290	1.3750	32.0	3.0	300	32.4	2.7	1.38	4.1	
15/03/2018	N74	Refer to site plan	CLAY	UTP	UTP	UTP	UTP	UTP	1.8755	1.4444	29.8	3.3	300	27.6	5 2.7	1.48	5.0	
This report sl Created By:	nould only l	be reproduced in full.	Date: 09/02/2018			1		1	** Gauge Wet	Densities outsid	e of the calibrat	ed range of 1.72	28 to 2.756 t/m <sup>3</sup>	are not accredi	ted and are outs	de the laborato	ries scope of acc	editation.
Checked By: Authorised S	JLM ignatory:	CS	Date: 17/07/2018 Date: 19/07/2018															Page: 1 of 2

45 Station Road

#### 17/07/2018



Project:         45 Station Road         Test Methods:         Notes:         Solic           Project No:         AKL2016-0634         Solic         Solic	id Density: id Density Data Sour sting Locations Selec Only samples <2.0mm ting	Assumed ce: N/A ced By: CMW Field Staff	
Project No: AKL2016-0634 Solid	id Density Data Sour sting Locations Selec Only samples <2.0mm	ce: N/A ed By: CMW Field Staff	
	sting Locations Selec Only samples <2.0mm	ed By: CMW Field Staff	
Location: Huapai NZS 4407 2015 Test 3.1 ◊ Test	only samples <2.0mm	,	
Report No: NZS 4407 2015 Test 4.2 0	ting	will be considered for endorsed	
Report Date: 3/12/2020 NZS 4407 2015 Test 4.3 test	ung		
Client: Cabra Developments Limited NZGS:August 2001 ① F	Blade size of 19mm us	ed.	
Client Address:			
Test results ind accreditation accreditation	Test results indicated as not creating of a cutside the scope of the laboratory's accreditation		
Vane ID In-situ Vane Shear Strengths Field and Laboratory Testing Data			
Date Sample No. Test Location* Soil Description* $\begin{cases} Solid \\ Density \\ (t/m^3)* \end{cases}$ $Head # \\ \begin{cases} Blade \# \\ (kPa) \end{cases}$ $Test 1 \\ (kPa) \end{cases}$ $Test 2 \\ (kPa) \end{cases}$ $Test 4 \\ (kPa) \end{cases}$ $rest 4 \\ (kPa) $ $re$	en Water ntent (%) (t/m³)	Oven Calculated Comments Air Voids (%) *	
5/10/2020 N82 Refer to site plan Limed CLAY 2.70 1589 158 158 UTP 210 UTP 183+ 1.67 1.24 34.7 11 300		No sample taken	
N83 Refer to site plan Limed CLAY 2.70 1589 1589 UTP UTP UTP UTP 1.81 1.29 40.2 1 300	36.1 1.3	3 3	
N84 Refer to site plan Limed CLAY 2.70 1589 1589 UTP UTP UTP UTP 1.85 1.37 34.3 2 300	53.9 1.2	9- 9	
N85 Refer to site plan Limed CLAY 2.70 1589 1589 UTP UTP UTP UTP 1.84 1.36 35.4 1 300	13.3 1.6	3 18	
N86         Refer to site plan         Limed CLAY         2.70         1589         UTP         UTP         UTP         UTP         1.84         1.36         35.4         1         300	32.3 1.3	3 Redry of N85	
N87         Refer to site plan         Limed CLAY         2.70         1589         UTP         UTP         UTP         UTP         1.85         1.37         34.3         2         300	22.8 1.5	0 10 Redry of N84	
20/10/2020 N88 Old pond infill Limed CLAY 2.70 1195 1195 196 196 UTP 187+ 1.85 1.42 30.6 4 300	28.4 1.4	4 6	
N89 Old pond infill Limed CLAY 2.70 1195 1195 182 193 154 196 181 1.93 1.49 28.9 1 300	27.5 1.5	1 3	
N90         Old pond infill         Limed CLAY         2.70         1195         1195         182         182         196         UTP         187+         1.90         1.47         29.3         2         300	26.8 1.5	0 4	
N91         Old pond infill         Limed CLAY         2.70         1195         1195         98         126         140         126         1.81         1.30         38.9         1         300	32.0 1.3	7 6	
21/10/2020 N92 Old Pond Limed CLAY 2.70 1195 1195 140 154 121 196 153 1.90 1.44 32.1 0 300	29.7 1.4	7 2	
22/10/2020 N93 Old Pond Limed CLAY 2.70 1195 1195 154 168 196 140 165 1.91 1.43 33.3 -1 300	36.4 1.4	-3	
23/10/2020 N94 Old Pond outlet CLAY 2.70 1195 1195 196 196 UTP 196 196+ 1.86 1.41 31.6 3 300	26.4 1.4	7 7	
This report should only be reproduced in full. ** Gauge Wet Densities outside of the calibrated range of 1.728 to 2.756 t/m <sup>3</sup> are no Croated Ru: UM	lot accredited and are out	I I I I I I I I I I I I I I I I I I I	
Created by: JLIVI Date: //JU/2020			
Authorised Signatory: JW Date: 3/12/2020		Page: 1 of 2	



Page 2 of 2



## LF14 Rev.12 Dynamic Cone Penetration (DCP) Test Report NZS 4402: 1988 Test 6.5.2

Geos Geos	ciences					102.1500 1050						
Project:		45 Station Road										
Project No:		AKL2016-0634				Auckland Labor CMW Geoscien	atory ces (NZ) Ltd Part	nership				
Location:		Huapai				Building C, 9 Pie PO Box 300206	ermark Drive, Ro , Albany, Aucklar	sedale, NZ 0632 nd, NZ 0752				
Report No:		AKL2016-0634LAN	Rev.0			Phone: +64 (09	) 4144 632					
Test Date:		20/10/2020				Testing Locatio	ns Selected By:		CMW Field Staf	f		
Tested By:		JW/HN										
Client:		Cabra Developmen	ts Limited			PCCREDITED	Test results	indicated as not	* Equivalent CBR Values are not			
Client Address:						scope of the laboratory's accreditation				accredited and are outside the scope of the laboratory's accreditation		
Test No		1		2		3		4		5		
Test Location	Road 10 - (	Road 1 End)	Roa	id 10	Roa	ad 10	Roa	d 10	Roa	d 10		
Chainage & Offset	СН	10R	СН	20L	CH	130R	СН	40L	СН	50R		
Material & Layer	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG		
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*		
0 - 100	2	4	6	13	3	6	4	8	3	6		
100 - 200	2	4	4	8	3	6	4	8	3	6		
200 - 300	3	6	4	8	2	4	3	6	3	6		
300 - 400	4	8	4	8	3	6	4	8	3	6		
400 - 500	4	8	4	8	4	8	2	4	4	8		
500 - 600	4	8	4	8	4	8	5	10	4	8		
600 - 700	4	8	5	10	5	10	5	10	5	10		
700 - 800	5	10	4	8	6	13	6	13	5	10		
800 - 900	5	10	5	10	6	13	5	10	6	13		
900 - 1000												
Test No		6		7		8		9	1	10		
Test Location	Roa	d 10	Roa	d 10	Roa	d 10	Road	d 10	Roa	d 10		
Chainage & Offset	CH	60L	СН	70R	CH	180L	СН	90R	CH	100L		
Material & Layer	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG		
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*		
0 - 100	8	18	5	10	7	15	3	6	4	8		
100 - 200	3	6	3	6	4	8	3	6	3	6		
200 - 300	3	6	3	6	3	6	3	6	3	6		
300 - 400	4	8	3	6	4	8	3	6	3	6		
400 - 500	5	10	3	6	4	8	3	6	3	6		
500 - 600	7	15	4	8	5	10	4	8	3	6		
600 - 700	5	10	5	10	6	13	3	6	3	6		
700 - 800	9	20	5	10	7	15	4	8	3	6		
800 - 900	8	18	5	10	6	13	5	10	4	8		
900 - 1000			<u> </u>		L				 	- <b>6</b> -0		
Created by:	JLM			Date:	20/10/2020	I his report should only be reproduced in full  *Equivalent CBR values calculated using AUSTROADS (2010) Guide to Pavement Technology Part 2. Environ 5.2. Env Eino Grained Cabacity Saile on						
Checked by:	WL			Date:	3/12/2020		are	relevant to fine gra	ined cohesive soils o	only.		
Authorised Signatory:	JW			Date:	3/12/2020				Page 1 of 4			

<b>CMW</b> Geosciences

## LF14 Rev.12 Dynamic Cone Penetration (DCP) Test Report NZS 4402: 1988 Test 6.5.2

zs	4402:	1988	Test	6.5.2

Geos	ciences										
Project:		45 Station Road				A set la set la base					
Project No:		AKL2016-0634				CMW Geoscien	ices (NZ) Ltd Partnership				
Location:		Huapai				Building C, 9 Pie PO Box 300206	rmark Drive, Ro Albany, Aucklar	sedale, NZ 0632 nd, NZ 0752			
Report No:		AKL2016-0634LAN	Rev.0			Phone: +64 (09)	4144 632				
Test Date:		20/10/2020				Testing Locations Selected By:			CMW Field Staff		
Tested By:		JW/HN									
Client:		Cabra Developmen	ts Limited			PCCREDITED	Test results	indicated as not	* Equivalent CBB Values are not		
Client Address:						TEST	scope of th accreditat	laboratory's ion	accredited and are the laboratory	outside the scope of 's accreditation	
client Address.						NGLABORA					
Test No		11									
	Read 10 /R	LL									
lest Location	KUAU IU (K										
Chainage & Offset	CH.										
Material & Layer	CLA	1750				[					
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100	4	8									
100 - 200	2	4									
200 - 300	3	6									
300 - 400	2	4									
400 - 500	3	6									
500 - 600	4	8									
600 - 700	4	8									
700 - 800	5	10									
800 - 900	5	10									
900 - 1000											
Test No											
Test Location											
Chainage & Offset											
Material & Layer		1				-					
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100											
100 - 200											
200 - 300											
300 - 400											
400 - 500											
500 - 600											
600 - 700											
700 - 800											
800 - 900											
900 - 1000											
							This	report should on	ly be reproduced i	n full	
Created by:	JLM			Date:	20/10/2020		*Equivalent CE Pavement Technol	R values calculated ogy Part 2, Figure 5	using AUSTROADS ( .3, For Fine Grained	2010) Guide to Cohesive Soils, and	
Checked by:	WL			Date:	3/12/2020		are relevant to fine grained cohesive soils only.				
Authorised Signatory:	JW			Date:	3/12/2020 Page 2 of 4				Page 2 of 4		

AKL2018-0634LAN Rev.0

45 Station Road



AKL2018-0634LAN Rev.0

45 Station Road



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### LF14 Rev.12 Dynamic Cone Penetration (DCP) Test Report NZ5 4402: 1988 Test 6.5.2

Geos	ciences				NEC.	1402. 1900 1030	5.5.2				
Project:		45 Station Road									
Project No:		AKL2016-0634				Auckland Labor CMW Geoscien	atory ces (NZ) Ltd Part	nership			
Location:		Huapai				Building C, 9 Pie PO Box 300206	ermark Drive, Ro , Albany, Aucklar	sedale, NZ 0632 nd, NZ 0752			
Report No:		AKL2016-0634LAM	Rev.0			Phone: +64 (09	) 4144 632				
Test Date:		19/10/2020				Testing Locatio	ns Selected By:		CMW Field Staf	f	
Tested By:		JW/HN									
Client:		Cabra Developmen	ts Limited			* Equivalent (				R Values are not	
Client Address:						THING LABORAT	accredited scope of th accreditati	are outside the e laboratory's on	accredited and are outside the scope of the laboratory's accreditation		
Test No		1		2		3 4				5	
Test Location	Roa	id 11	Road 11 Road 11 Road 11						Roa	d 11	
Chainage & Offset	CH	200R	CH	210L	CH	220R	CH2	230L	CH2	240R	
Material & Layer	9	G	S	G	9	G	S	G	S	G	
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100	6	13	4	8	5	10	3	6	8	18	
100 - 200	4	8	3	6	4	8	3	6	4	8	
200 - 300	3	6	3	6	3	6	4	8	3	6	
300 - 400	3	6	2	4	3	6	4	8	2	4	
400 - 500	4	8	3	6	3	6	3	6	4	8	
500 - 600	3	6	2	4	3	6	4	8	5	10	
600 - 700	4	8	3	6	3	6	5	10	6	13	
700 - 800	5	10	3	6	4	8	5	10	10	20+	
800 - 900	5	10	3	6	3	6	4	8			
900 - 1000											
Test No		6		7		8		9	1	10	
Test Location	Roa	id 11	Roa	id 11	Roa	ad 11	Roa	id 11	Roa	d 11	
Chainage & Offset	CH	250L	CH2	260R	CH	270L	CH2	280R	CH2	290L	
Material & Layer	S	G	S	G	5	SG	S	G	S	G	
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100	3	6	5	10	5	10	1	2	5	10	
100 - 200	3	6	3	6	3	6	2	4	2	4	
200 - 300	3	6	2	4	3	6	2	4	3	6	
300 - 400	3	6	3	6	2	4	4	8	3	6	
400 - 500	4	8	3	6	2	4	4	8	2	4	
500 - 600	3	6	4	8	3	6	3	6	3	6	
600 - 700	2	4	9	20	4	8	3	6	3	6	
700 - 800	2	4	10	20+	3	6	3	6	3	6	
800 - 900	1	2	7	15	3	6	2	4	5	10	
900 - 1000					<u> </u>						
Created by:	JLM			Date:	20/10/2020	This report should only be reproduced in full *Equivalent CBR values calculated using AUSTROADS (2010) Guide Pavement Technology Part 2, Figure 5.3, For Fine Grained Cohesive So				n full 2010) Guide to Cohesive Soils, and	
Authorized Stanaton	WL			Date:	3/12/2020		are	relevant to fine gra	Page 1 of 7	y.	
Authorised Signatory:	JW			Date:	3/12/2020				rage IUI/		



### LF14 Rev.12 Dynamic Cone Penetration (DCP) Test Report NZ5 4402: 1988 Test 6.5.2

Geose Geose	ciences				1423 4402. 1300 1630 0.3.2						
Project:		45 Station Road									
Project No:		AKL2016-0634				Auckland Labor CMW Geoscien	atory ces (NZ) Ltd Part	nership			
Location:		Huapai				Building C, 9 Pie PO Box 300206	ermark Drive, Ro , Albany, Aucklar	sedale, NZ 0632 nd, NZ 0752			
Report No:		AKL2016-0634LAM	Rev.0			Phone: +64 (09	4144 632				
Test Date:		19/10/2020				Testing Location	ns Selected By:		CMW Field Staf	f	
Tested By:		JW/HN				CREDITA	1				
Client:		Cabra Developmen	ts Limited				Test results i accredited	ndicated as not are outside the	* Equivalent CB	R Values are not	
Client Address:						TESTING	scope of the accreditation	accredited and are outside the scope of the laboratory's accreditation			
						- LABO					
Test No	1	1	1	.2	:	13	1	4	1	.5	
Test Location	Roa	d 11	Roa	d 11	Roa	id 11	Roa	d 11	Roa	d 11	
Chainage & Offset	CH	300R	CH	310L	CH	320R	CH	330L	CH	340R	
Material & Layer	S	G	S	G	5	G	S	G	S	G	
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100	4	8	4	8	2	4	3	6	5	10	
100 - 200	2	4	3	6	4	8	3	6	3	6	
200 - 300	3	6	3	6	2	4	3	6	2	4	
300 - 400	3	6	4	8	2	4	2	4	3	6	
400 - 500	4	8	4	8	3	6	2	4	3	6	
500 - 600	3	6	6	13	3	6	3	6	2	4	
600 - 700	4	8	4	8	3	6	3	6	3	6	
700 - 800	3	6	5	10	5	10	3	6	3	6	
800 - 900	3	6	6	13	3	6	3	6	3	6	
900 - 1000											
Test No	1	16	1	.7	-	18	1	.9	2	0	
Test Location	Roa	d 11	Roa	d 11	Roa	ad 11	Roa	d 11	Roa	ad 1	
Chainage & Offset	CH	350L	CH3	860R	CH	370L	CHE	380R	CH4	10R	
Material & Layer	S	G	S	G	5	G	S	G	S	G	
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100	6	13	3	6	2	4	5	10	3	6	
100 - 200	3	6	2	4	2	4	2	4	3	6	
200 - 300	4	8	2	4	2	4	2	4	3	6	
300 - 400	4	8	2	4	3	6	2	4	3	6	
400 - 500	3	6	2	4	4	8	3	6	3	6	
500 - 600	3	6	2	4	5	10	2	4	3	6	
600 - 700	4	8	3	6	6	13	4	8	3	6	
700 - 800	3	6	2	4	6	13	3	6	4	8	
800 - 900	5	10	3	6	5	10	3	6	3	6	
900 - 1000	<u> </u>				<u> </u>				L		
Created by:	JLM			Date:	20/10/2020		*Equivalent CE	report should on	ly be reproduced i	n full 2010) Guide to	
Checked by:	JW			Date:	3/12/2020		ravement lechnol	relevant to fine gra	ined cohesive soils c	onesive soils, and inly.	
Authorised Signatory:	JW			Date:	3/12/2020				Page 2 of 7		



## LF14 Rev.12 Dynamic Cone Penetration (DCP) Test Report NZS 4402: 1988 Test 6.5.2

Geos	ciences					102. 1900 1050								
Project:		45 Station Road												
Project No:		AKL2016-0634				Ацскнана Laboratory CMW Geosciences (NZ) Ltd Partnership								
Location:		Huapai				Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752								
Report No:		AKL2016-0634LAM	Rev.0			Phone: +64 (09)								
Test Date:		19/10/2020				Testing Location	ocations Selected By: CMW Field Staff							
Tested By:		JW/HN												
Client:		Cabra Developmen	ts Limited			PCCREDITEO	RED/7E0 Test results indicated as not * Environment CBR Values are							
Client Address:						TEATING LABORAT	accredited scope of th accreditati	are outside the e laboratory's on	accredited and are outside the scope of the laboratory's accreditation					
Test No		21	2	2		23	2	24	2	5				
Test Location	Ro	ad 1	Roa	ad 1	Ro	ad 1	Roa	ad 1	Roa	ad 1				
Chainage & Offset	CH	390L	CH	380R	CH	370L	CH	360R	CH	350L				
Material & Layer	9	G	S	G	9	G	S	G	S	G				
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*				
0 - 100	9	20	6	13	4	8	4	8	3	6				
100 - 200	7	15	8	18	4	8	3	6	2	4				
200 - 300	6	13	10	20+	3	6	3	6	2	4				
300 - 400	4	8	8	18	4	8	2	4	4	8				
400 - 500	4	8	4	8	4	8	3	6	3	6				
500 - 600	3	6	3	6	3	6	4	8	3	6				
600 - 700	3	6	3	6	3	6	6 4		2	4				
700 - 800	3	6	3	6	3	6	6 2		2	4				
800 - 900	3	6	3	6	3	6	3	6	2	4				
900 - 1000														
Test No	2	26	2	.7	:	28	2	.9	30					
Test Location	Ro	ad 1	Roa	ad 1	Ro	ad 1	Road 1		Roa	Road 1				
Chainage & Offset	CH	340R	CH	330L	CH	320R	CH300L		CH2	CH290R				
Material & Layer	S	G	S	G	5	G	S	G	S	SG				
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*				
0 - 100	4	8	13	20+	9	20	5	10	5	10				
100 - 200	4	8	7	15	5	10	4	8	4	8				
200 - 300	3	6	4	8	3	6	5	10	3	6				
300 - 400	4	8	5	10	3	6	4	8	3	6				
400 - 500	4	8	3	6	2	4	3	6	3	6				
500 - 600	5	10	4	8	3	6	2	4	3	6				
600 - 700	6	13	2	4	3	6	4	8	3	6				
700 - 800	7	15	3	6	3	6	5	10	3	6				
800 - 900	6	13	3	6	3	6	4	8	3	6				
900 - 1000	1		<u> </u>		<u> </u>				 	- f. II				
Created by:	JLM			Date:	20/10/2020		This report should only be reproduced in full *Equivalent CBR values calculated using AUSTROADS (2010) Gui			n Tull 2010) Guide to Cohesive Soils. and				
Checked by:	WL			Date:	3/12/2020		are	relevant to fine gra	ined cohesive soils o	nly.				
Authorised Signatory:	JW			Date:	3/12/2020				Page 3 of 7					

CAALA	
CMW	Geosciences

### LF14 Rev.12 Dynamic Cone Penetration (DCP) Test Report

zs	4402:	1988	Test	6.5.2

CMW Geos	ciences				NZ	NZS 4402: 1988 Test 6.5.2								
Project:		45 Station Road												
Project No:		AKL2016-0634				Auckland Labor CMW Geoscien	:land Laboratory N Geosciences (NZ) Ltd Partnership							
Location:		Huapai				Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752								
Report No:		AKI 2016-0634I AM	Rev.0			) 4144 632								
Test Date:		19/10/2020				Tasting Locations Selected Pro								
Tested Dur		104//LIN					is selected by.							
Tested By:		JW/HN				PCCREDITEO			* Equivalent CBR Values are not accredited and are outside the scope of the laboratory's accreditation					
Client:		Cabra Developmen	its Limited				Test results accredite scope of t	a indicated as not are outside the he laboratory's						
Client Address:						STING LABORA	o <sup>e</sup> accredita	tion						
	-				•									
Test No	:	31	:	32										
Test Location	Ro	ad 1	Ro	ad 1										
Chainage & Offset	CH	280L	CH	270R										
Material & Layer	9	SG	9	G				1		1				
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*				
0 - 100	3	6	5	10										
100 - 200	4	8	4	8										
200 - 300	3	6	4	8										
300 - 400	3	6	4	8										
400 - 500	3	6	5	10										
500 - 600	4	8	5	10										
600 - 700	4	8	6	13										
700 - 800	6	13	7	15										
800 - 900	6	13	7	15										
900 - 1000														
Test No														
Test Location														
Chainage & Offset														
Material & Layer		ŀ		1		1								
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*				
0 - 100														
100 - 200														
200 - 300														
300 - 400														
400 - 500														
500 - 600														
600 - 700														
700 - 800														
800 - 900														
900 - 1000	<u> </u>							.,	<u> </u>					
							This	report should on	iy be reproduced i	n tull				
Checked by:	JLM			Date:	20/10/2020		Pavement Techno	logy Part 2, Figure 5	3.3, For Fine Grained	Cohesive Soils, and				
Authorised Signatory	WL			Date:	3/12/2020		are	relevant to fine gra	Page 1 of 7	nny.				
machioriseu Signatury:	iatory: JW Date:				3/12/2020		Page 4 of 7							

AKL2018-0634LAM Rev.0

45 Station Road





### 19/12/2020

AKL2018-0634LAM Rev.0

45 Station Road



CM	WGeosci	LF11 Rev.13 Soil Field	d Density NDM Di	irect Tra	nsmiss	sion w	rith V	SS Re	port (	Cohe	sive S	oils)		Auckland Labo CMW Geoscie Building C, 9 F PO Box 30020 Phone: +64 (0	oratory ences (NZ) Ltd Piermark Drive 16, Albany, Aud 19) 4144 632	Partnership , Rosedale, NZ :kland, NZ 0752	0632 2			
Project:		45 Station Road Test Methods: Notes: Solid Density: Assumed								Assumed										
Project No:		AKL2016-0634															Solid Density	y Data Source	:	N/A
Location:		Huapai												NZS 4407 20	15 Test 3.1 🛇		Testing Loca	tions Selecte	d By:	CMW Field Staff/Contractor
Report No:		AKL2016-0634LAO Rev.0												NZS 4407 20	15 Test 4.2		0 Only camp	les <2 0mm v	vill be conside	ared for endorsed testing
Report Date:		20/04/2021												NZS 4407 20	15 Test 4.3		v only samp	103 2.011111	will be conside	ered for endorsed testing
Client:		Cabra Developments Limited												NZGS:August	t 2001		① Blade size	of 19mm use	ł.	
Client Address	:													TEST	NG LABOR ALO	Test results i accredited scope of th accreditatio	ndicated as not are outside the e laboratory's on		Measurement outside the se	s marked * are not accredited and are cope of the laboratories accreditation
					Var	ne ID		In-situ Va	ane Sheai	r Strengtl	ns			Fie	eld and Labora	tory Testing D	ata			
Date Sampled	Sample No.	Test Location*	Soil Description*	Solid Density (t/m <sup>3</sup> ) *	Head #	Blade #	Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m <sup>3</sup> ) **	Gauge Dry Density (t/m <sup>3</sup> )	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth (mm)	Oven Water Content (%)	Oven Dry Density (t/m <sup>3</sup> )	Oven Calculated Air Voids (%) *	Comments
25/03/2021	N95	Refer to site plan	CLAY	2.70	1195	1195	UTP	UTP	UTP	UTP	UTP	1.81	1.36	33.6	4	300	31.0	1.38	6	
26/03/2021	N96	Old Pond	CLAY	2.70	1195	1195	159	196	UTP	UTP	178+	1.90	1.43	32.9	0	300	33.2	1.43	0	
7/04/2021	N97	Station Road	CLAY	2.70	1195	1195	196	182	140	196	179	1.86	1.42	31.3	3	300	27.3	1.46	6	
This report sl	nould only	be reproduced in full.			1							** Gauge Wet [	Densities outsid	e of the calibrate	d range of 1.728	to 2.756 t/m <sup>3</sup> a	re not accredited	d and are outsid	e the laboratorie	s scope of accreditation.
Created By:	ШΜ		Date:				29/0	13/2021												
Checked By:	JLM		Date:				20/0	)4/2021												
Authorised S	ignatory:	W	Date:				20/0	04/2021												Page: 1 of 2

#### AKL2016-0634LAO Rev.0

#### 45 Station Road

#### 20/04/2021



05/02/2020

A3

1

08/04/2020

## **Appendix E: Producer Statements**



26 March 2021

Document Ref: AKL2016\_0634AQ Rev.0

Cabra Developments Limited PO Box 197 Orewa 0946

Attention: Duncan Unsworth

Dear Sir

## RE: CONSTRUCTION REVIEW FOR TIMBER POLE CANTILEVER RETAINING WALLS 11A, 11B, 11C, 13B, 13C AND KEYSTONE RETAINING WALL 12 – BCO10306103

#### **19 PODGORA AVENUE, HUAPAI (45 STATION ROAD)**

CMW Geosciences (CMW) has visited the site at 19 Podgora Avenue, Huapai (45 Station Road) legally described as Lot 32 DP 544111 on several occasions between October 2020 and March 2021 to observe the site works for the construction of timber pole retaining walls 11A, 11B, 11C, 13B, 13C and keystone retaining wall 12.

Our work has included review of the following documents and drawings:

- Conditions of Auckland Council Building Consent referenced BCO10306103 issued 3 July 2020;
- Consented construction drawings, prepared by CMW Geosciences, referenced Stage 1A & 1B Keystone Wall 1-3 and Stage 1A &1B Timber Pole Retaining Wall 1-3, figures 201 to 206 dated 23 March 2017;
- Geotechnical report for Stage 1A & 1B Retaining Wall Design prepared by CMW Geosciences, referenced AKL2016\_0634AD Rev.0, dated 23 March 2017.

The site works observed and/or tested by CMW staff for the <u>Timber Pole Cantilever Retaining Walls</u> incorporated:

- Measurement of pile hole depth, spacing and diameter;
- Assessment of soil strengths in the pile holes;
- Measurement of timber pile and rail sizes;
- Drainage placement and outlets;
- Confirmation of timber treatment levels.

Inspections for timber pole retaining walls 11A, 11B, 11C, 13B and 13C were carried out between October 2020 and November 2020. Vane shear strengths met the design specifications in the bases and sides of the pile holes and averaged over 60kPa. No groundwater was encountered in any of the pile holes.

The measurements, dimensions and drainage placement and outlets across the timber pole cantilever retaining walls were observed and met design specifications.

The site works observed and/or tested by CMW staff for the Keystone Retaining Wall incorporated:

- Assessment of soil strengths at subgrade and In the retained ground;
- Drainage placement and outlets,
- Backfill aggregate quality and placement;
- Geogrid type, orientation and placement.

In March 2021 CMW inspected the construction of keystone wall 12. Vane shear strengths within the subgrade and retained ground met designed specifications and were in excess of 100kPa.

The compaction of the hardfill between geogrid layers was tested using an impact hammer and observed values ranged from 18 to 20 CIV. The hardfill was observed to be generally well compacted.

Geogrid type, orientation, placement, drainage and outlets were all observed to be as per design specifications.

On the basis of our observations and testing, we consider that the works have been undertaken in accordance with the approved Building Consent and related approved documentation described above, are in accordance with the requirements and/or recommendations of the geotechnical design report and provide the basis for our attached PS4 Construction Review producer statement.

CMW's site presence during construction for this project included periodic observations of specific elements of work as described herein. As we were not on site at all times during construction, we have relied on the Contractor's diligence, construction observations and their attached PS3 certification to ensure that the works have been carried out in accordance with:

- a) The approved Contract drawings and design details;
- b) The approved Contract specifications;
- c) Authorised Variations to (a) and (b) during the execution of the works;
- d) The conditions of Resource and Building Consents where applicable;
- e) The relevant Geotechnical Investigation reports, recommendations and site instructions;

and that all as-built information and other details provided to the Client and/or CMW are accurate and correct in all respects.

For and on behalf of CMW Geosciences

Knowles RA

**Richard Knowles** 

Principal Geotechnical Engineer + CPEng

Distribution: 1 electronic copy to Cabra Developments Limited via email Original held at CMW Geosciences

Attachments: Producer Statement - Construction Review





17 March 2020

Document Ref: AKL2016\_0634AL Rev.0

Cabra Developments Limited PO Box 197 Orewa 0946

Attention: Duncan Unsworth

Dear Sir

## RE: CONSTRUCTION REVIEW FOR TIMBER POLE CANTILEVER AND KEYSTONE RETAINING WALLS- CONSENT BCO10092414-1-B

#### 45 STATION ROAD, HUAPAI (53 STATION ROAD, HUAPAI)

CMW Geosciences (CMW) has visited the site at 45 Station Road, Huapai now legally described as 53 Station Road, Huapai Lot 2 DP 533552 on several occasions between April 2017 and February 2020 to observe the site works for the construction of timber pole cantilever retaining walls 4A (beyond chainage 170m), 7A, 7B, 7C, 9, 10, 11A and keystone retaining walls 3, 8 and 27.

Our work has included review of the following documents and drawings:

- Conditions of Auckland Council Building Consent referenced BCO10092414-1-B issued 5 May 2017;
- Consented construction drawings, prepared by CMW Geosciences, referenced AKL2016\_0634 Stage 1A & 1B Keystone Wall figures 201 to 203 and Stage 1A & 1B Timber Pole Wall figures 204-206 dated 23 March 2017;
- Geotechnical Report for Stage 1A & 1B Retaining Wall Designs 45 Station Road, Huapai prepared by CMW Geosciences, referenced AKL2016\_0634AD Rev.0, dated 23 March 2017.

The site works observed and/or tested by CMW staff for the Keystone Retaining Walls incorporated:

- Assessment of soil strengths at subgrade and retained ground;
- Drainage placement and outlets;
- Backfill aggregate quality and placement;
- Geogrid type, orientation and placement.

Our testing demonstrated vane shear strengths in the subgrade and retained ground that met design specifications. Drainage placement and outlets were observed by CMW.

During February 2020 a few layers of wall 27 were removed and sleeves for fence posts were installed. Grids and compacted hardfill was then re-installed. The orientation and placement of grids were inspected by CMW

and met design specifications. Hardfill compaction was inspected using impact hammer tests; CIV values met design specifications ranging between 228 and 35.

The site works observed and/or tested by CMW staff for the <u>Timber Pole Cantilever Retaining Walls</u> incorporated:

- Measurement of pile hole depth, spacing and diameter;
- Assessment of soil strengths in the pile holes;
- Measurement of timber pole and rail sizes;
- Drainage placement and outlets;
- Confirmation of timber treatment levels.

Between April 2017 and February 2020 CMW inspected the construction of the timber pole cantilever retaining walls. Vane shear strengths in the bases and sides of the pile holes met design specifications, exceeding 70kPa in the 2 metre crust and in excess of 50kPa below 2 metres depth. Groundwater was encountered in some of the piles holes during construction and the contractors were advised to pump this out before concrete was poured.

During December 2019 it was observed that the bridging detail piles at the wall 11A and 11B connections had been drilled too close together. Two piles were subsequently pulled out and redrilled to the correct spacing and depth. Bridging details across the remaining pipe crossings were inspected and met design specifications.

The measurements, dimensions and drainage placement and outlets across the timber pole cantilever retaining walls were observed and met design specifications.

On the basis of our observations and testing, we consider that the site works have been undertaken in accordance with the approved Building Consent and related approved documentation described above, are in accordance with the requirements and/or recommendations of the geotechnical design report and provide the basis for our attached PS4 Construction Review producer statement.

CMW's site presence during construction for this project included periodic observations of specific elements of work as described herein. As we were not on site at all times during construction, we have relied on the Contractor's diligence, construction observations and their attached PS3 certification to ensure that the works have been carried out in accordance with:

- a) The approved Contract drawings and design details;
- b) The approved Contract specifications;
- c) Authorised Variations to (a) and (b) during the execution of the works;
- d) The conditions of Resource and Building Consents where applicable;
- e) The relevant Geotechnical Investigation reports, recommendations and site instructions;

and that all as-built information and other details provided to the Client and/or CMW are accurate and correct in all respects.

For and on behalf of CMW Geosciences

RJ Knowles

Richard Knowles Principal Geotechnical Engineer

- Distribution: 1 electronic copy to Cabra Developments Limited via email Original held at CMW Geosciences
- Attachments: Producer Statement Construction Review





PRODUCER STATEMENT – PS4 – CONSTRUCTION REVIEW
ISSUED BY: CMW Geosciences (NZ) Limited Partnership
(Construction Review Firm) TO: Cabra Developments Limited
(Owner/Developer) TO BE SUPPLIED TO: Auckland Council (Building Concent Authority)
IN RESPECT OF: The Construction of Timber Cantilever Retaining Walls 4A (part), 7A, 7B, 7C, 9, 10, 11 and (Description of Building Work)
AT: Segmental Block Retaining Walls 3, 8 and 27 at 45 Station Road, Huapai (53 Station Road, Huapai) (Address)
Town/City: Auckland LOT. <sup>2</sup> DP 533552 SO
We .CMW Geosciences (NZ) Limited Partnership have been engaged by .Cabra Developments Limited (Construction Review Firm)
To provide CM1 CM2 CM3 CM4 CM5 (Engineering Categories) or observation as per agreement with
owner/developer.Cabra Developments Limited
or otheras described in CMW letter referenced AKL2016_0634AL Rev.0, dated 17 March 2020 (Extent of Engagement)
in respect of clause(s)
documents relating to Building Consent No. BCO10092414-1 and those relating to
Building Consent Amendment(s) Nos. BCO10092414-1-B issued during the course of the works. We have sighted these Building Consents and the conditions of attached to them.
Authorised instructions/variations(s) No
On the basis of this review these review(s) and information supplied by the contractor during the course of the works and <b>on behalf of the firm</b> undertaking this Construction Review, <b>I believe on reasonable grounds</b> that All or Part only of the building works have been completed in accordance with the relevant requirements of the
Building Consent and Building Consent Amendments identified above, with respect to Clause(s)B1
I, am: CPEng
I am a member of: Engineering New Zealand NZIA and hold the following qualifications BE (civil), CMEngNZ, CPEng The Construction Review Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less than \$200,000*.
The Construction Review Firm is a member of ACENZ:
SIGNED BY Richard Knowles (AC Author #2342) (Name of Construction Review Professional)
ON BEHALF OF CMW Geosciences (NZ) Limited Partnership Date. 18/3/20
Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of \$200,000*.

This form is to accompany Forms 6 or 8 of the Building (Form) Regulations 2004 for the issue of a Code Compliance Certificate.

THIS FORM AND ITS CONDITIONS ARE COPYRIGHT TO ACENZ, ENGINEERING NEW ZEALAND AND NZIA

PRODUCER STATEMENT PS4

Building Code Clause(s). B1