

17 March 2020

45 STATION ROAD, STAGE 1A HUAPAI

GEOTECHNICAL COMPLETION REPORT

Cabra Developments Limited Ref: AKL2016_0634AK Rev.0

AKL2016_0634AK				
Date	Revision	Comments		
17 March 2020	А	Initial draft for internal review		
18 March 2020	0	Final issue to client		

	Name	Signature	Position
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Reviewed by	Andrew Linton	Jus	Principal Geotechnical Engineer
Authorised by	Richard Knowles	RJ Knowles	Principal Geotechnical Engineer



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1. INTRODUCTION

In accordance with our instructions, this Geotechnical Completion Report has been prepared for Cabra Development Limited as part of the documentation to be submitted to Auckland Council following earthworks to form Stage 1A of the Huapai Triangle Sub Precinct A (45 Station Road, Huapai) Development. Construction of this residential subdivision has been undertaken in accordance with the Auckland Council Resource Consent number ENG 60068582 and SUB 60035794 and Engineering Approval letter dated 23 February 2017. Specific structures constructed during the civil works to create the subdivision include timber pole retaining walls and keystone retaining walls.

This report contains our Suitability Statement, specific comments related to items raised in the Resource Consent, relevant test data and the Cato Bolam Consultants As-built plan set as provided in Appendix B.

This report covers the construction period December 2016 to February 2020 and is intended to be used for certification purposes for new lots (listed below) created from Lot 1 DP 540873 as follows:

- 54 new residential lots numbered balance lot 2, together with lots 13 to 58 and 60 to 66;
- 1 new road numbered lot 1002 and named Vinistra Road;
- 2 new drives numbered lots 1001 and 1003 and named Dida Park Drive and Vintry Drive respectively;
- 1 new avenue numbered lot 1003 and named Podgora Avenue;
- 1 new right of way numbered 500.

This stage of the 45 Station Road Development is located off Station Road, Huapai. As can be seen from the As-built plans, 52 of the lots have been affected by filling as part of the earthworks operations to a maximum depth of approximately 10 metres.

2. PROJECT BACKGROUND

The geotechnical investigations and design were undertaken by CMW Geosciences as presented in the following reports:

- Geotechnical Report for Huapai Development Stage 1, Huapai, prepared by CMW Geosciences referenced 2015_1029AB Rev.0 dated 24 November 2014;
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced AKL2017_0089AB Rev.0 dated 12 July 2017;
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced AKL2018-0195AA Rev.0 dated 12 December 2018;
- Stage 1A & 1B Retaining Wall Designs prepared by CMW Geosciences, referenced AKL2016_0634AD Rev.0 dated 23 March 2017;
- Remaining Scope of Stage 1A & 1B Retaining Wall Designs prepared by CMW Geosciences, referenced AKL2016_0634AH Rev.1 dated 19 March 2019;
- Timber Pole Retaining Wall 2A prepared by CMW Geosciences, referenced AKL2016_0634AI Rev.0 dated 6 September 2019.

3. DESCRIPTION OF EARTHWORKS

Earthworks operations for the entire 45 Station Road Development began in early December 2016 with the installation of silt fences and other environmental controls. Cut and fills for Stage 1A were conducted in conjunction with the other stages of this development and with neighbouring developments, all of which

were under the observation of CMW Geosciences. The majority of this stage is in fill, with a small area of cut required to form approximately 10 of the western lots and a portion of Vinistra Road and Dida Park Drive.

By March 2017 the bulk earthworks for Stage 1A was mostly complete. A gully muck-out was completed in the northern portion of the site running from east to west and two subsoil drain coils were placed surrounded by scoria and Bidim cloth. In February 2018 an undercut was conducted along the western boundary of Lots 20 to 27 where the proposed retaining wall was to be constructed, through approximately 8 lots, and some of Vinistra Road to the east. A subsoil drain was installed, and filling placed in this area until finished level was reached. Development on Stage 1A was then put on hold for the remainder of the 2018 season.

Development of Stage 1A recommenced in mid-January 2019 and began with an undercut in the south western corner of the site (Lots 27 to 31). An underfill drain was installed along the back of the undercut behind the proposed retaining wall. Backfilling of this undercut was completed in late January 2019.

Civil works including road construction began in January 2019 and continued until completion of this stage in February 2020.

Construction of the timber pole and segmental block retaining walls began in April 2017 with completion of both types of walls in February 2020 and certification to be provided separately to this report.

4. GEOTECHNICAL QUALITY CONTROL

4.1. Site Observations

During the earthworks site visits were typically undertaken several times each week to assess compliance with NZS 4431 and specific design recommendations and specifications.

Site visits were carried out to observe and confirm compliance relating to:

- Adequate topsoil stripping;
- Fill areas prior to the placement of fill materials to ascertain that all mullock and soft inorganic subsoils had been removed;
- Installation and backfilling of subsoil drains;
- Excavation and backfilling of sewer and stormwater trenches;
- Construction of cantilever timber pole retaining walls including ground conditions, pile size, spacing and depth; and
- Construction of keystone walls including ground conditions, block placement, geogrid placement and hardfill backfill;
- Placement and compaction of engineered fills.

4.2. Compaction Control

Compaction of engineered earth fills was controlled by undrained shear strength measured by hand held shear vane calibrated using the NZGS 2001 method and by air voids as defined by NZS4402.

General Fills

The criteria for undrained shear strength were a minimum single value of 110 kPa and minimum average of any 10 consecutive tests of 140 kPa.

The criteria for air voids were a maximum single value of 12% and maximum average of any 10 consecutive tests of 10%.

Vane shear strength, water content and in situ density tests were carried out on all areas of the engineered filling to at least the frequency recommended by NZS 4431.

These tests showed on occasions that the contractor was struggling to achieve the required compaction standards with the prevailing site and soil conditions, but to the best of our knowledge, all areas of fill were re-worked as necessary. Subsequent testing confirmed compliance with the specification.

5. EVALUATION OF COMPLETED EARTHWORKS

5.1. Natural Hazards

The appended as-built drawings depict the extents of a series of zones that contain limitations intended to ensure that future building and/ or earthworks on the lots is undertaken in a manner that does not lead to buildings being subject to any of the natural hazards described in Section 71(3) of the Building Act, i.e. erosion, falling debris, subsidence, slippage, and inundation. Consideration of the inundation hazard was outside the scope of CMW's brief and has been assessed by others. The applied zones on this subdivision are all **Specific Design Zones (retaining)** - intended to protect the retaining walls from overloading at the crest or undermining at the toe that could lead to instability.

Full descriptions of the restrictions associated with each of these zones are presented in the Suitability Statement (Appendix A). Additional information is also provided in some of the following sections.

5.2. Natural Soils Geotechnical Assessment

Portions of lots within this stage of the subdivision are formed partially within natural soils, which are predominantly of alluvial origin.

During construction and earthworks, lenses of firm to stiff organically stained clay soils were observed and were undercut and replaced with engineered fill.

Hand auger boreholes conducted in the centre of each lot as part of the post earthworks investigations did not encounter further lenses of organically stained soils but observed some lower strength natural subsoils.

While not expected, some discrete lenses of organic stained clay soils may still exit within the natural soils beneath lot areas. We do not consider that liquefaction and/or settlement due to discrete lenses of organically stained clay soils are a significant geotechnical risk for development or future residential dwellings built in general accordance with NZS 3604.

5.3. Land Stability and Erosion Control

The subdivision scheme layout includes a series of batter slopes to form level terraces for building platforms. The batters include portions of the residential lots with maximum gradients of 1(v) in 2.5(h) as depicted on the as-built drawings.

Design of the works to provide appropriate stability conditions that meet regulatory requirements for the land within this stage, has led to the construction of deep subsoil drainage, cantilever pole retaining walls and keystone walls.

Building and landscape designers must ensure that all runoff from solid surfaces is directed into the stormwater system. It is also important that care is paid to the disposal of stormwater during construction so that concentrated discharges (e.g. from unconnected spouting) are not directed towards steep ground.

Depths of mulch and topsoil applied to sloping areas should be limited to less than 150mm to minimise the risks of saturation leading to localised slumping on batter faces. Wherever practical on such land, and particularly on steep batters, existing vegetation and grass cover should be well maintained. Any vegetation cleared beyond the immediate area of building platforms for temporary construction purposes should be replanted or replaced as soon as possible. The roots of an established vegetation cover can serve to bind the surface soils while the foliage can reduce rain infiltration and soil saturation, resulting in better resistance to erosion and shallow slumping.

5.4. Retaining Walls

Cantilever timber pole and keystone retaining walls have been constructed in the locations shown on the appended Cato Bolam Limited As-built Plans. These walls reach a maximum height of approximately 2 metres and were designed by CMW Geosciences, and the construction was also observed by this consultancy. Copies of the Producer Statements - Construction Review (PS4) are provided in Appendix E.

Descriptions of the building and earthworks restrictions within the vicinity of these walls (Specific Design Zones – retaining) are contained in the Suitability Statement in Appendix A. Lots containing these zones include balance lot 2, together with lots 13 to 58 and 60 to 66 inclusive.

5.5. Fill Induced Settlement

On the basis of the elapsed time since fill has been placed across this stage of the subdivision, we consider that remaining post-construction settlements will be within code limits.

5.6. Service Line Trenches

As part of the civil works, sanitary sewer and stormwater services were trenched throughout the development as shown on the appended Cato Bolam Consultants Stormwater and Wastewater As-built Plans.

As is normal on all subdivisions, building developments involving foundations within a 45 degree zone of influence from pipe inverts will require engineering input. The Auckland Council drawing referenced SW22 provided in Appendix B extracted from Chapter 4 of the Auckland Council Code of Practice for Land Development and Subdivision depicts their requirements for stormwater pipes. Details for water and wastewater pipes are available in the Watercare COP1 - General Requirements and Procedures. The majority of lots are known to have service trenches within the lots as shown on the appended stormwater and wastewater as-built plans. The resulting restrictions are presented in the Suitability Statement below.

5.7. Subsoil Drains

The appended Cato Bolam Consultants Cut/Fill As-built Plans show the positions of counterfort drains which were constructed in the natural ground during the earthworks operations. The drains were installed to help control groundwater levels and are either linked to the reticulated storm water system or extend to formed outlets within bush areas. The ongoing operation of these drains is important to the overall stability conditions of the site.

Typical trench excavations extended into natural ground and lie between 1.4 and 9.9 metres below the finished surface. Accordingly they are predominantly beyond the depths of anticipated foundations.

Descriptions of the restrictions related to these subsoil drains are contained in the appended Suitability Statement.

5.8. Road Subgrades

Penetration resistance testing was carried out on the road subgrades during construction and the results of this testing were forwarded to Cato Bolam Consultants for pavement remedial design. Where soft ground with low equivalent CBR values was identified it was generally undercut and replaced with engineered fill, including placement of geogrid and geotextile cloth. All road subgrade areas were subsequently lime/ cement stabilised to achieve appropriate CBR values.

Benkelman Beam testing of the base course was carried out by Roadtest Limited on each road and those results were also forwarded to Cato Bolam Consultants.

5.9. Design of Shallow Foundations

5.9.1. Bearing Capacity

Once bulk earthworks and top-soiling of the building platforms had been completed, our staff drilled hand auger boreholes on platforms in natural ground to determine representative finished ground conditions and hence evaluate likely foundation options for future building development. Our assessments of bearing capacity for the design of shallow foundations on each building platform are contained in the appended Suitability Statement.

At current subgrade levels balance lot 2, together with lots 13 to 58 and 60 to 66 inclusive have been assessed as having a geotechnical ultimate bearing capacity of 300 kPa within the influence of conventional shallow residential building foundation loads.

If higher geotechnical ultimate bearing capacities are required, further specific site investigation and design of foundations should be carried out prior to Building Consent application.

5.9.2. Foundation Settlements

At the bearing pressures specified above, and subject to the design requirements for soil expansiveness provided below, differential settlement of shallow foundations for buildings designed in accordance with NZS 3604 (including the 600mm subfloor fill depth limit) should be within code limits.

5.9.3. Soil Expansiveness Classification

12 sets of soil tests were carried out on samples taken from likely foundation level on lots within this stage of the development.

Testing to assess the Shrink-Swell Index (I_{ss}) was completed in accordance with AS 1289 Test 7.1.1 and was used in conjunction with the advice in Acceptable Solution B1/AS1 of the New Zealand Building Code to calculate the characteristic surface movement (y_s) and the expansive soil class. All test results are appended.

On the basis of the laboratory test results, and the general consistency of these results across both cut and fill areas, we have assessed the lots to be H (Highly) expansive.

Foundation design details for a specified range of NZS 3604 dwellings, including minimum embedment depths and reinforcing steel requirements are also contained in B1/AS1.

In recent years in Auckland, there have been examples of concrete floors and/ or foundations that have been poured on dry, desiccated subgrades in summer months on expansive soils and have undergone heaving and cracking once the soil moisture contents have returned to higher levels. Foundation contractors need to be made aware of this issue and the need to maintain appropriate moisture contents in the footings and building platform subgrade between the time of excavation and the pouring of concrete.

Remedial actions that may be appropriate include platform protection with a hard fill layer, pouring of a blinding layer of concrete in footing bases and soaking of the building platform with sprinklers for an extended period.

Land owners need to be mindful of the B1/AS1 advice that the planting or removal of high water demand plants where their roots may extend close to footings (i.e. within a lateral distance of 1.5 times the mature tree height) can also cause settlement damage.

5.10. Topsoil Depths

Topsoil depths have been checked by the drilling of a borehole in the approximate centre of the building platform on each lot. The results are considered indicative for each lot, but may be subject to variations. Topsoil depths are between 100 and 300mm on these stages of the development.

Site specific findings are contained in the appended Suitability Statement Summary (Appendix A). However, it is possible that further levelling works have been undertaken since our investigations and accordingly, we strongly recommend that lot purchasers complete their own checks of topsoil depths.

6. CLOSURE

The appended Statement of Professional Opinion is provided to the Auckland Council and Cabra Developments Limited for their purposes alone on the express condition that it will not be relied upon by any other person. It is important that prospective purchasers satisfy themselves as to any specific conditions pertaining to their particular land interest.

Although regular site visits have been undertaken for observation, for providing guidance and instruction and for testing purposes, the geotechnical services scope did not include full time site presence. To this end, our appended Suitability Statement also relies on the Contractors' work practices and assumes that when we have not been present to observe the work, it has been completed to high standards and in accordance with the drawings, instructions and consent conditions provided to them.

Similarly it assumes that all as-built information and other details provided to the Client and/or CMW by other members of the project team are accurate and correct in all respects.

Appendix A: Statement of Professional Opinion as to the Suitability of Land for Building Development

STATEMENT OF PROFESSIONAL OPINION AS TO THE SUITABILITY OF LAND FOR BUILDING DEVELOPMENT

I, Richard Knowles, of CMW Geosciences (NZ) Limited, Auckland, hereby confirm that:

- As a Chartered Professional Engineer experienced in the field of geotechnical engineering, I am a Geoprofessional as defined in Section 1.2.2 of NZS 4404 and was retained by the Developer as the Geotechnical Engineer on Stage 1A of the Huapai Triangle Sub Precinct A (45 Station Road, Huapai) Development.
- 2. The extent of preliminary investigations carried out to date are described in:
- Geotechnical Report for Huapai Development Stage 1, Huapai, prepared by CMW Geosciences referenced 2015_1029AB Rev.0 dated 24 November 2014;
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced AKL2017_0089AB Rev.0 dated 12 July 2017;
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced AKL2018-0195AA Rev.0 dated 12 December 2018;

The conclusions and recommendations of these documents have been re-evaluated in the preparation of this report. The results of all tests carried out are also appended.

- 3. In my professional opinion, not to be construed as a guarantee, I consider that:
 - (a) The earth fills shown on the appended Cato Bolam Consultants Cut/Fill As-built Plans have been placed in compliance with NZS 4431, the Auckland Council Unitary Plans and related documents.
 - (b) Specific Design Zone (Retaining) areas have been applied on balance lot 2, together with lots 13 to 58 and 60 to 66 inclusive for the protection of the function of the retaining walls. The Keystone walls on this stage of the development were designed for a maximum of 12 kPa surcharge load and 0° toe slope. The cantilever timber pole walls were designed for a range of conditions including a maximum 12 kPa surcharge load, maximum 1 in 2.5 (V:H) backslope and maximum 5° toe slope, as appropriate. No building construction and no earthworks (i.e. cut or fills) should take place within the Specific Design Zone areas that exceed these design limits on the walls unless endorsed by a Chartered Professional Engineer experienced in geomechanics and familiar with the contents of this report who has considered the stability implications of the earthworks and/ or building proposals on the retaining walls.
 - (c) The function of the subsoil drains installed beneath balance lot 2 and lots 18, 27, 28, 41, 51 and 65 must not be impaired by any building development or landscaping works. Any bored or driven piles must be positioned to avoid damaging the draincoils. Where any subsoil drain is intercepted by building works, it must be reinstated under the direction of a Chartered Professional Engineer to ensure the integrity of the subsoil drainage system.
 - (d) A geotechnical ultimate bearing capacity of 300 kPa may be assumed for shallow foundation design on the building platforms of balance lot 2, together with lots 13 to 58 and 60 to 66 inclusive.

If for any reason higher geotechnical bearing capacities are required, further specific site investigation and design of foundations should be carried out prior to Building Consent application.

(e) The expansive site Class for all lots has been assessed using B1/AS1 as H (Highly) expansive with a characteristic surface movement (y_s) up to 78mm. We recommend that building designers

note on the Building Consent drawings the need to maintain appropriate moisture levels across building subgrades and in footing excavations (as described in section 5.9.3 of the Geotechnical Completion Report) for reference by foundation contractors.

(f) The backfilling and compaction of the storm water and sanitary sewer trenches on this subdivision has been carried out to appropriate standards having regard for the prevailing ground conditions and associated compaction induced pipe loadings.

However, no building development should take place within the 45 degree zone of influence of drain inverts unless endorsed by specific design and by construction inspections undertaken by a Chartered Professional Engineer experienced in geomechanics to ensure that lateral stability and differential settlement issues are addressed and that building loads are transferred beyond the influence of the pipe and trench backfill. A copy of drawing SW22 extracted from Chapter 4 of the Auckland Council Code of Practice for Land development and Subdivision this document is provided in Appendix B for clarification. Details for water and wastewater pipes are available in the Watercare COP1 - General Requirements and Procedures.

- (g) Subject to the geotechnical limitations, restrictions and recommendations contained in clauses 3(b), 3(c), 3(d), 3(e) and 3(f) above:
 - (i) The filled and natural ground is generally suitable for residential buildings constructed in accordance with NZS 3604 and the requirements of Acceptable Solution B1/AS1 of the New Zealand Building Code for the appropriate expansive soil class.
 - (ii) Where shallow foundations are appropriate, design may be carried out in accordance with B1/AS1 (Class H) or alternately, a specific foundation and structural design may be undertaken by a Chartered Professional Engineer.
- 4. Road subgrades have been formed with appropriate regard for slope stability and settlement risks.

The following table summarises the conditions on each of each residential lots.

For and on behalf of CMW Geosciences

Ry Knowles

Richard Knowles Principal Geotechnical Engineer, CPEng.

Condition	Specific Design Zone (retaining)	Subsoil Drains Present	Geotechnical Ultimate Bearing Capacity (kPa)	B1/AS1 Expansive Class	Service Lines Restrictions	Indicative Topsoil Depth (mm)
GCR SOPO Clause	3(b)	3(c)	3(d)	3(e)	3(f)	
Lot number						
2	•	•	300	Н	•	200
13	•		300	Н	•	150
14	•		300	Н	•	200
15	•		300	Н	•	150
16	•		300	Н	•	200
17	•		300	Н	•	200
18	•	•	300	Н	•	200
19	•		300	Н	•	200
20	•		300	Н	•	200
21	•		300	Н	•	100
22	•		300	Н	•	150
23	•		300	Н	•	200
24	•		300	Н	•	150
25	•		300	Н	•	200
26	•		300	Н	•	150
27	•	•	300	Н	•	150
28	•	•	300	Н	•	150
29	•		300	Н	•	150
30	•		300	Н	•	200

Condition	Specific Design Zone (retaining)	Subsoil Drains Present	Geotechnical Ultimate Bearing Capacity (kPa)	B1/AS1 Expansive Class	Service Lines Restrictions	Indicative Topsoil Depth (mm)
GCR SOPO Clause	3(b)	3(c)	3(d)	3(e)	3(f)	
31	•		300	Н	•	100
32	•		300	Н	•	100
33	•		300	Н	•	200
34	•		300	Н	•	200
35	•		300	Н	٠	200
36	•		300	Н	٠	150
37	•		300	Н	•	250
38	•		300	Н	•	200
39	•		300	Н	•	200
40	•		300	Н	•	150
41	•	•	300	Н	•	200
42	•		300	Н	•	200
43	•		300	Н	•	150
44	•		300	Н	•	150
45	•		300	Н	•	200
46	•		300	н	•	250
47	•		300	н	•	200
48	•		300	Н	•	100
49	•		300	Н	•	100

Condition	Specific Design Zone (retaining)	Subsoil Drains Present	Geotechnical Ultimate Bearing Capacity (kPa)	AS2870 Expansive Class	Service Lines Restrictions	Indicative Topsoil Depth (mm)
GCR SOPO Clause	3(c)	3(d)	3(f)	3(g)	3(h)	
Lot number						
50	•		300	Н	•	200
51	٠	•	300	Н	•	100
52	٠		300	Н	•	150
53	•		300	Н	•	100
54	•		300	Н	•	100
55	•		300	Н	•	100
56	•		300	Н	•	150
57	•		300	Н	•	100
58	•		300	Н	•	150
60	٠		300	Н	•	150
61	•		300	Н	•	250
62	•		300	Н	•	200
63	•		300	Н	•	250
64	•		300	Н	•	300
65	•	•	300	Н	•	250
66	•		300	Н	●	250

Appendix B: Drawings

Title	Reference No.	Date	Revision
Cato Bolam Consultants Cut/Fill Asbuilt Plans –	42356-DR-SU-9007	February	0
Stage 1A	to 9011	2020	
Cato Bolam Consultants Final Contours and Retaining Walls Asbuilt Plans – Stage 1A	42356-DR-SU-9012 to 9016	February 2020	0
Cato Bolam Consultants Retaining Wall Specific	42356-DR-SU-9017	February	0
Design Asbuilt Plans – Stage 1A	to 9021	2020	
Cato Bolam Consultants Roading Asbuilt Plans	42356-DR-SU-9100	February	0
– Stage 1A	to 9103	2020	
Cato Bolam Consultants Wastewater Asbuilt	42356-DR-SU-9200	February	0
Plans – Stage 1A	to 9203	2020	
Cato Bolam Consultants Stormwater Asbuilt	42356-DR-SU-9300	February	1
Plans – Stage 1A	to 9306	2020	
Cato Bolam Consultants Water Supply Asbuilt	42356-DR-SU-9400	February	0
Plans – Stage 1A	to 9403	2020	
Auckland Council Stormwater Pipe and Manhole Construction Clearance Requirements	ACSD SW22	September 2013	1

Cabra Developments Ltd - 45 Station Road, Huapai - Stage 1A

Asbuilt Plans



LOCATION DIAGRAM Scale 1:10,000

Plan No	Rev	Plan Title	Plan No	Rev	Plan
		Earthworks	9102	0	Roa
9007	0	Cut/Fill Asbuilt Plan - Stage 1A (Overall)	9103	0	Road
9008	0	Cut/Fill Asbuilt Plan - Stage 1A (Sheet 1 of 4)			Wa
9009	0	Cut/Fill Asbuilt Plan - Stage 1A (Sheet 2 of 4)	9200	0	Was
9010	0	Cut/Fill Asbuilt Plan - Stage 1A (Sheet 3 of 4)	9201	0	Was
9011	0	Cut/Fill Asbuilt Plan - Stage 1A (Sheet 4 of 4)	9202	0	Was
9012	0	Final Contours and Retaining Walls - Asbuilt- Stage 1A (Overall)	9203	0	Was
9013	0	Final Contours and Retaining Walls - Asbuilt- Stage 1A (Sheet 1 of 4)	9300	1	Stor
9014	0	Final Contours and Retaining Walls - Asbuilt- Stage 1A	9301	1	Stor
		(Sheet 2 of 4)	9302	1	:
9015	0	Final Contours and Retaining Walls - Asbuilt- Stage 1A (Sheet 3 of 4)	9303	1	Stor
9016	0	Final Contours and Retaining Walls - Asbuilt- Stage 1A	9304	1	Stor
0010	Ŭ	(Sheet 4 of 4)	9305	1	Stor
9017	0	Retaining Wall Specific Design - Asbuilt Plan - Stage 1A (Overall)	9306	0	Stori (She
9018	0	Retaining Wall Specific Design - Asbuilt Plan - Stage 1A			
		(Sheet 1 of 4)	9400	0	Wate
9019	0	Retaining Wall Specific Design - Asbuilt Plan - Stage 1A (Sheet 2 of 4)	9401	0	Wate
9020	0	Retaining Wall Specific Design - Asbuilt Plan - Stage 1A	9402	0	Wate
0020	Ū	(Sheet 3 of 4)	9403	0	Wate
9021	0	Retaining Wall Specific Design - Asbuilt Plan - Stage 1A (Sheet 4 of 4)			
		Roading			
9100	0	Roading Asbuilt Plan- Stage 1A (Overall)			









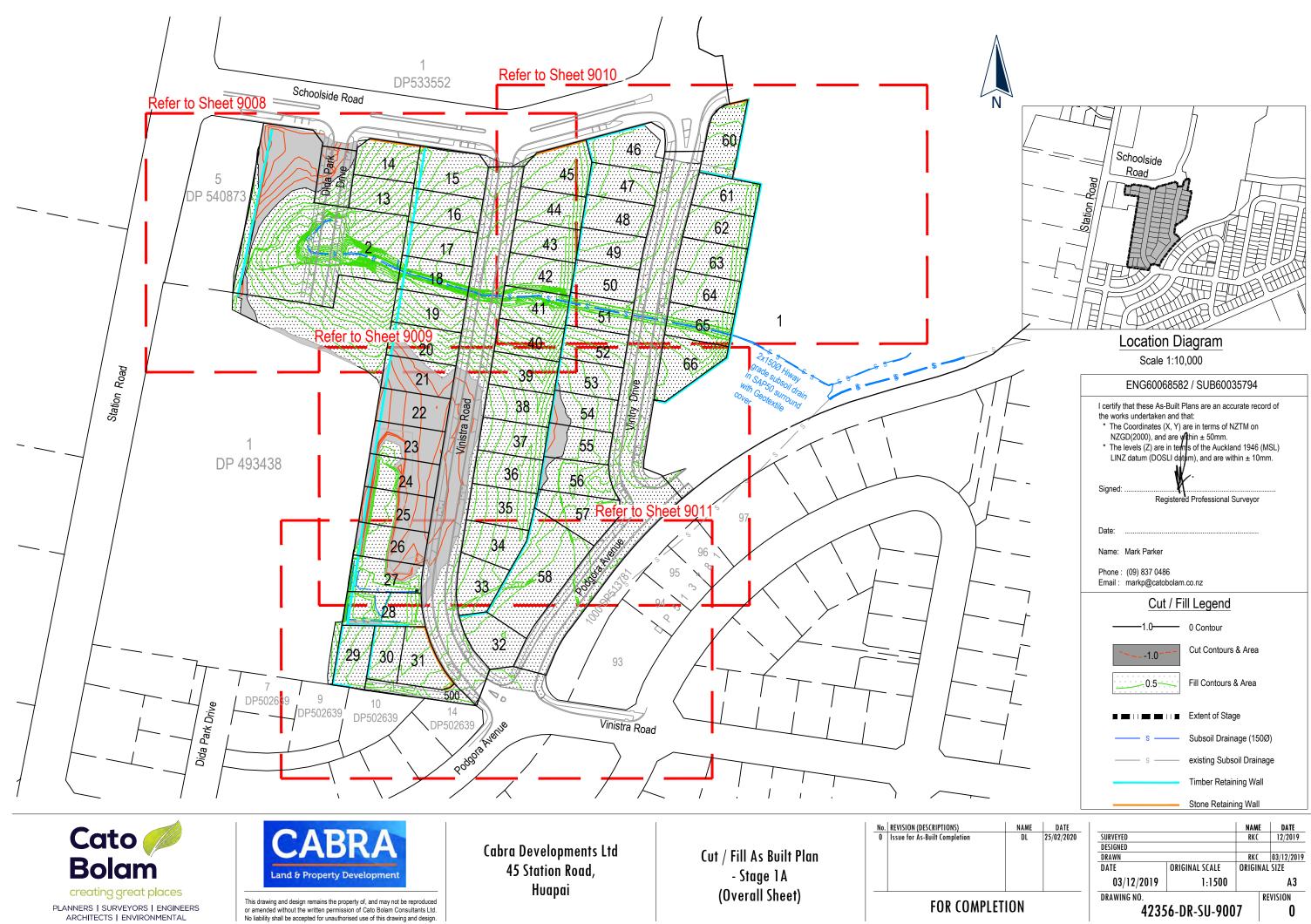
PLANNERS | SURVEYORS | ENGINEERS ARCHITECTS | ENVIRONMENTAL

an Title ading Asbuilt Plan- Stage 1A (sheet 2 of 3) ading Asbuilt Plan- Stage 1A (sheet 3 of 3) 'astewater astewater Asbuilt Plan- Stage 1A (Overall) astewater Asbuilt Plan- Stage 1A (Sheet 1 of 3) stewater Asbuilt Plan- Stage 1A (Sheet 2 of 3) astewater Asbuilt Plan- Stage 1A (Sheet 3 of 3) Stormwater ormwater Asbuilt Plan- Stage 1A (Overall) ormwater Asbuilt Plan- Stage 1A (Sheet 1 of 6) Stormwater Asbuilt Plan- Stage 1A (Sheet 2 of 6) ormwater Asbuilt Plan- Stage 1A (Sheet 3 of 6) ormwater Asbuilt Plan- Stage 1A (Sheet 4 of 6) ormwater Asbuilt Plan- Stage 1A (Sheet 5 of 6) ormwater Zone of Influence Asbuilt Plan- Stage 1A neet 6 of 6)

Water

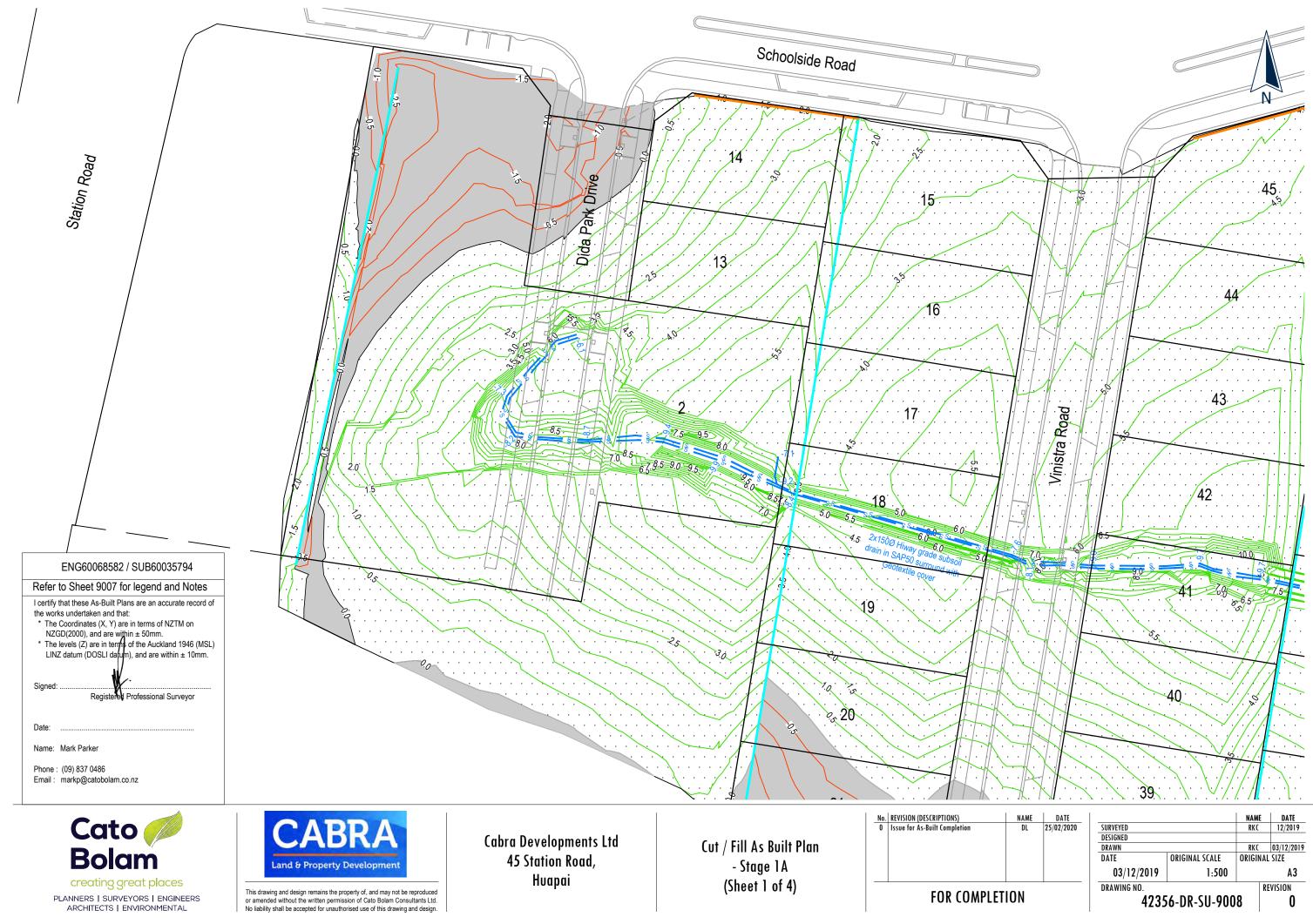
ater Supply Asbuilt Plan- Stage 1A (Overall) ter Supply Asbuilt Plan- Stage 1A (sheet 1 of 3) ater Supply Asbuilt Plan- Stage 1A (sheet 2 of 3) ter Supply Asbuilt Plan- Stage 1A (sheet 3 of 3)

DRAWING NO.	4235	6-DR-SU-00	001
DATE	Feb 2020	ORIGINAL SIZE	A3

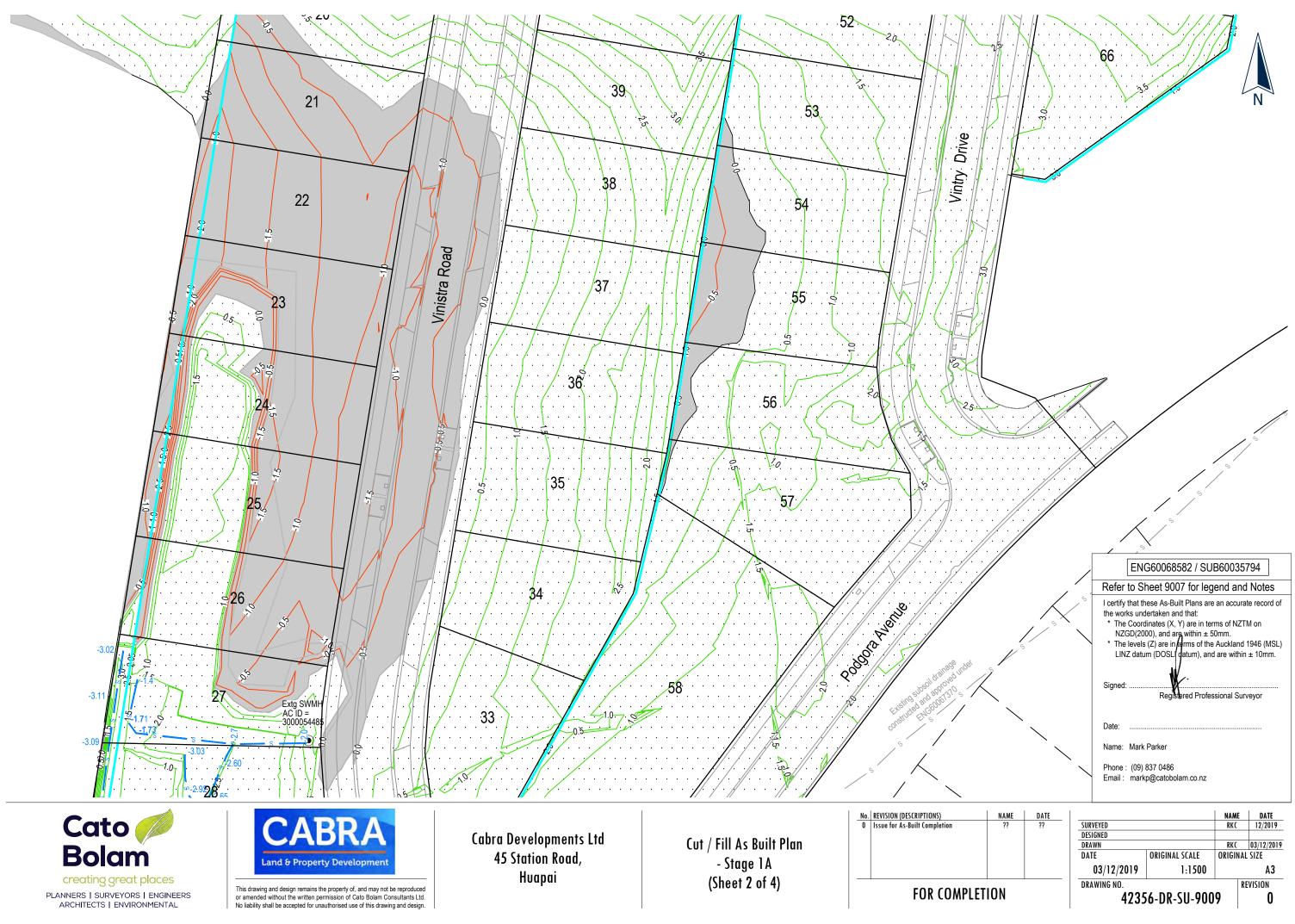




















Cabra Developments Ltd 45 Station Road, Huapai

Cut / Fill As Built Plan - Stage 1A (Sheet 3 of 4)

No. REVISION (DESCRIPTIONS) NAM 0 Issue for As-Built Completion DL FOR COMPLETION FOR COMPLETION



LINZ datum (DOSLI datum), and are within Signed:						
i intervels (Z) are intervels of the Auckland	NZGD(2000), and are within ± 50mm. * The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm. Signed:Registered Professional Surveyor					
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	ENG60068582 / SUB60035794 Refer to Sheet 9007 for legend and Notes					

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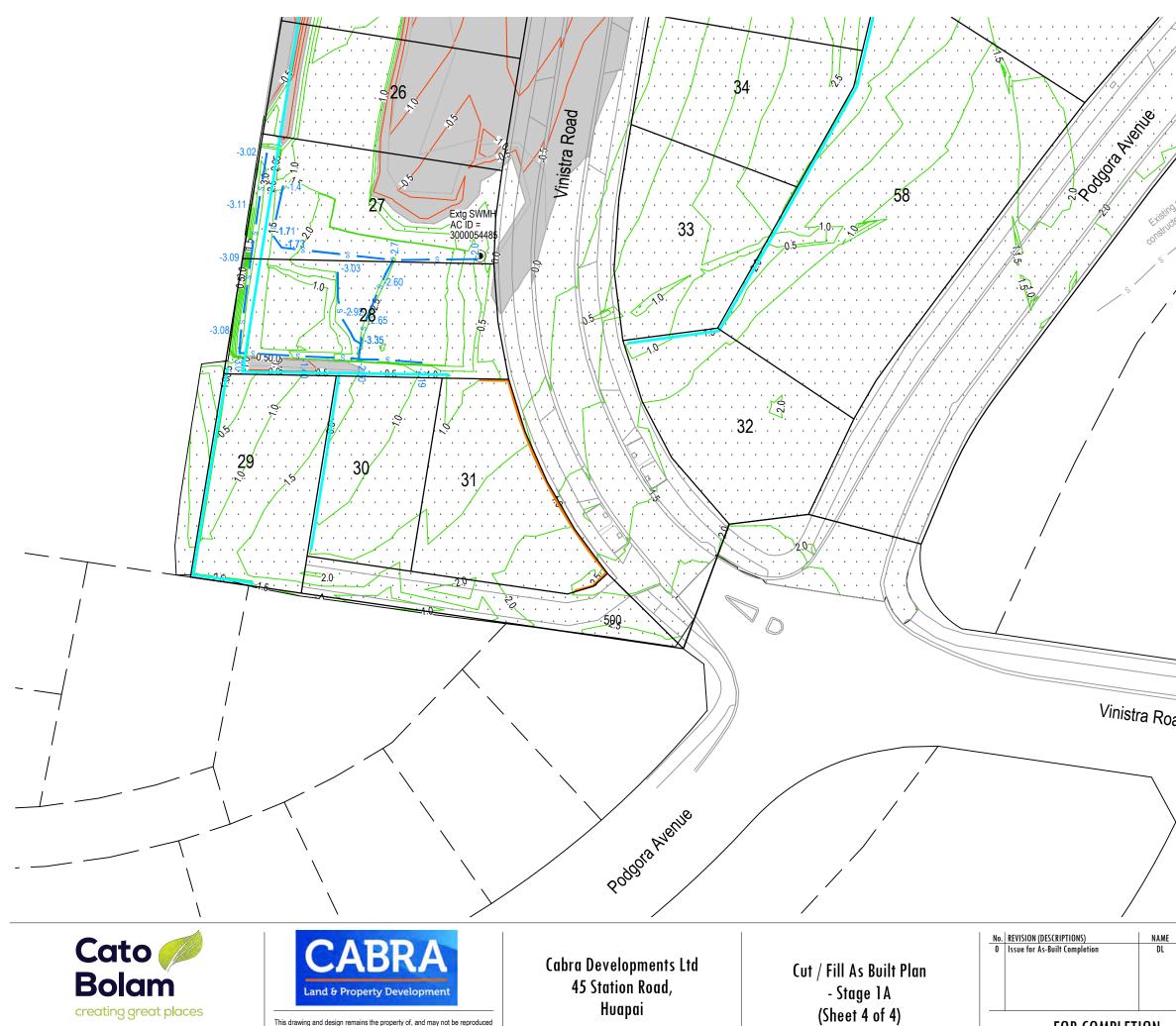
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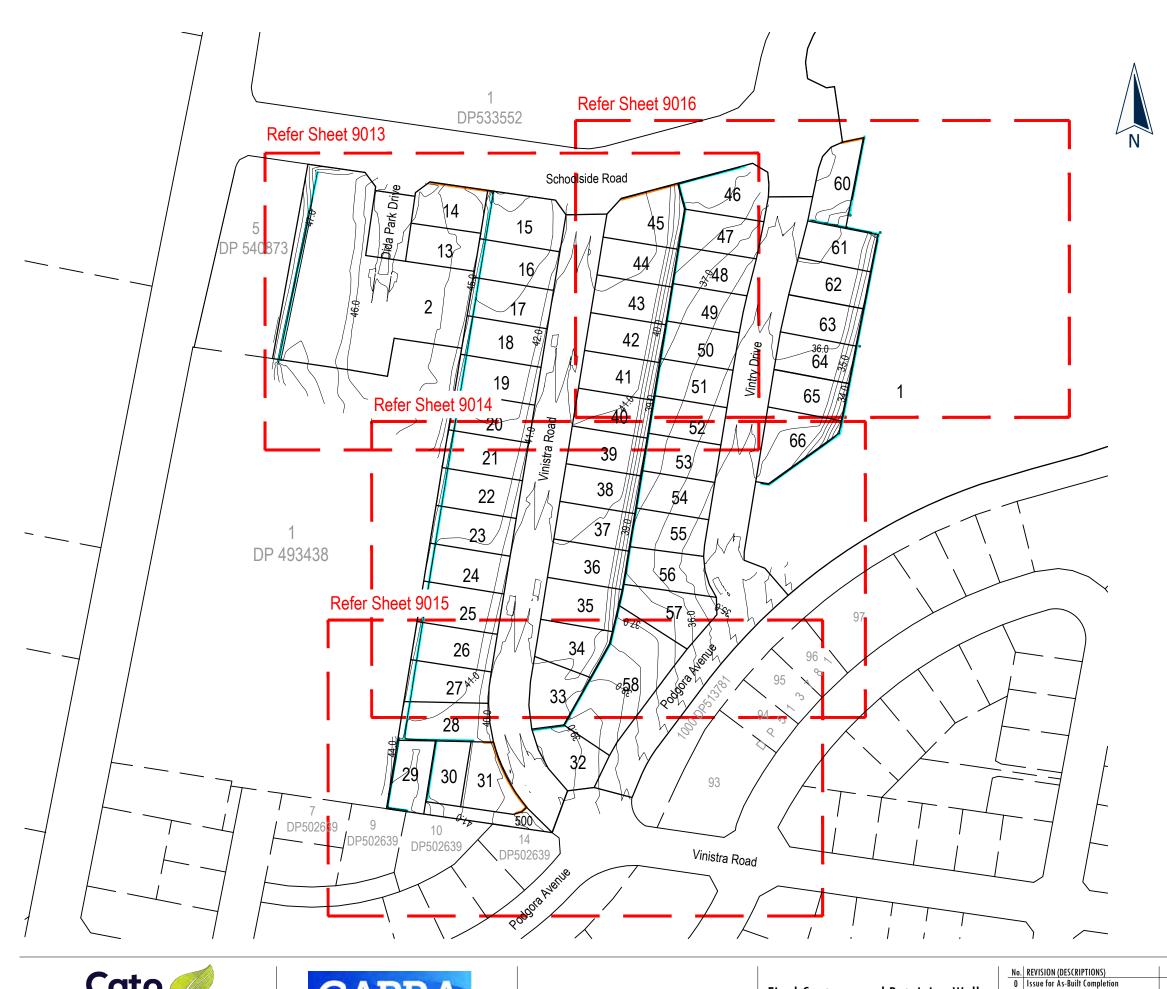
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	Signed:	Registered Profes	sional Sur	veyor
/	Date:			
/	Name: Mark	Parker		
	Phone : (09) Email : mark	837 0486 p@catobolam.co.nz		
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Cabra Developments Ltd 45 Station Road, Huapai Final Contours and Retaining Walls As Built Plan - Stage 1A (Overall Sheet)



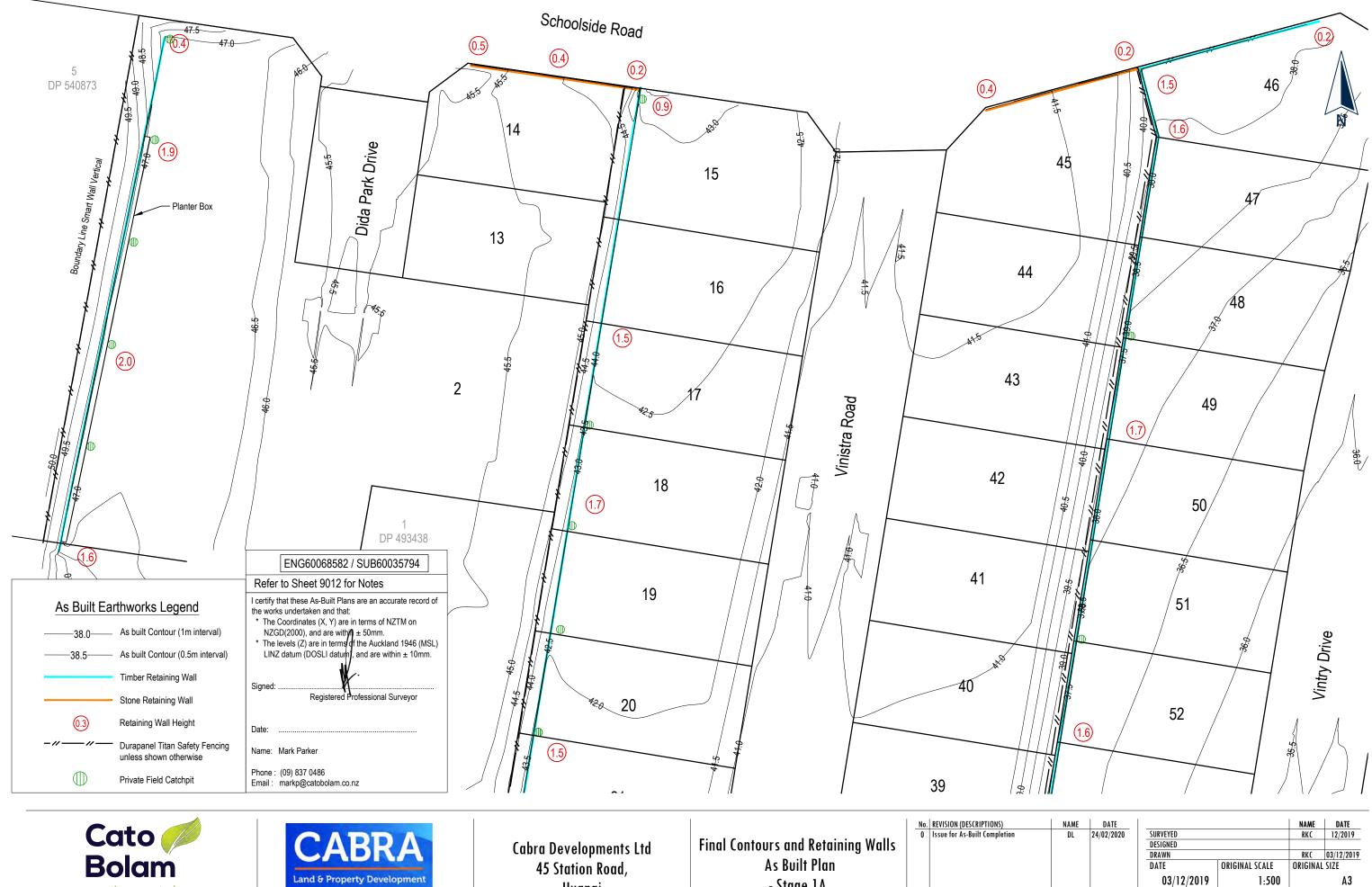
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Schoolside Road	
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Location Diagram	

Scale 1:10,000

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Date:					
Name: Mark Parker					
Phone:(09) 837 0486 Email: markp@catobolam	.co.nz				
As Built Earthworks Legend					
	As built Contour (1m interval)				
	As built Contour (0.5m interval)				
	Timber Retaining Wall				
	Stone Retaining Wall				
0.3	Retaining Wall Height				
-////	Durapanel Titan Safety Fencing unless shown otherwise				
\bigcirc	Private Field Catchpit				

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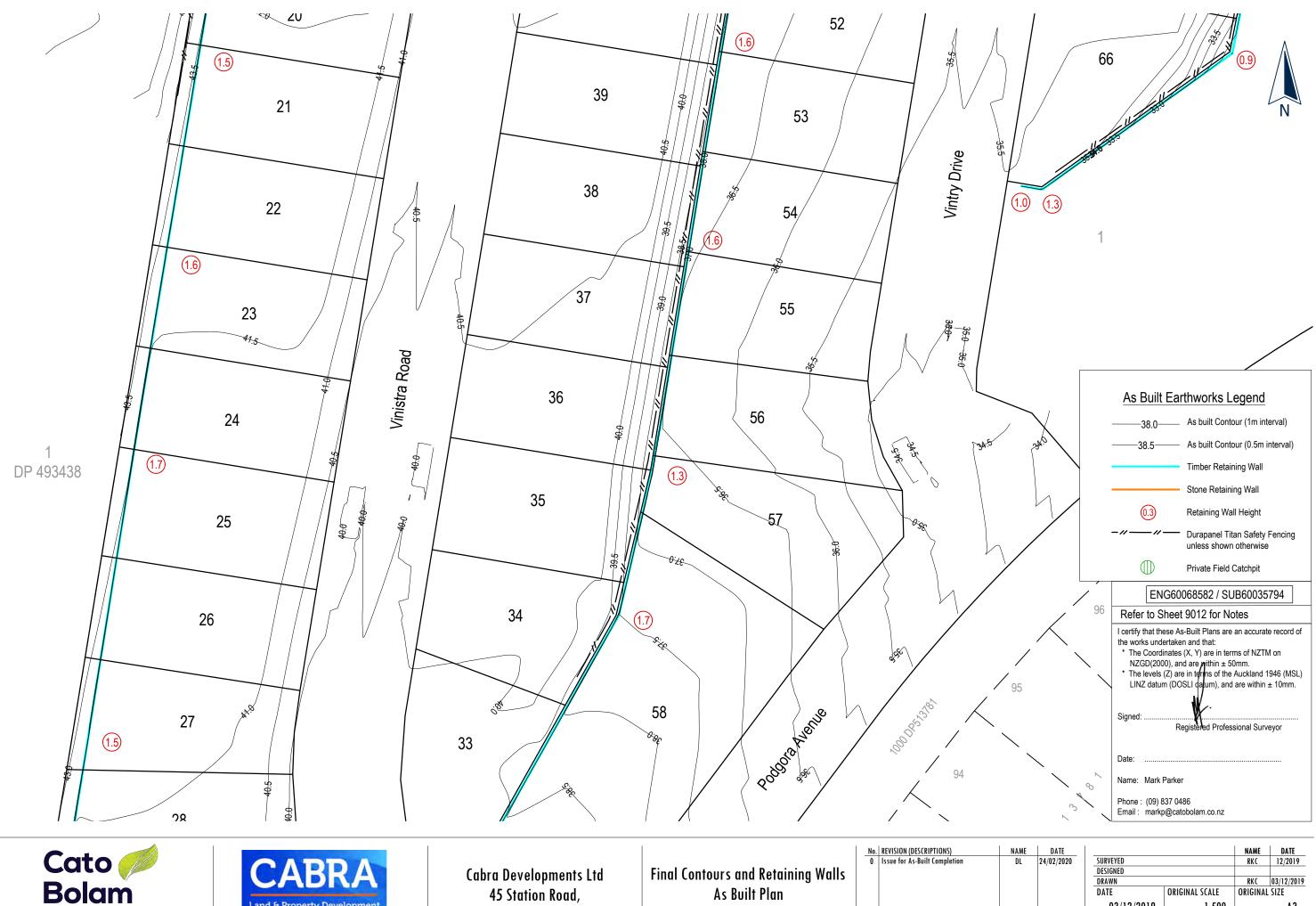
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Huapai

- Stage 1A (Sheet 1 of 4)

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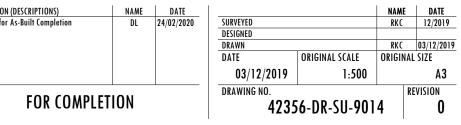
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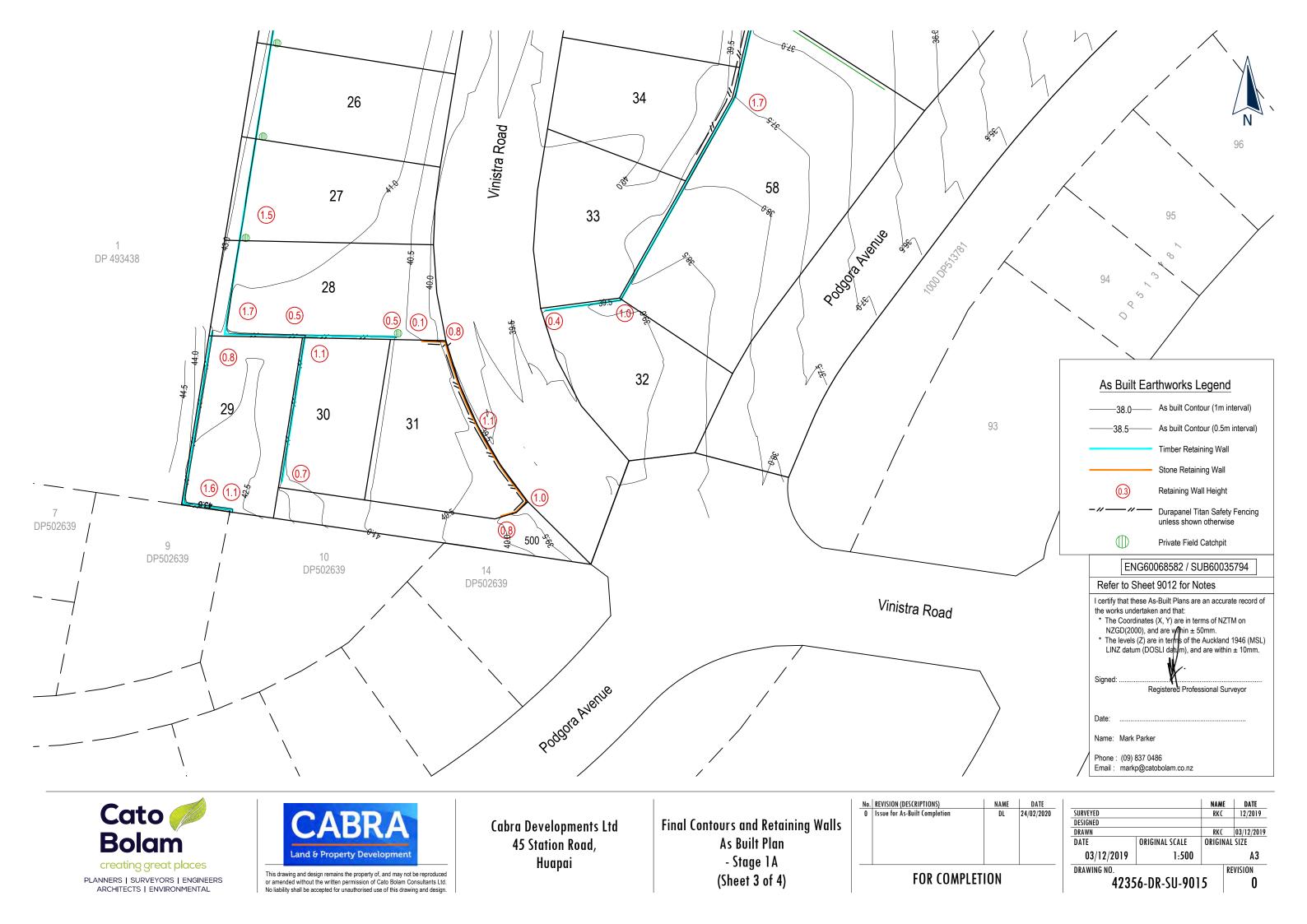


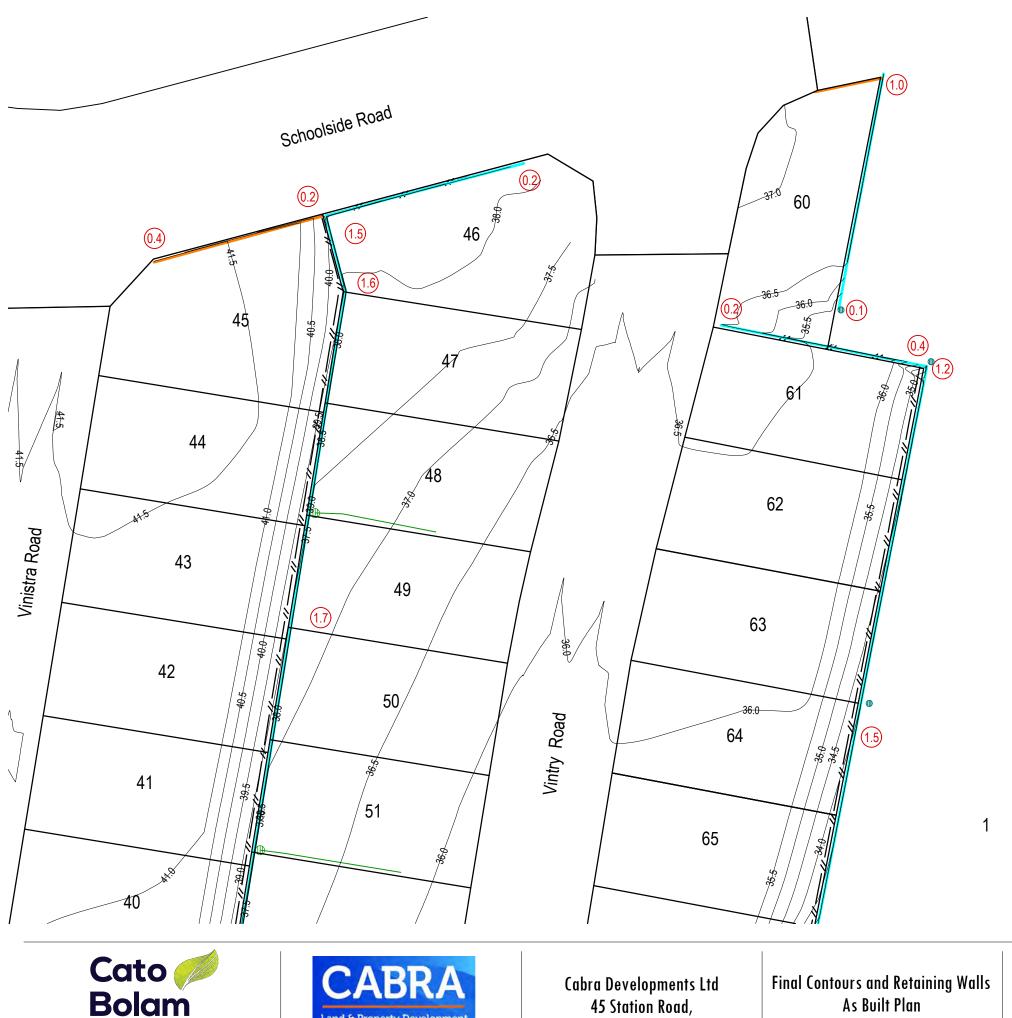
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Huapai

As Built Plan - Stage 1A (Sheet 2 of 4)







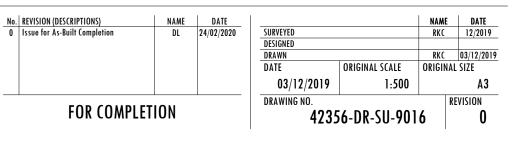
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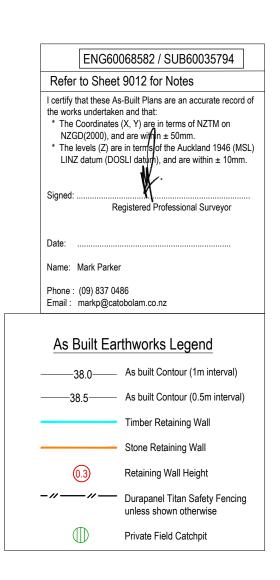
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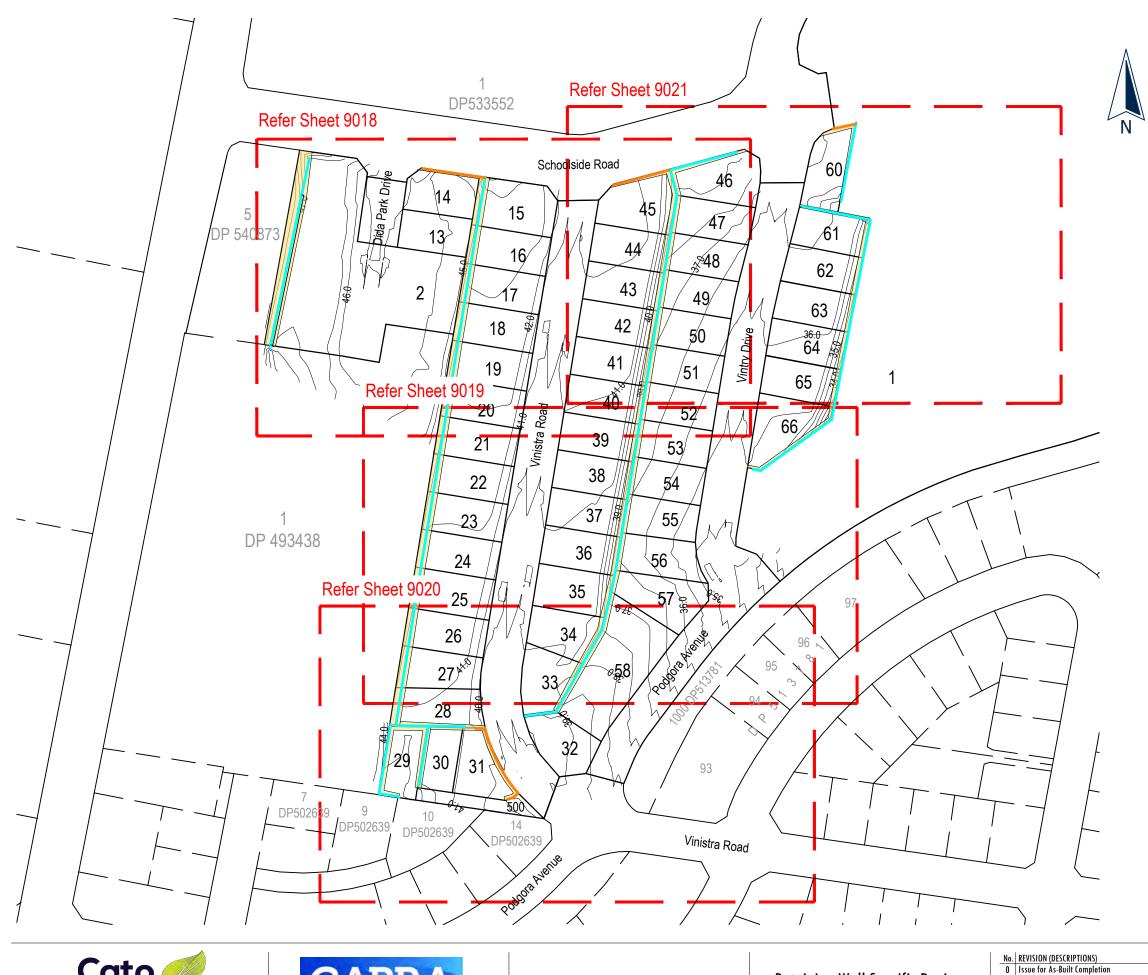
45 Station Road, Huapai

As Built Plan - Stage 1A (Sheet 4 of 4)













Cabra Developments Ltd 45 Station Road, Huapai Retaining Wall Specific Design As Built Plan - Stage 1A (Overall Sheet)

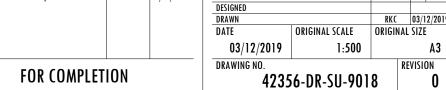


Schoolside Road Boad Location Diagram Scale 1:10,000	
ENG60068582 / SUB60035794 I certify that these As-Built Plans are an accurate record of the works undertaken and that: * The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within ± 50mm. * The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm. Signed:	
As Built Earthworks Legend	5

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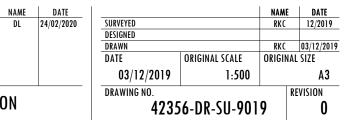


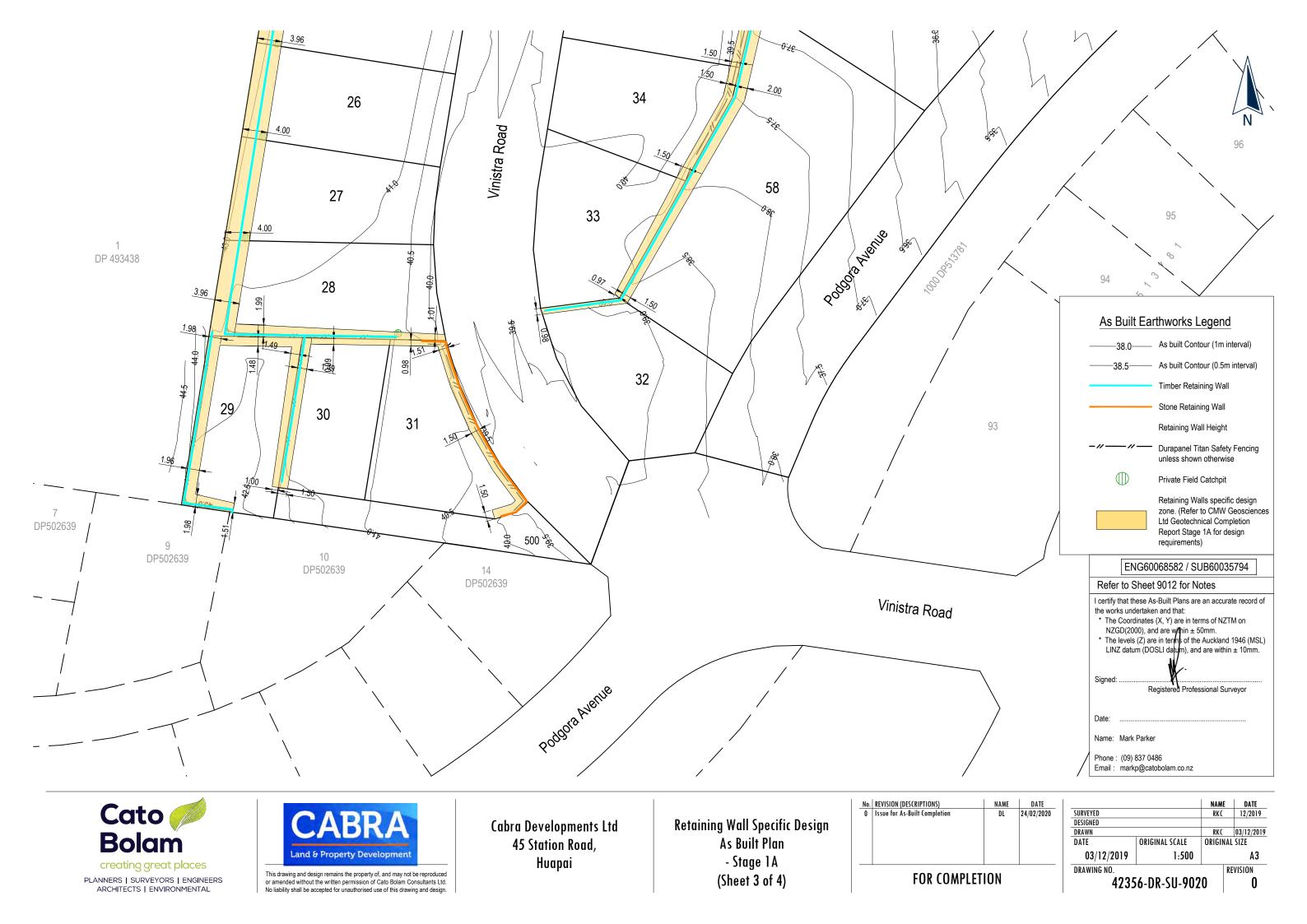
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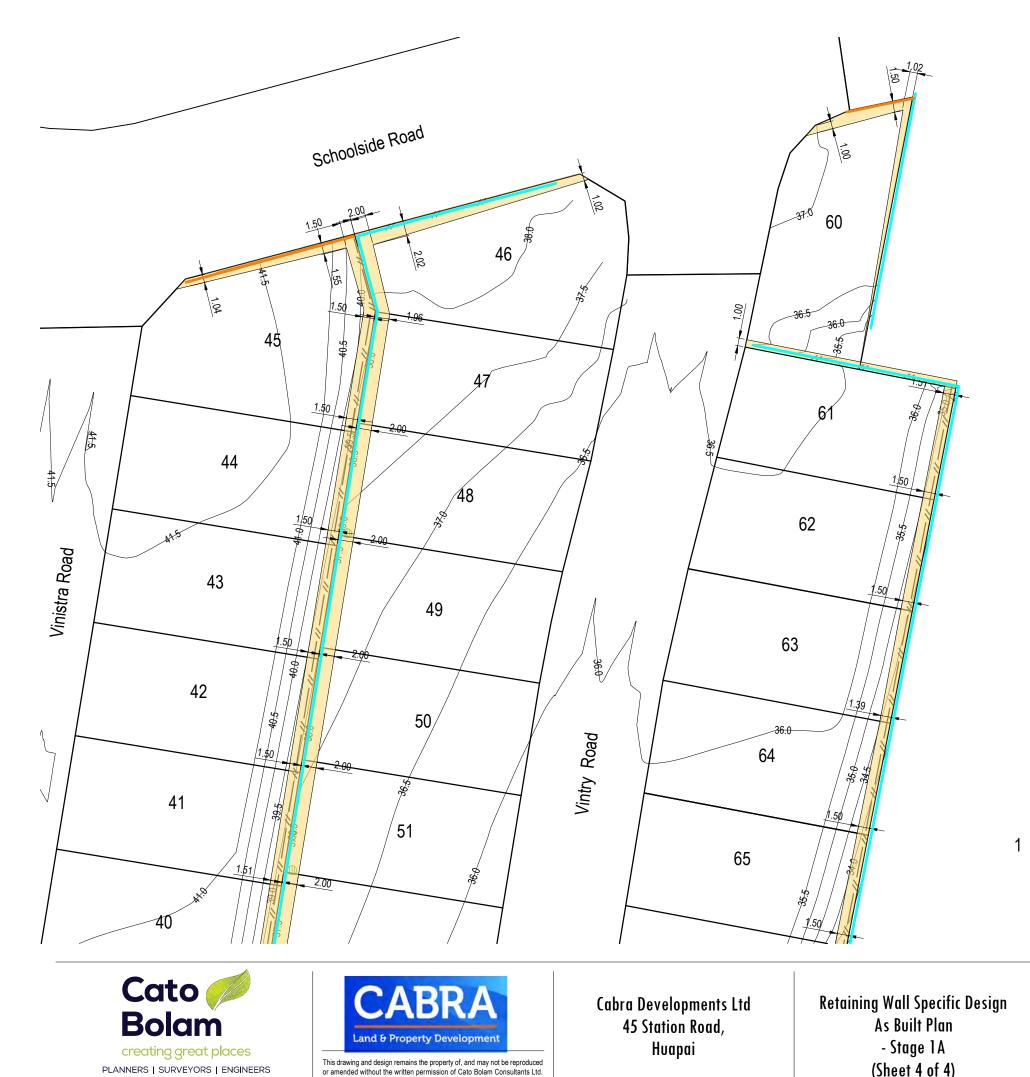
Huapai

Retaining Wall Specific Design As Built Plan - Stage 1A (Sheet 2 of 4)

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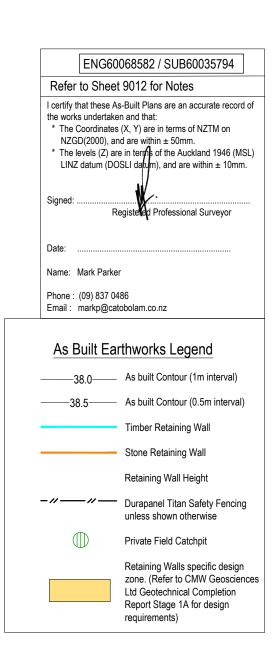
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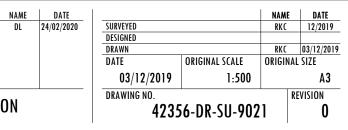
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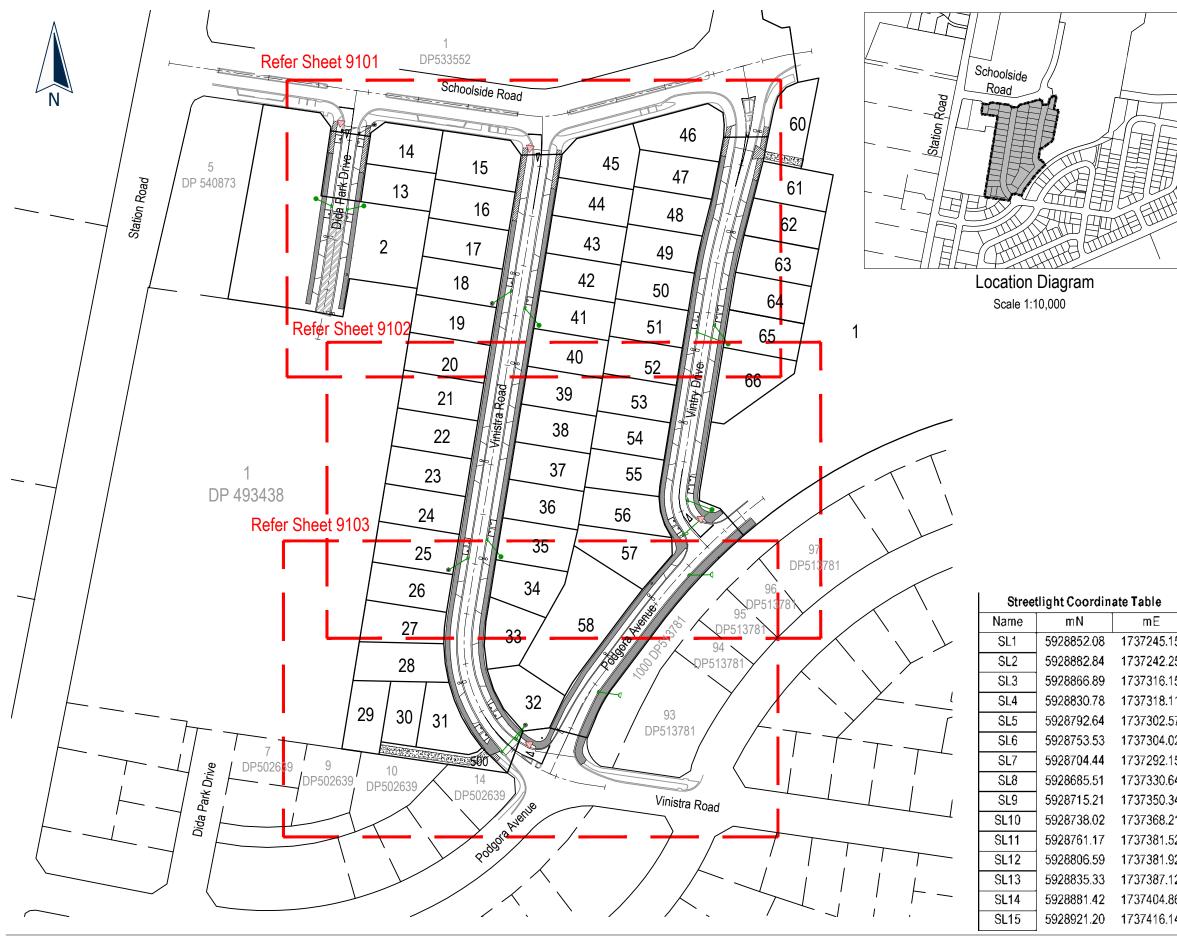
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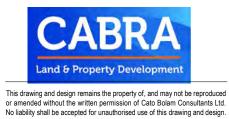












Cabra Developments Ltd 45 Station Road, Huapai

Roading As Built Plan - Stage 1A (Overall Plan)

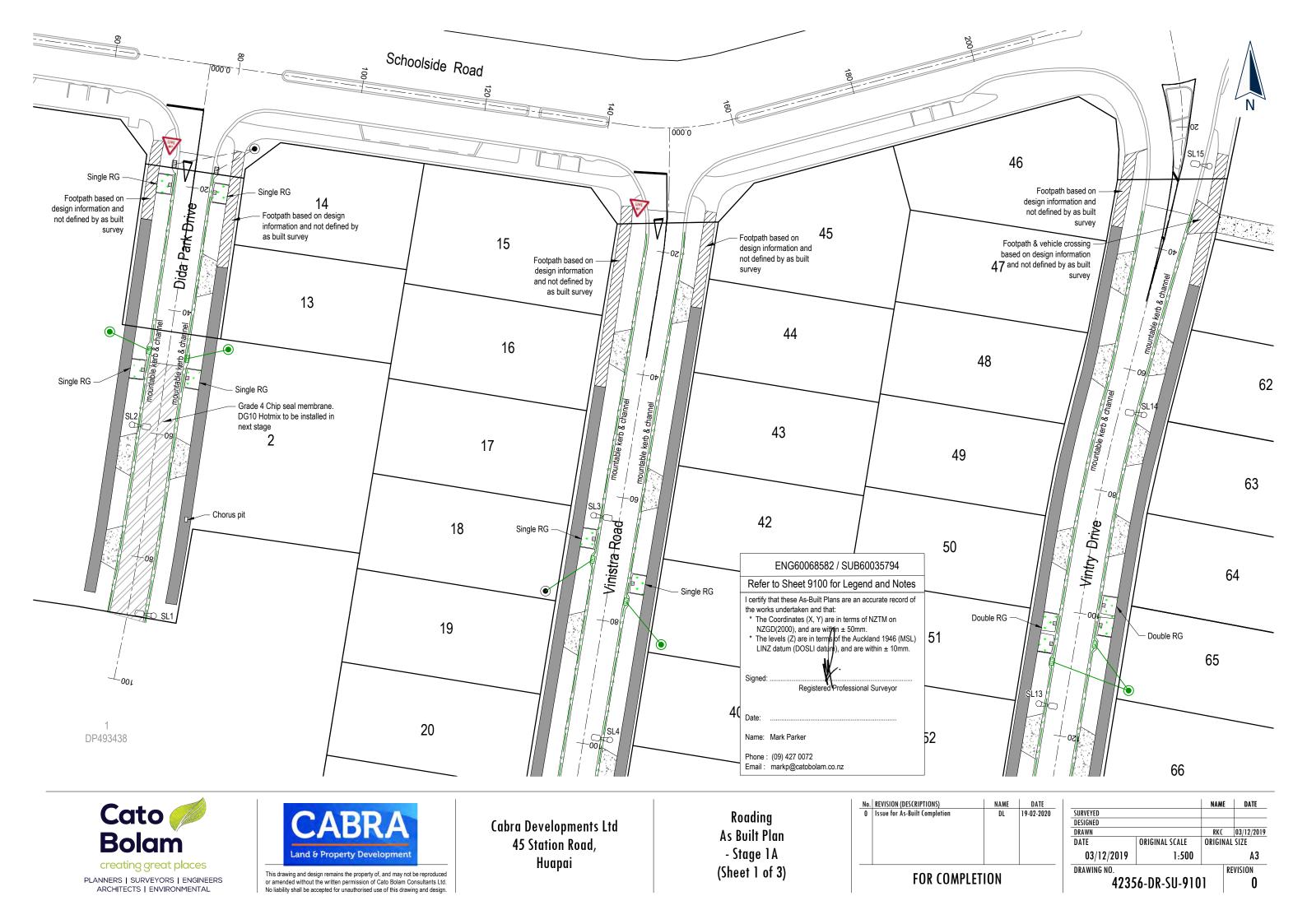
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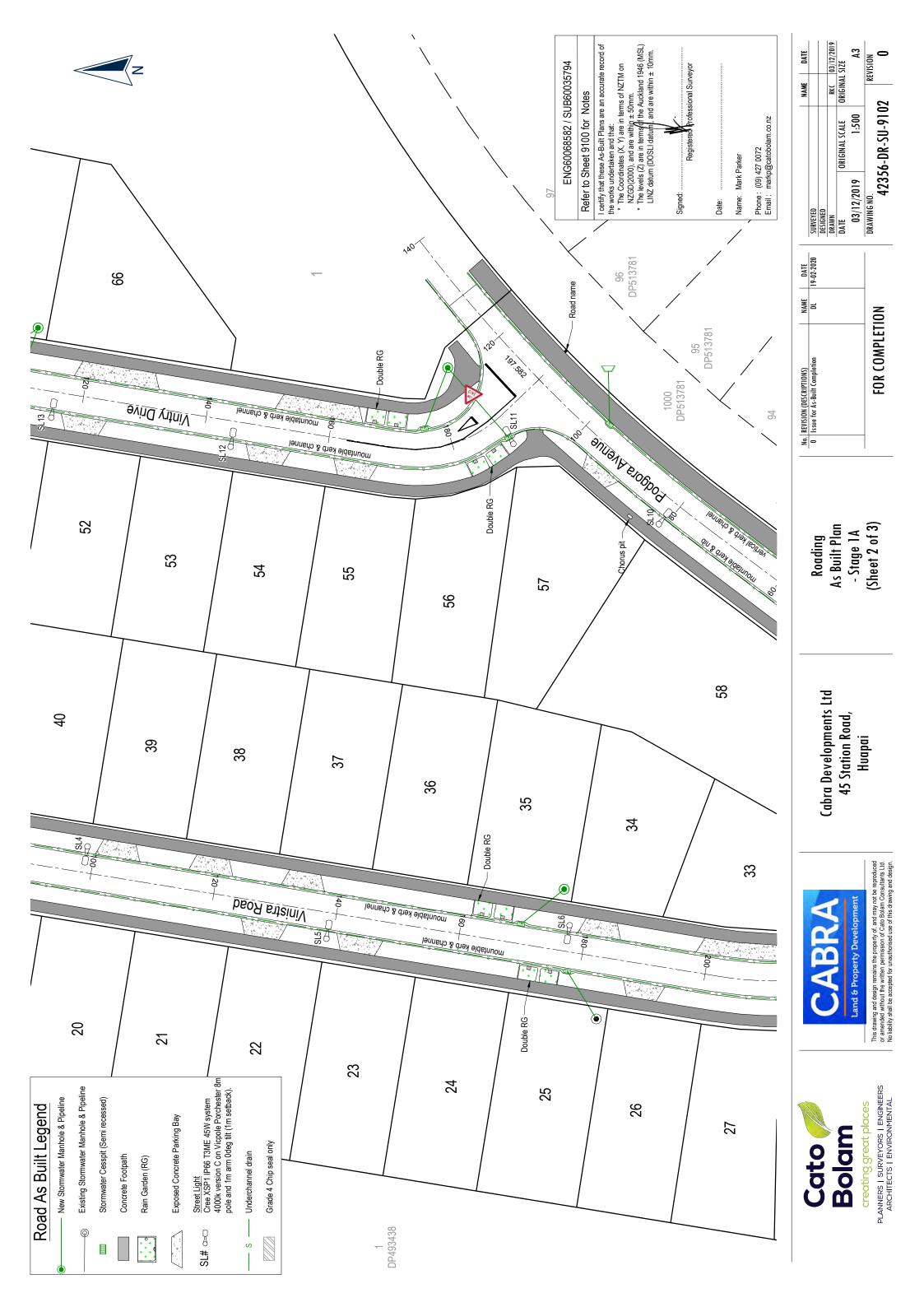
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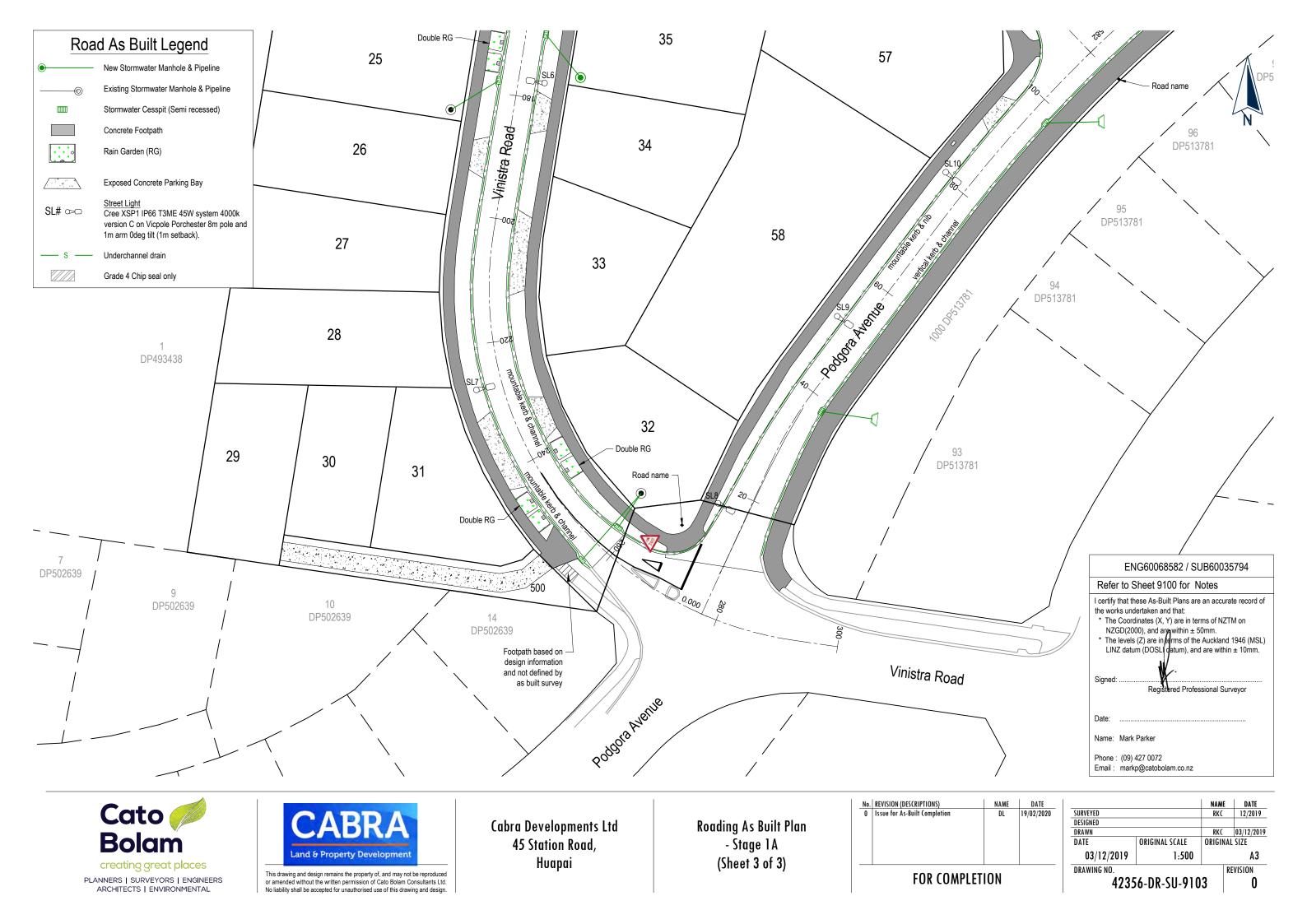
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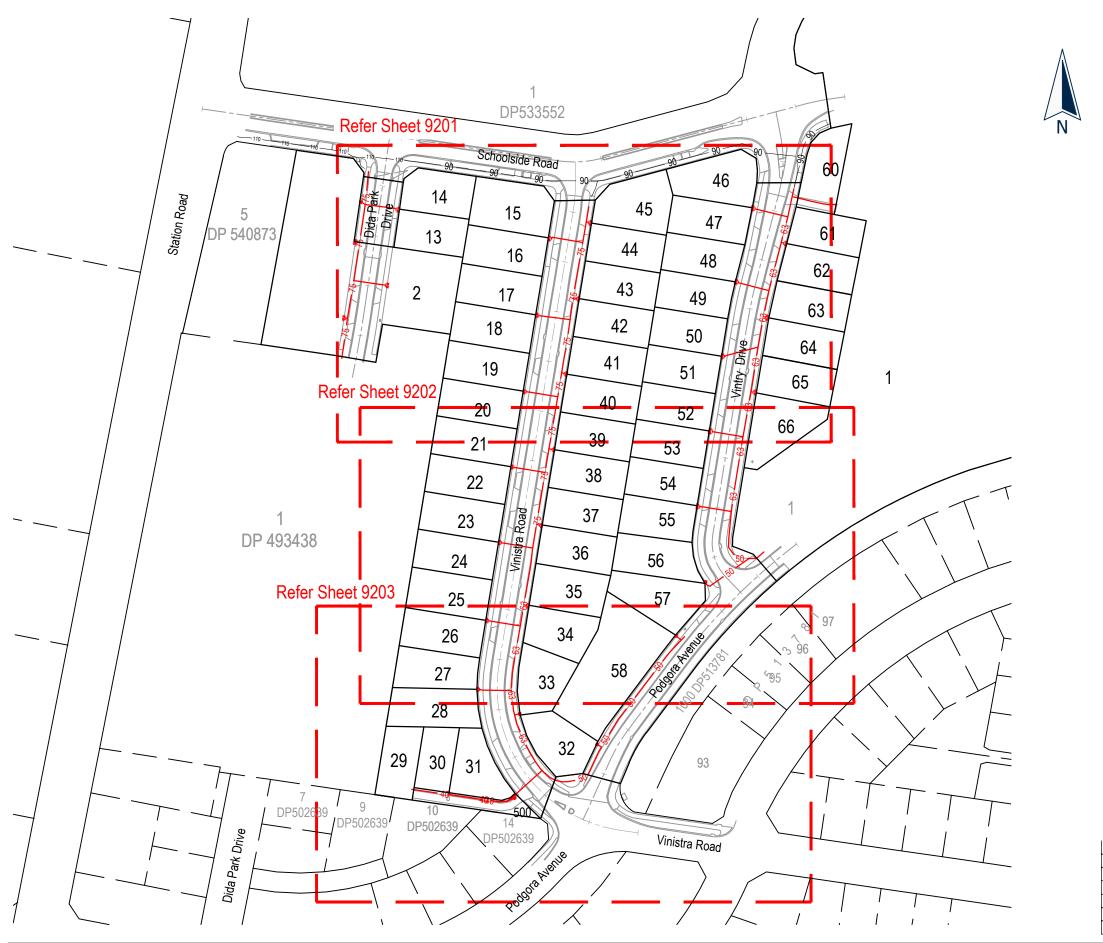
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Cabra Developments Ltd 45 Station Road, Huapai

Wastewater As Built Plan - Stage 1A (Overall Sheet)

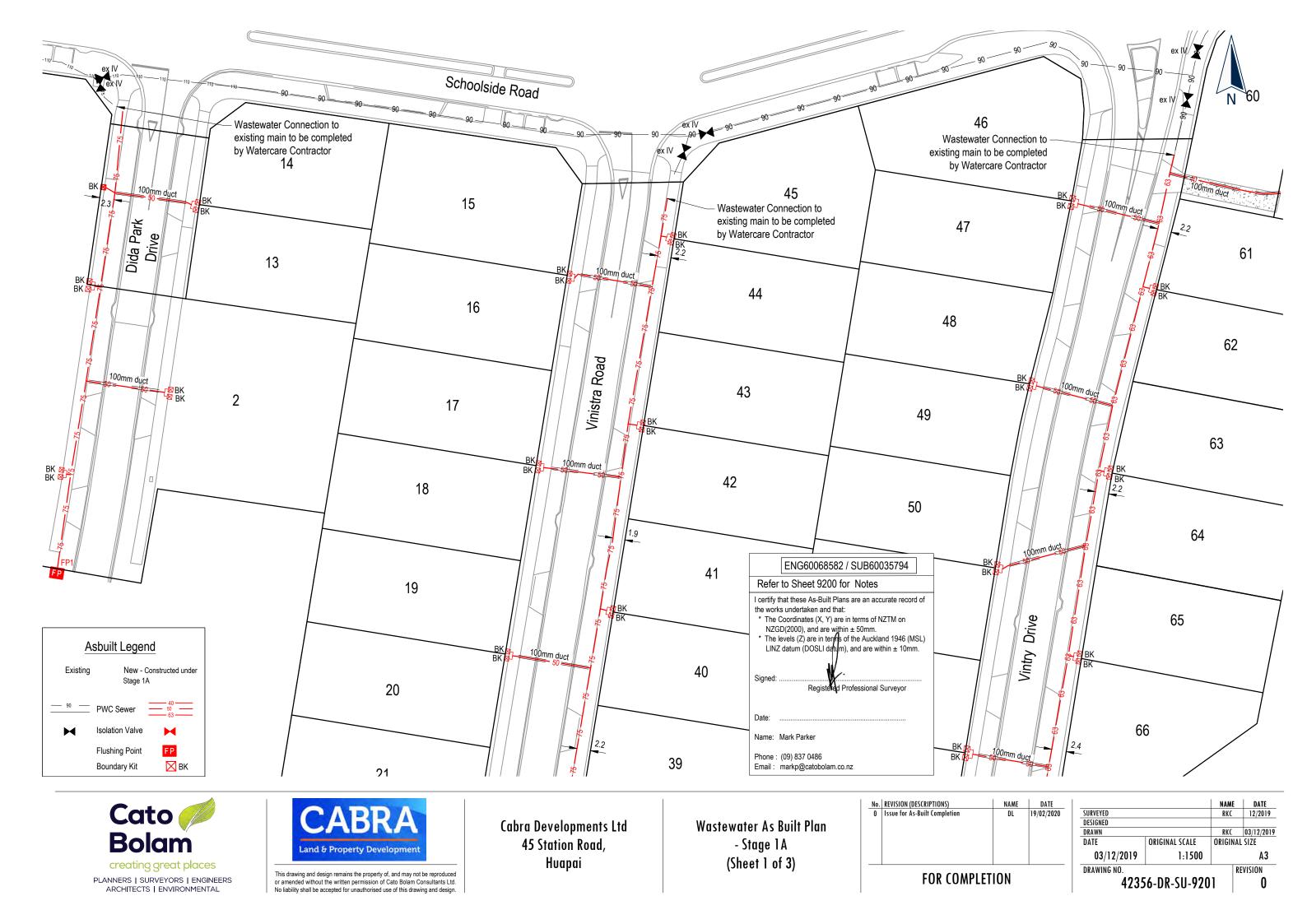


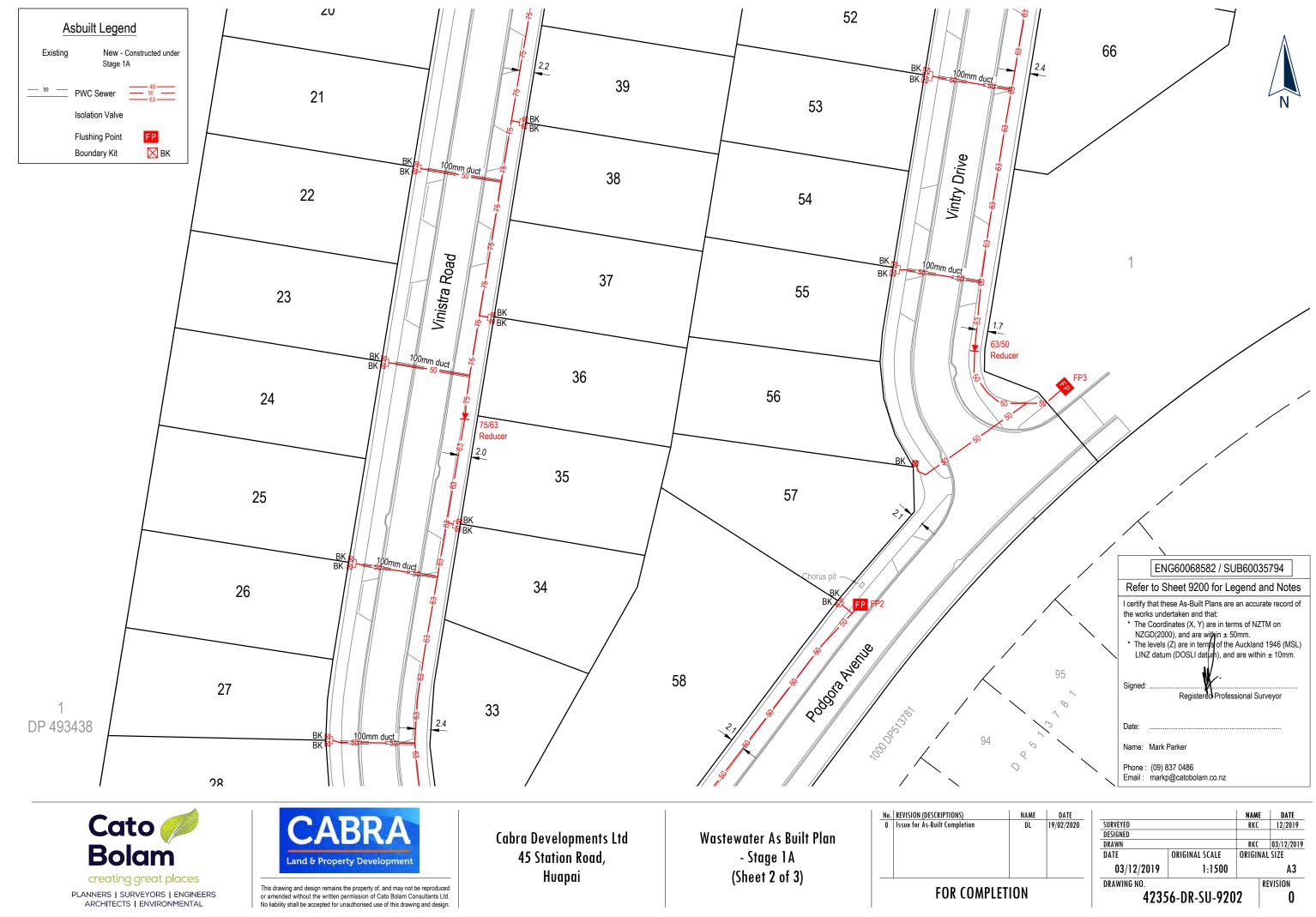
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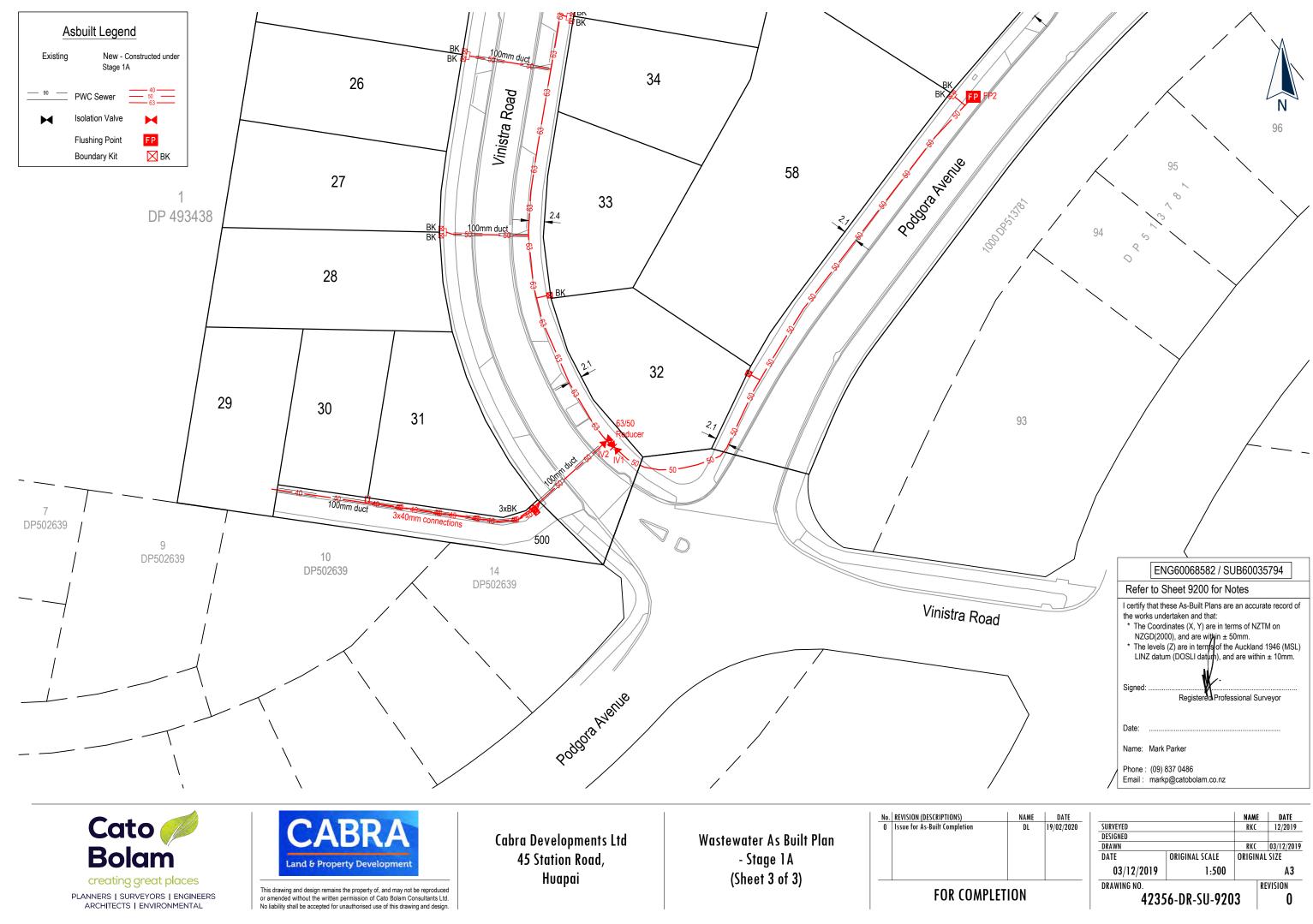
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	WAS		RETICULATION	-	
		ERAL	are in terms of NZT	M 2000.	
	NOT	ES			
	Email :	markp@ca	tobolam.co.nz		
		: (09) 837 04			
	Name:	Mark Parke	ır		
	Date:				
		R	egistered Profession	al Surveyor	
	Signed	:			
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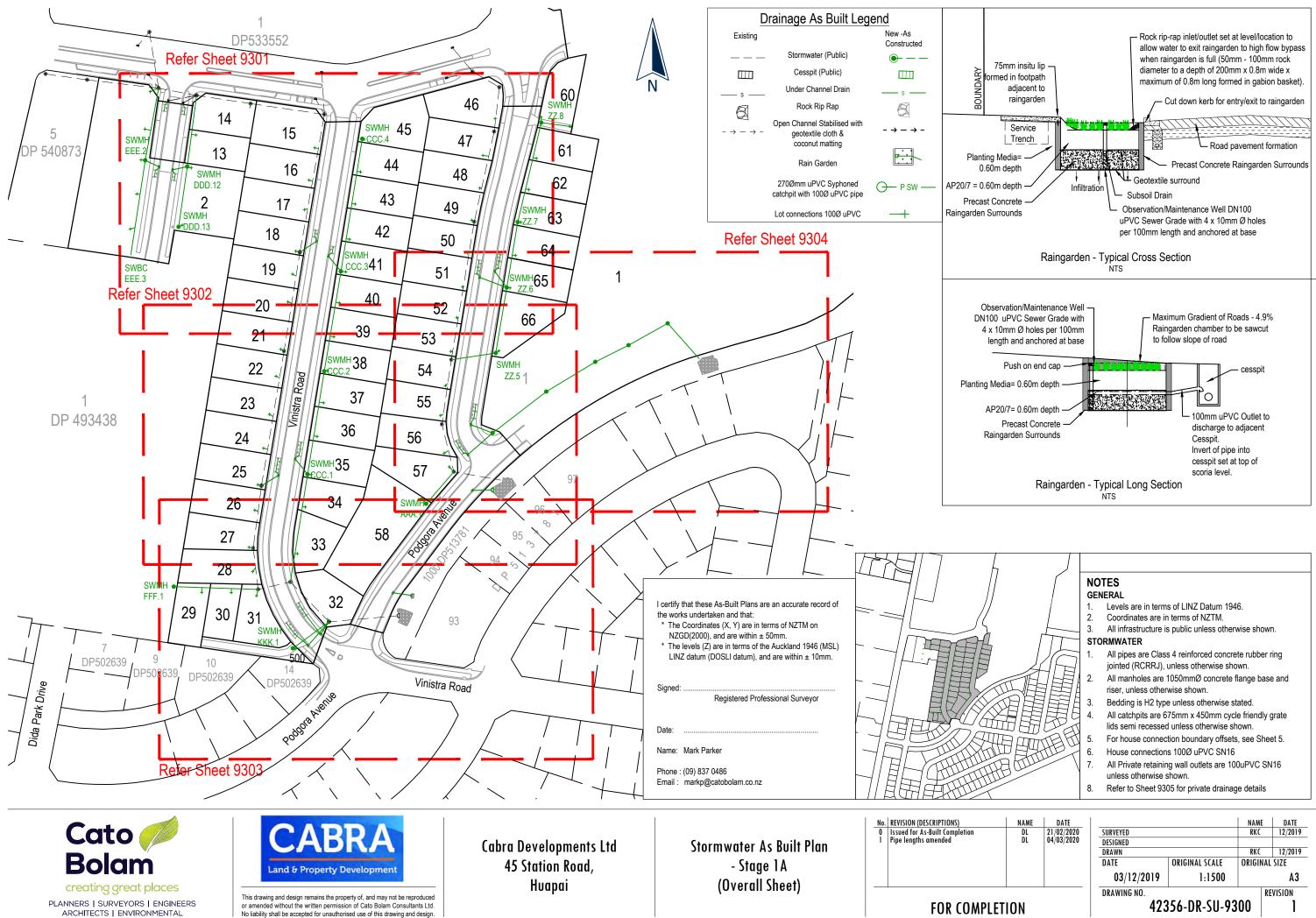
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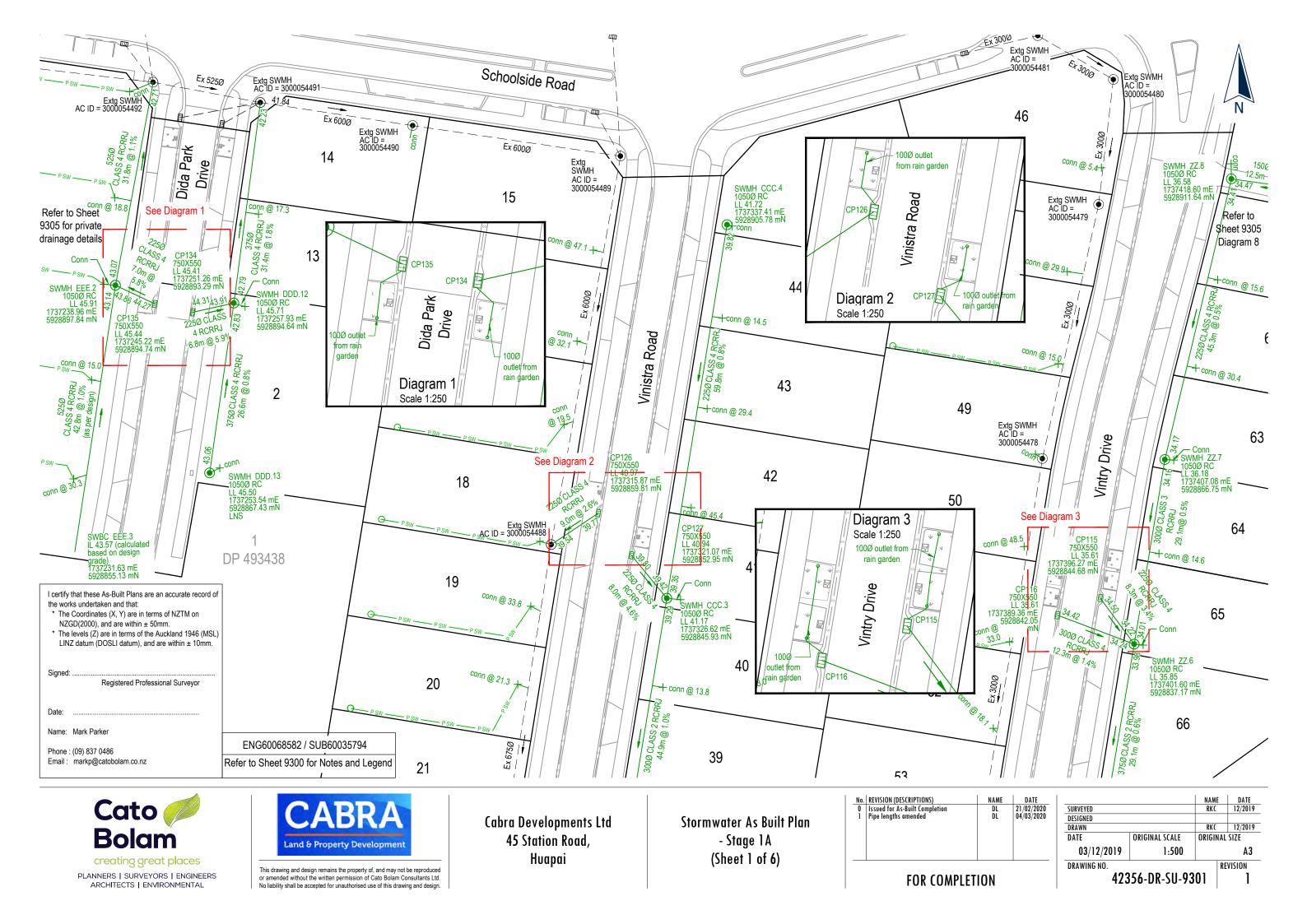


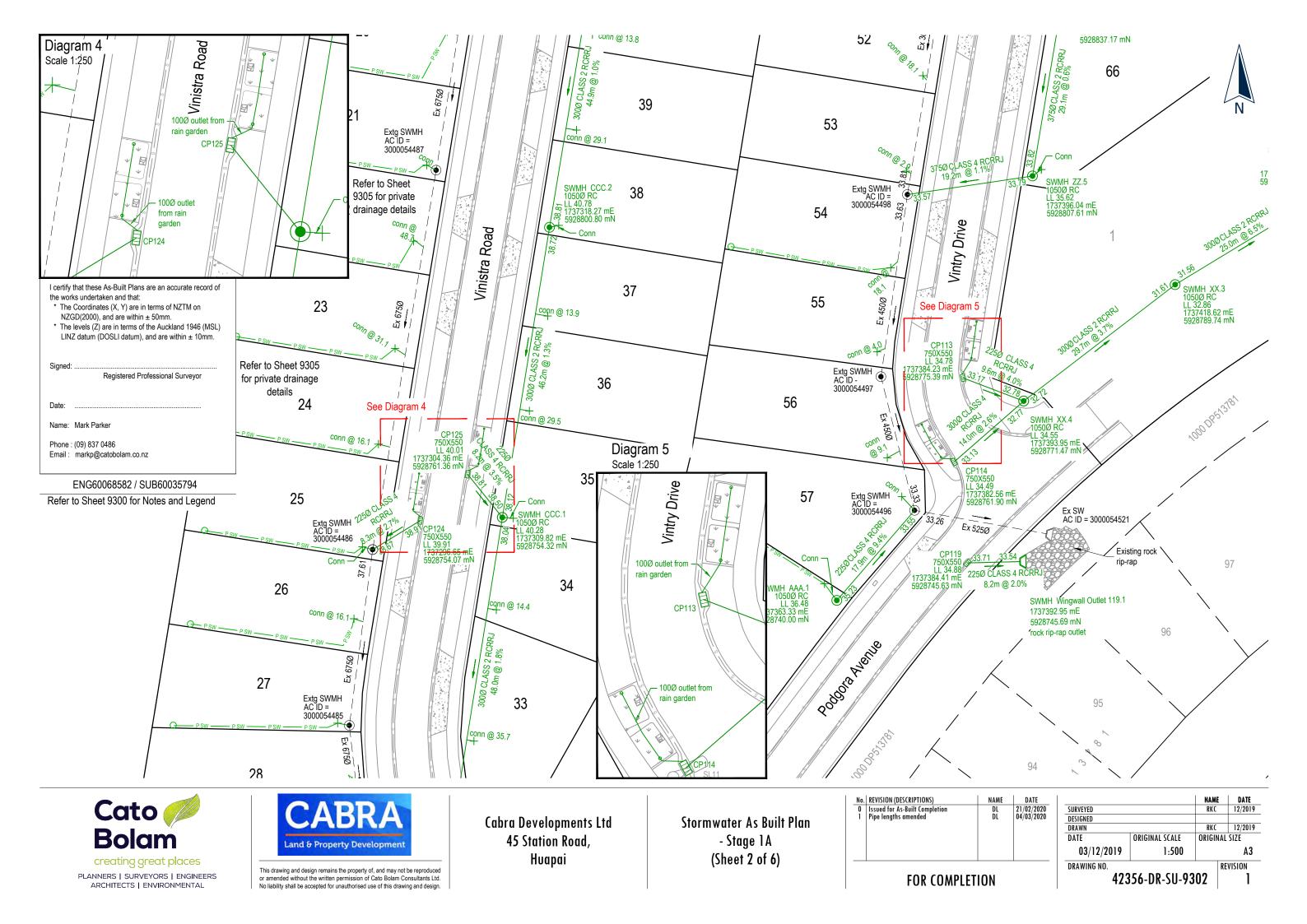
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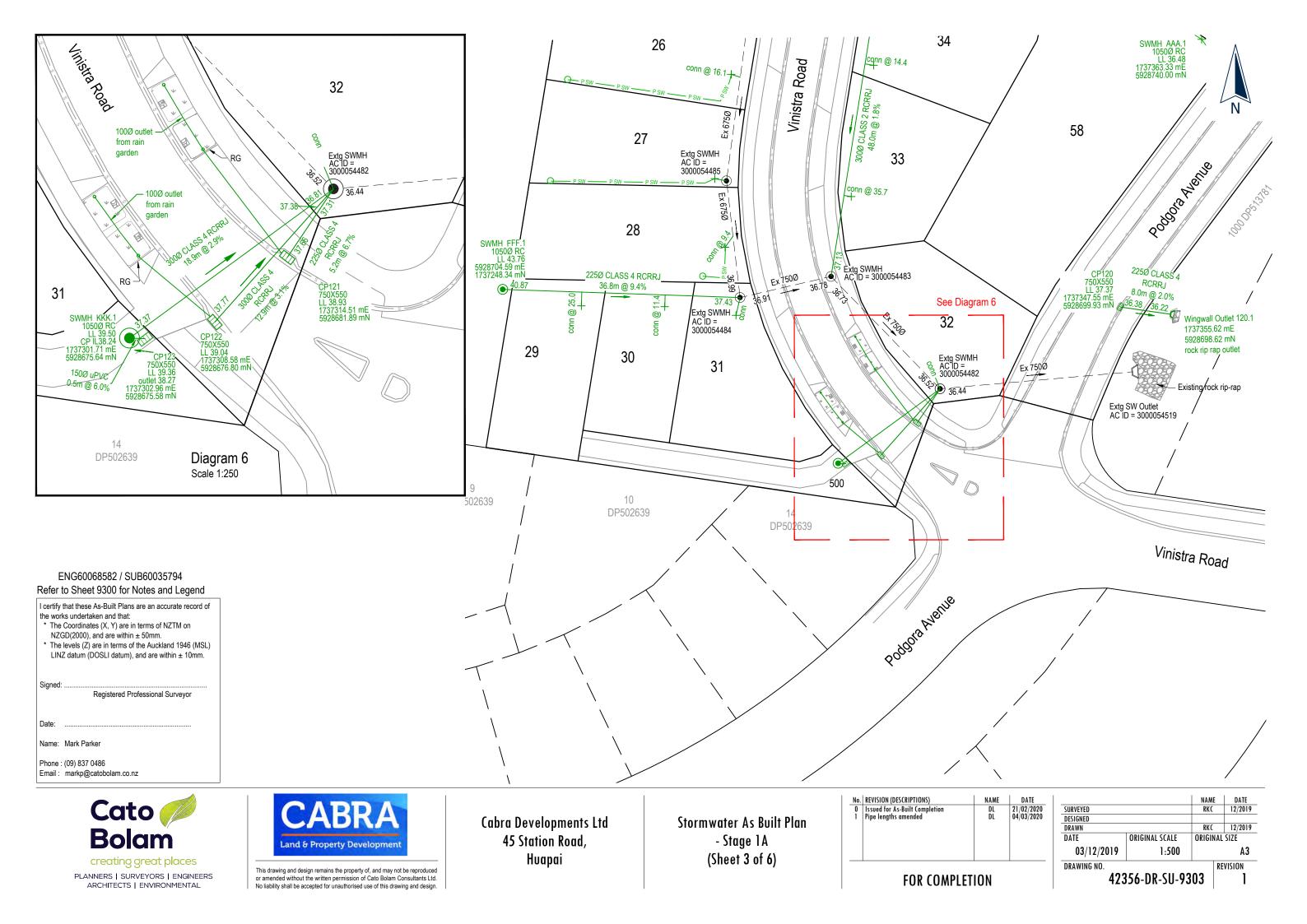


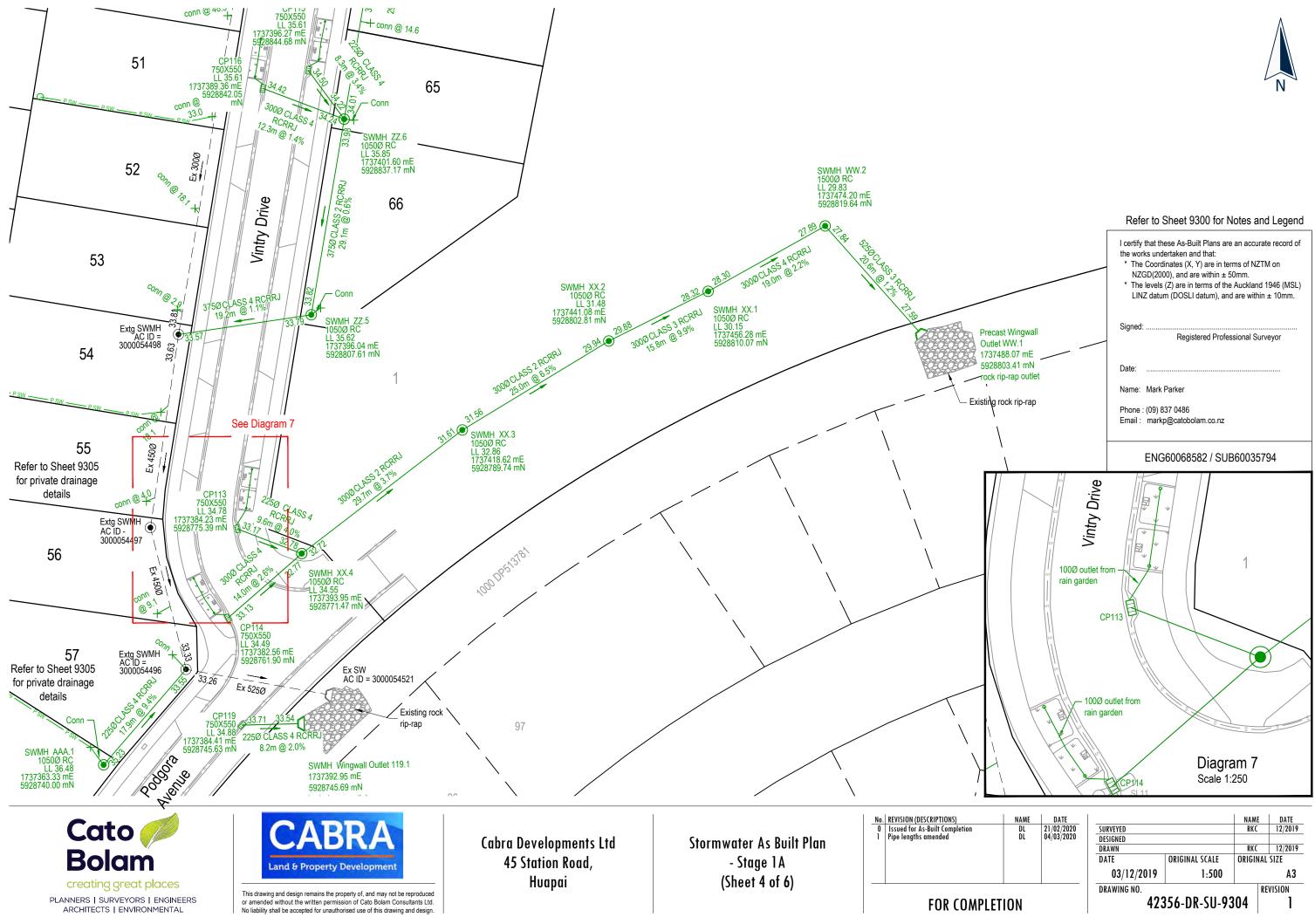


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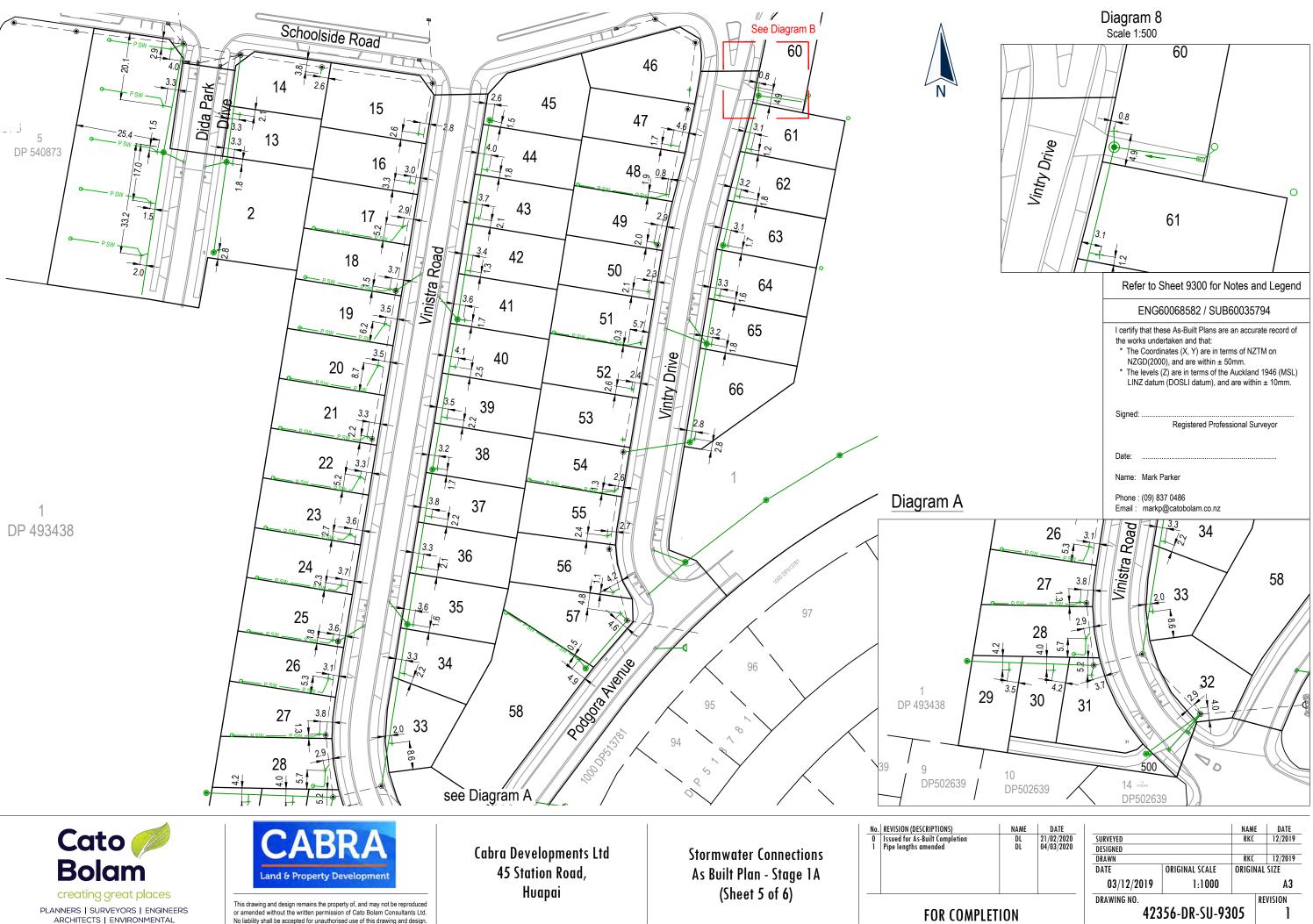


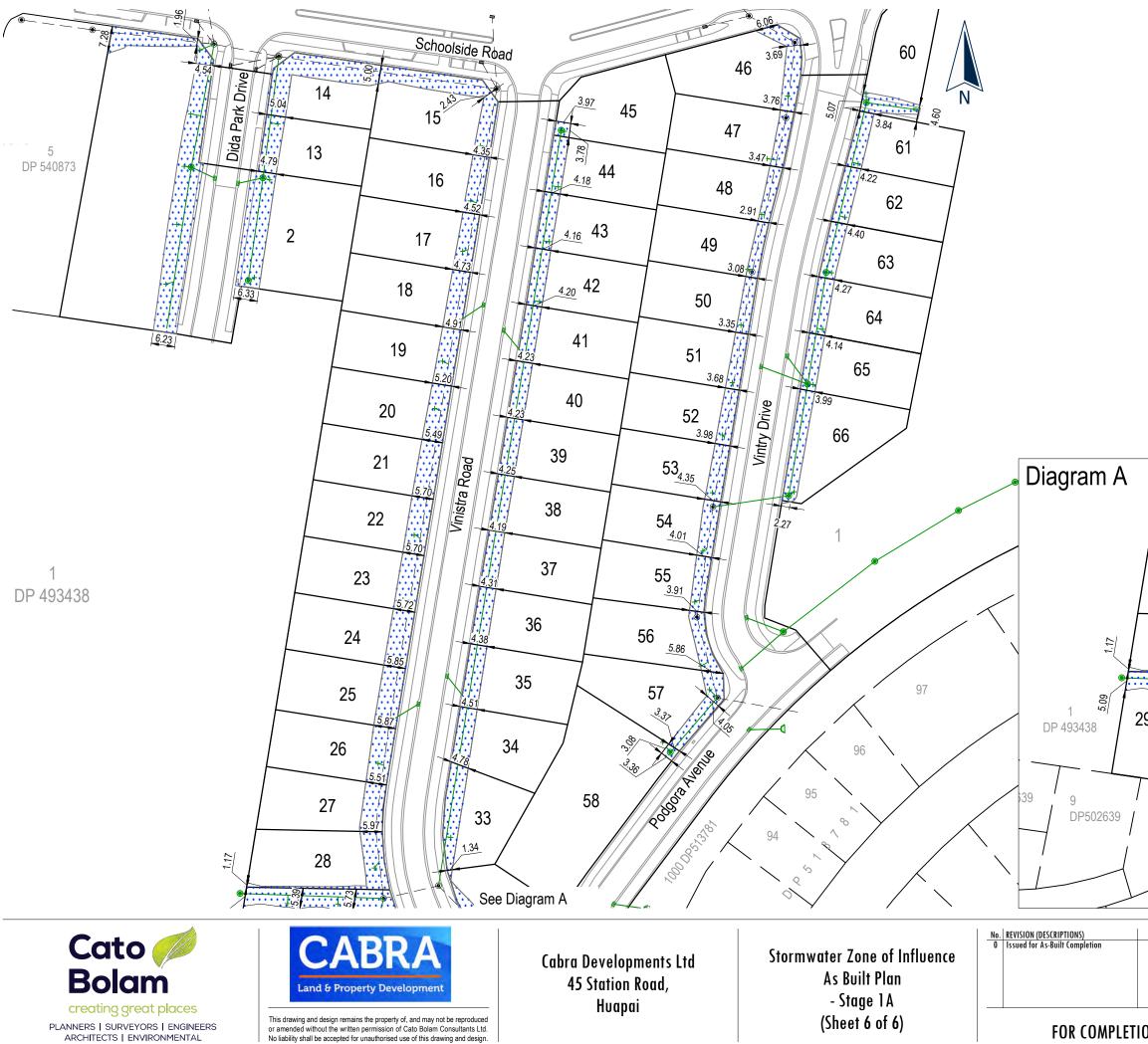




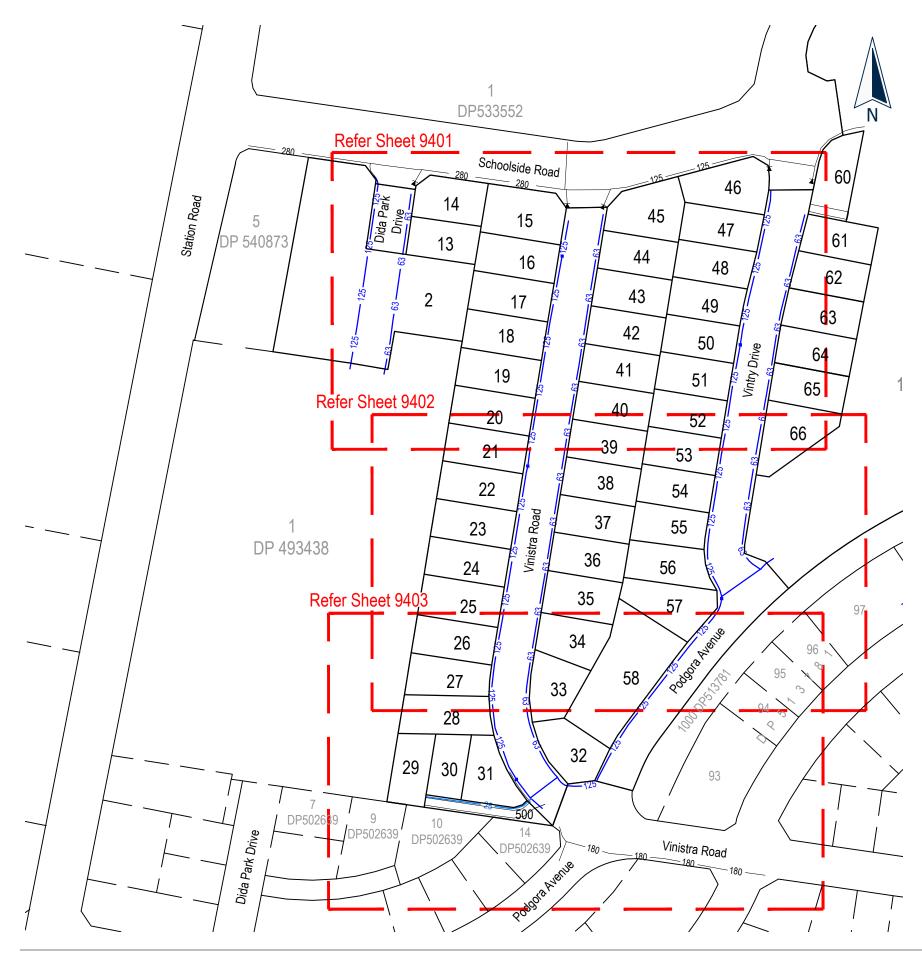








	Zone of Influence Legend	
	Pipeline 45° zone of influence are from 5 of Stormwater Pipes	00mm below invert
	Refer to Sheet 9300 for other notes and l	egends
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	Refer to Sheet 9300 for Notes and	Legend
the *	tertify that these As-Built Plans are an accurate re- e works undertaken and that: The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within \pm 50mm. The levels (Z) are in terms of the Auckland 1946 LINZ datum (DOSLI datum), and are within \pm 10	(MSL)
Si	gned: Registered Professional Surveyor	r
Da	ate:	
Na	ame: Mark Parker	
	none : (09) 837 0486 nail : markp@catobolam.co.nz	
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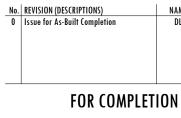
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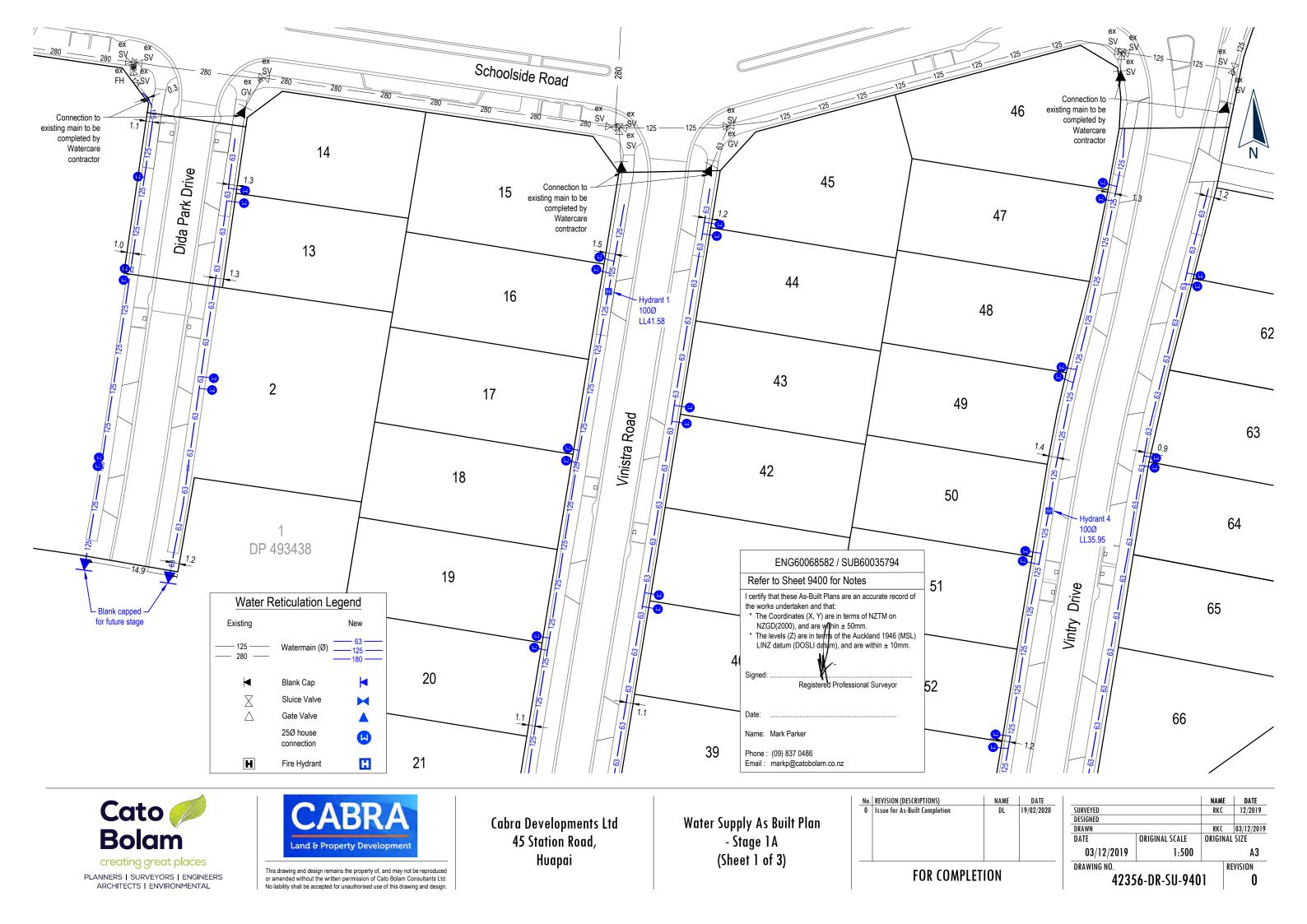
Cabra Developments Ltd 45 Station Road, Huapai

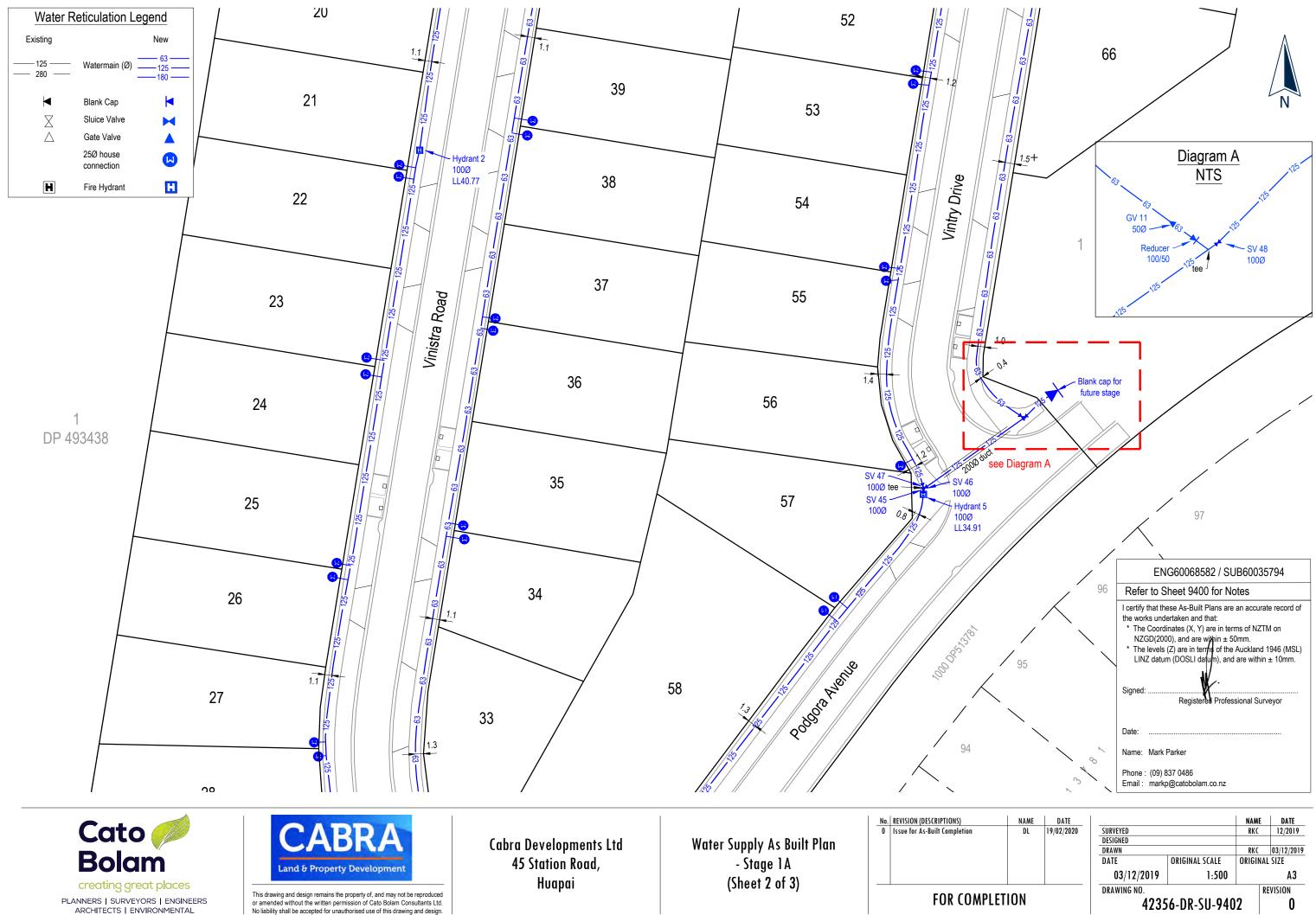
Water Supply As Built Plan - Stage 1A (Overall Sheet)

Water Supp Name SV41 592 SV43 592 SV44 592 SV45 59 59 SV46 SV47 59 SV48 592 GV9 593 GV11 59 Hydrant 1 592 Hydrant 2 592 Hydrant 3 592 Hydrant 4 592 Hydrant 5 59



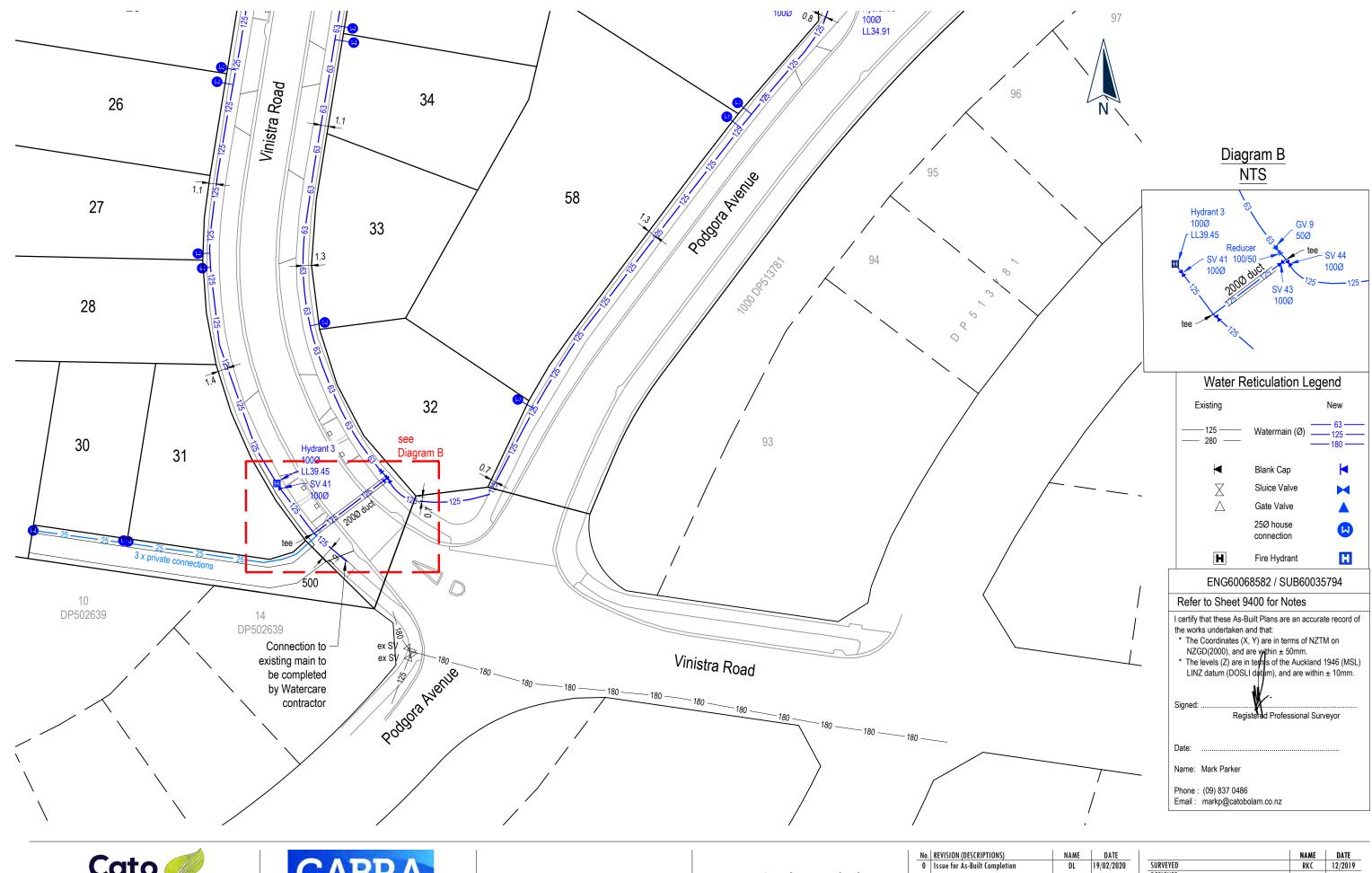
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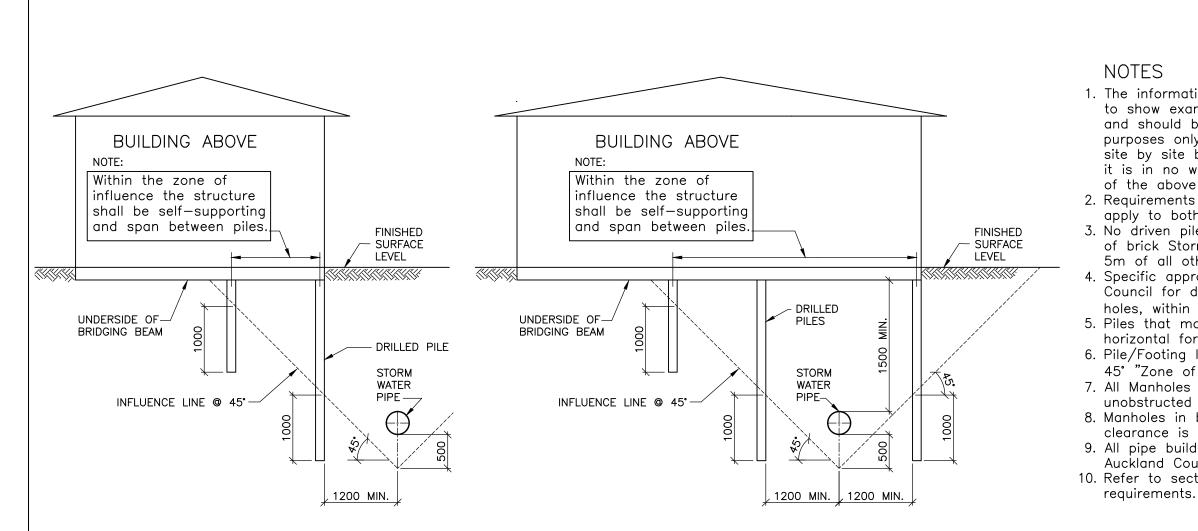
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Cabra Developments Ltd 45 Station Road, Huapai Water Supply As Built Plan - Stage 1A (Sheet 3 of 3)

FOR COMPLETION

DATE			NAME	DATE
19/02/2020	SURVEYED		RKC	12/2019
	DESIGNED			
	DRAWN		RKC	03/12/2019
	DATE	ORIGINAL SCALE	ORIGIN	IAL SIZE
	03/12/2019	1:500		A3
	DRAWING NO.			REVISION
	4235	56-DR-SU-940	3	0

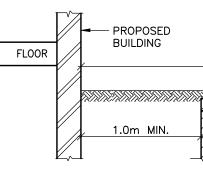


"BUILD CLOSE" NOTES:

- 1. Specific approval is required from Auckland Council if building adjacent to pipes, larger than 375mm internal diameter, or greater than 3.0m of depth.
- 2. Building to be outside all overland flow paths and floodplains.
- 3. Pile constructed to a depth of 1.0m below influence line.
- 4. Outside zone of influence, normal foundation requirements apply.

"BUILD OVER" NOTES:

- 1. Applies to stormwater pipes 375mm nominal diameter or less.
- 2. Bridging over pipes larger than 375mm nominal diameter is NOT allowed under any circumstances.
- 3. Pile constructed to a depth of 1.0m below influence line.
- 4. Outside zone of influence, normal foundation requirements apply.
- 5. Bridging is NOT allowed over pipes where clear vertical seperation distance from top of pipe to underside of bridging beam is less than 1.5m



STORMWATER ENGINEERING STANDARD DETAILS ISSUE/REVISION: 1

DATE: 30 September 2013 CAD FILE: AC-STD-SW22

$C \odot U \mathbb{N} C \square L$ AUGKLAND.

STORMWATER PIPE AND MANHOLE CONSTRUCTION CLEARANCE REQUIREMENTS MANHOLES NEAR BUILDINGS AND BUILDING CLOSE OR OVER PIPES

1. The information on this page is intended to show examples of typical scenarios and should be used for general guidance purposes only. Significant variations on a site by site basis are to be expected and it is in no way implied that meeting any of the above will guarantee approval. 2. Requirements for foundation design etc. apply to both sides of pipe. 3. No driven piles are permitted within 10m of brick Stormwater Structures, or within 5m of all other Stormwater Structures. 4. Specific approval is required from Auckland Council for driven piles in partially drilled holes, within the 5m/10m zone. 5. Piles that may be required to resist horizontal forces will require specific design. 6. Pile/Footing location point must be below 45° "Zone of Influence". 7. All Manholes shall have 24 hours unobstructed access. 8. Manholes in basements, or where sufficient clearance is unavailable, are not permitted. 9. All pipe buildovers will require approval by Auckland Council. 10. Refer to section 4.3.21 for pipe buildover ADJACENT 5m MIN. CLEARANCE BUILDING 3.2m MIN. 1.0m MIN. MANHOLE CONSTRUCTION CLEARANCE ORIGINAL SCALE: AS<u>NOTED</u> ENVIRONMENTAL-SW DRAWING No. REV ACSD Auckland OF Council SW22

Appendix C: Laboratory Test Data



Please reply to: W.E. Campton

CMW Geosciences Ltd. PO Box 300 206 Albany Auckland 0752

Attention: **JASMINE WALDEN**

Babbage Geotechnical Laboratory Level 4 68 Beach Road P C Auckland 1010 Ne Telephone 64-E-mail we

P O Box 2027 New Zealand 64-9-367 4954 wec@babbage.co.nz

Page 1 of 5

Job Number: 63282#L BGL Registration Number: 2766 Checked by: WEC

24th February 2020

SHRINK-SWELL INDEX TESTING

Dear Jasmine,

Re: STATION ROAD, HUAPAI (your reference AKL2016-0634) Report Number: 63282#L/SS Station Road, Huapai

The following report presents the results of Shrink-swell Index testing at BGL of 54mm diameter undisturbed push-tube soil samples delivered to this laboratory on the 14th of February 2020. The test standards used were:

Water Content:	NZS4402:1986:Test 2.1					
Sampling Tube <i>in situ</i> Density:	NZS4402:1986:Test 2.1 NZS4402:1986:Test 5.1.3 AS1289:Test 7.1.1 - 2003					
Shrink-swell Index:	AS1289:Test 7.1.1 - 2003					

As per the reporting requirements of AS1289: Test 7.1.1 – 2003: the shrink-swell index value has been reported to the nearest 0.1%. As per the reporting requirements of NZS4402: 1986: Test 2.1: water content is reported to two significant figures for values below 10%, and to three significant figures for values of 10% or greater. As per the reporting requirements of NZS4402: 1986: Test 5.1.3: sampling tube density, all density values have been reported to the nearest 0.02t/m³ and air voids have been reported to two significant figures.

For calculating the air voids percentages a solid density of 2.65t/m³ was assumed for these tests. Note that this assumed value is not part of the IANZ endorsement for this report.



Sample Descriptions (not part of BGL IANZ Accreditation)

- 1 LOT 06 / 0.50 0.65m: CLAY, slightly silty, very stiff, moderately plastic, pinkish yellow, light grey & light brown, slightly moist.
- 2 LOT 12 / 0.50 0.65m: CLAY, silty, trace gravel, very stiff, moderately plastic, mottle light grey, brown & pink, slightly moist.
- 3 LOT 16 / 0.50 0.65m: CLAY, slightly silty, very stiff, moderately plastic, mottled light grey, orange, brown & pink, slightly moist.
- 4 LOT 22 / 0.50 0.65m: CLAY, slightly silty, very stiff, moderately plastic, light grey with occasional yellow mottles, slightly moist.
- 5 LOT 28 / 0.40 0.55m: CLAY, silty, very stiff, moderately plastic, mottled grey, orange, & brown, slightly moist.
- 6 LOT 30 / 0.50 0.65m: CLAY, silty, very stiff, moderately plastic, mottled light grey, orange, & brown, slightly moist.
- 7 LOT 35 / 0.50 0.65m: CLAY, silty, very stiff, moderately plastic, mottled yellow, pink, black & white, moist.
- 8 LOT 41 / 0.50 0.65m: CLAY, silty, very stiff, moderately plastic, dark yellow with black & white mottles.
- 9 LOT 46 / 0.50 0.65m: CLAY, very stiff, moderately plastic, mottled light grey, orange, & pink, slightly moist.
- 10 LOT 52 / 0.50 0.65m: CLAY, slightly silty, stiff to very stiff, moderately plastic, mottled light grey, orange, pink & brown, slightly moist.
- 11 LOT 58 / 0.50 0.65m: CLAY, slightly silty, stiff to very stiff, moderately plastic, streaked pinkish light grey, slightly moist.
- 12 LOT 62 / 0.50 0.65m: CLAY, fine to medium gravelly, very stiff, moderately plastic, light greyish brown, slightly moist to dry.



Job Number: 63282#L 24th February 2020 Page 3 of 5

	SHRINK-SWELL TEST RESULTS											
Sample Nu	mber	4	5	6								
Lot Numb	ber	LOT 06	LOT 12	LOT 16	LOT 22	LOT 28	LOT 30					
Depth (n	n)	0.50 – 0.65	0.50 - 0.65	0.50 – 0.65	0.50 – 0.65	0.50 – 0.65	0.50 - 0.65					
SWELL TEST												
Initial Water Content	%	46.8	29.9	41.3	44.5	30.6	34.0					
Initial Bulk Density	t/m³	1.70	1.94	1.74	1.72	1.88	1.82					
Initial Dry Density	t/m³	1.10	1.58	1.22	1.18	1.44	1.38					
Initial Air Voids	%	0.0	3.6	0.84	0.64	1.9	4.1					
Total Swell	mm	0.0	0.1	0.1	0.1	0.03	0.1					
Swelling Strain	5 0/2		0.3	0.2	0.3	0.1	0.3					
			SHRIN	IKAGE TI	EST							
Water Content	%	45.6	32.8	47.3	48.0	34.0						
Initial Bulk Density	t/m³	1.58	1.88	1.72	1.72	1.86	1.86					
Initial Dry Density	t/m³	1.08	1.42	1.18	1.16	1.40	1.38					
Initial Air Voids	%	10	0.50	0.31	0.16	1.2	0.85					
Total Shrinkage	mm	12.2	6.3	8.0	11.0	7.1	6.1					
Shrinkage Strain	%	11.2	6.5	9.1	10.1	6.5	7.3					
			SHRINK	SWELL I	NDEX							
SHRINK- SWELL INDEX	%	6.2	3.7	5.1	5.7	3.7	4.2					



Job Number: 63282#L 24th February 2020 Page 4 of 5

	SHRINK-SWELL TEST RESULTS											
Sample Nu	mber	7	8	9	10	11	12					
Lot Numb	ber	LOT 35	LOT 41	LOT 46	LOT 52	LOT 58	LOT 62					
Depth (n	n)	0.50 – 0.65	0.50 - 0.65	0.50 – 0.65	0.50 – 0.65	0.50 – 0.65	0.50 - 0.65					
SWELL TEST												
Initial Water Content	%	37.8	43.4	36.9	47.5	40.9	31.1					
Initial Bulk Density	t/m³	1.86	1.82	1.80	1.76	1.80	1.88					
Initial Dry Density	t/m³	1.38	1.32	1.28	1.24	1.26	1.44					
Initial Air Voids	%	0.55	0.15	1.2	0.94	0.0	0.63					
Total Swell	mm	0.1	0.1	0.1	0.3	0.1	0.1					
Swelling Strain	3 % 114		0.7	0.7	1.3	0.6	0.7					
			SHRIN	IKAGE TI	EST							
Water Content	%	40.3	43.0	42.0	51.6	35.2	29.7					
Initial Bulk Density	t/m³	1.78	1.76	1.76	1.68	1.82	1.84					
Initial Dry Density	t/m³	1.26	1.22	1.24	1.12	1.34	1.42					
Initial Air Voids	%	1.2	1.0	1.2	0.55	2.3	4.0					
Total Shrinkage	mm	7.1	10.9	7.6	12.4	5.8	4.5					
Shrinkage Strain	%	6.6	10.0	8.6	11.5	7.0	4.4					
			SHRINK-	SWELL	NDEX							
SHRINK- SWELL INDEX	%	3.8	5.8	5.0	6.7	4.1	2.6					



Job Number: 63282#L 24th February 2020 Page 5 of 5

Please note that the test results relate only to the samples under test.

Thank you for the opportunity to carry out this testing. If you have any queries regarding the content of this report please contact the person authorising this report below at your convenience.

Yours faithfully,

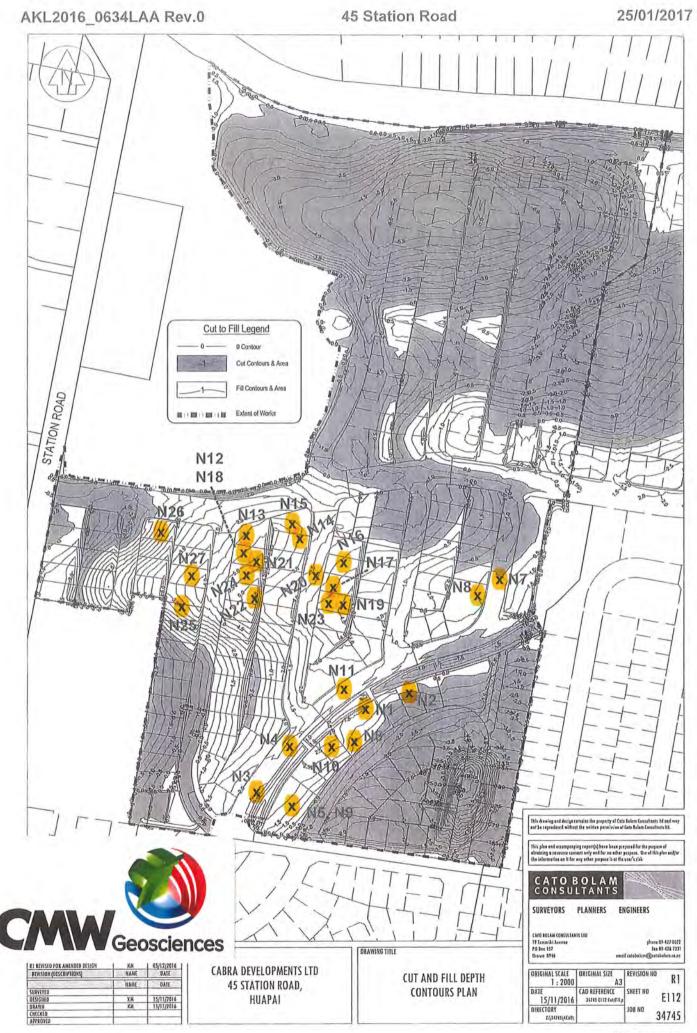
Justin Franklin Signatory (Assistant Laboratory Manager) Babbage Geotechnical Laboratory



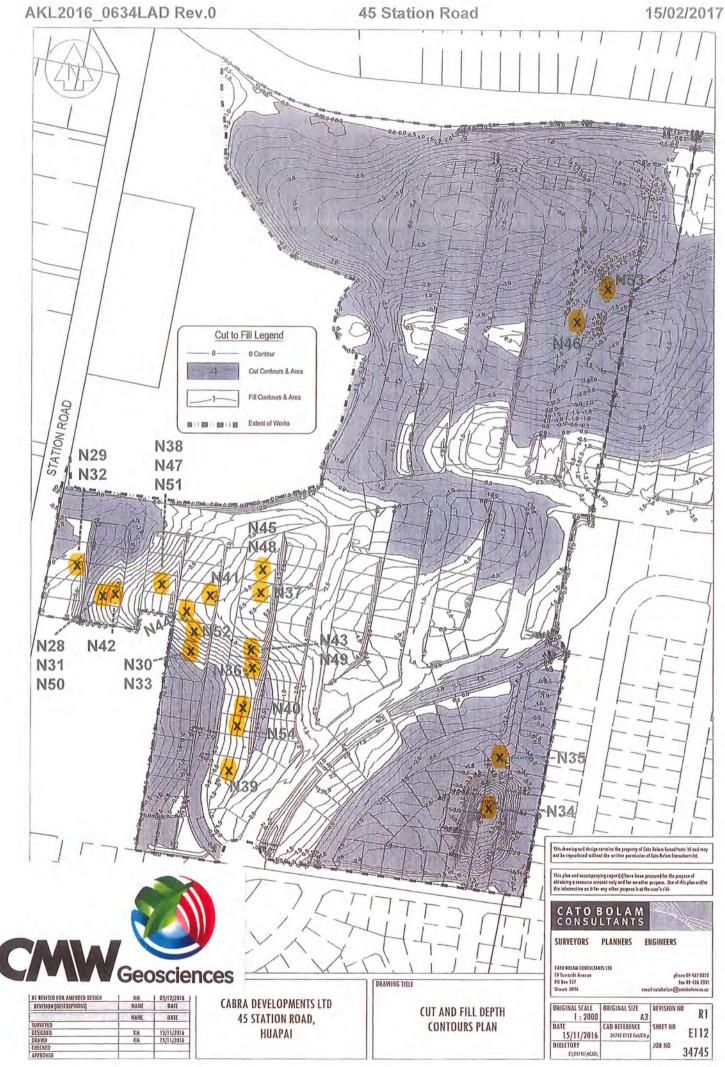
All tests reported herein have been performed in accordance with the laboratory's scope of accreditation. This report may not be reproduced except in full & with written approval from BGL.

Appendix D: Field Test Data

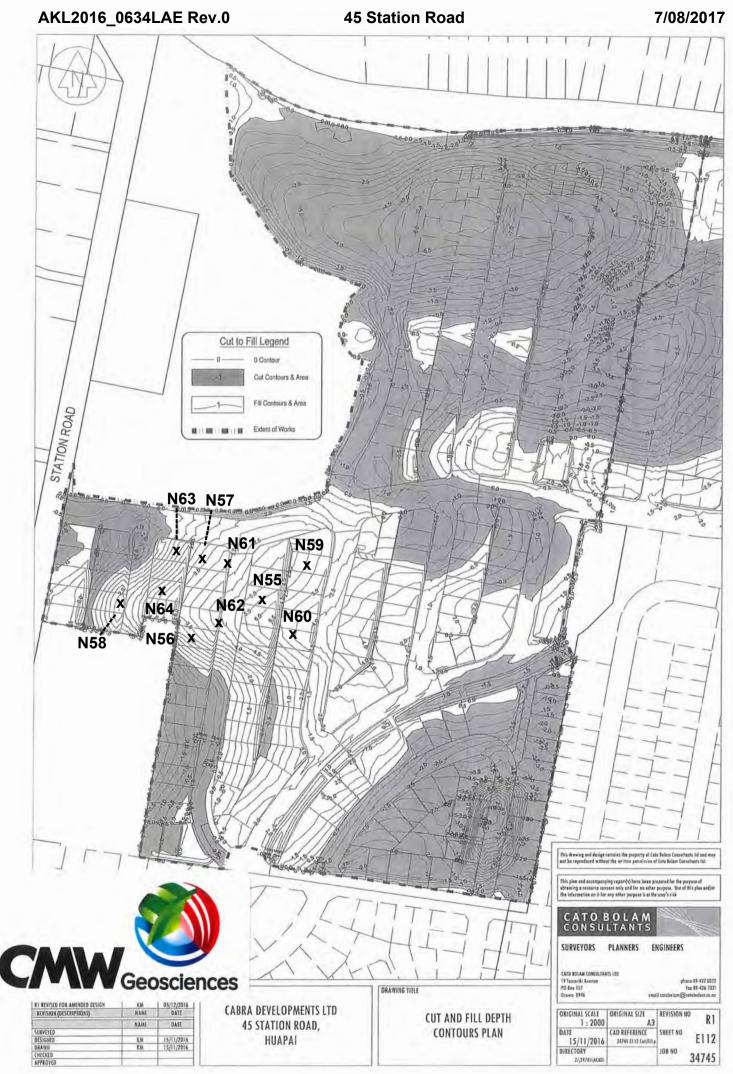
Chepman Morton Woodward	LF11 Rev 4 Soil	l Field Density ND	M Dire	ct Tr	ansr	nissio	on wi	th VSS	Report	100.00	Building C, 9 P	nces (NZ) Limi Iermark Drive 6, Albany, Auc	ted Rosedale, NZ 0 kland, NZ 0752	632			
Project: Project No: Location: Report No:	45 Station Road, Huapai AKL2016_0634 Huapai AKL2016_0634LAA Rev.0										Test Methor NZS 4402.2. NZS 4407.4. NZGS:Augus	1:1986 2.2:2015	5	Notes: Solid Densit Festing Loca	y: ations Selecte		Assumed CMW Field Staff
Report Date: Client: Client Address: Client Reference:	25/01/2017 Cabra Developments Limited										ACCR		L not a the s	s indicated as accredited are cope of the atory's accred	outside	and are outside	marked * are not accred the scope of the laborate accreditation
			In	-situ Var	ne Shear	Strength	5				Field and	Laboratory Te	sting Data				
Date Sampled Sample No.	Test Location	Soil Description	Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m ³)	Gauge Dry Density (t/m³)	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth	Oven Water Content (%)	Solid Density (t/m ³) *	Oven Dry Density (t/m³)	Calculated Air Voids (%)	Comments
7/12/2015 N1	Lot 97	CLAY	189	189	>189	>189	>189	1.8543	1,3865	33.7	1.76	300	29.3	2.7	1.44	4.9	
N2	Lot 99	CLAY	>189	>189	>189	>189	>189	1.8482	1,3592	36.0	0.65	300	37.4	2.7	1.34	-0.1	
14/12/2016 N3	Refer to site plan	CLAY	UTP	UTP	UTP	UTP	UTP	1,7574	1.3213	33.0	7.35	300	29.6	2.7	1.36	9.6	
N4	Refer to site plan	ELAY	173	178	189	189	182	1,8379	1.3691	34.2	2.31	300	35.0	2.7	1.36	1,9	
16/12/2016 N5	Lot 93	CLAY	175	173	189	189	182	1.7953	1.2722	41.1	0.45	250	31.0	2.7	1,38	6.8	
N6	Lot 97	CLAY	UTP	UTP	UTP	UTP	UTP	1,7945	1.3099	37.0	2.91	300	33.4	2.7	1.34	5.2	
N7	Refer to site plan	CLAY	>189	>189	UTP	UTP	>189	1.8157	1.3169	37.9	1,24	250	30.8	2.7	1.38	5.9	
NB	Lot B6	CLAY	UTP	UTP	UTP	UTP	UTP	1.8583	1.3689	35.7	0,25	300	38.6	2.7	1.34	-1.4	
20/12/2016 N9	Lot 93	CLAY	UTP	UTP	>189	>189	>189	1.8210	1.3141	38.6	0.52	300	34,9	2.7	1.34	2,9	
N10	Lot 95	CLAY	UTP	UTP	>189	>189	>189	1.8331	1.3237	38.5	-0.08	300	33.5	2.7	1.38	3,2	
22/12/2016 N11	Road	CLAY	189	186	183	159	179	1.7840	1.3012	37.7	2.30	300	36.3	2.7	1,30	4.0	
30/12/2016 N12 N13	Lot 44 Lot 45	Silty CLAY	>194	>194	193	141	>181	1,7770	1.2843	38.4	3.05	300	48.3	2.7	1,20	-2:2 }	alled
N14	Lot 47	Clayey SILT Silty CLAY	UTP	151 >194	158 >194	148 >194	>162	1,7915	1.3219	35.5	3.98	300	38.7	2.7	1,30	2,2	
N15	Lot 46	Silty CLAY	>194 >194	>194	>194	1.1.1	>194	1.8021	1.3070	37.9 36.8	1.97	300	39.5	2.7	1.30	1,1	
4/01/2017 N16	Lot 62	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8617	1.3130	37.0	2,88	300	40.3	2.7	1.28	1.0	
N17	Lot 63	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8246	1.3219	38.0	0.65	300	31.6	2.7	1.42	2.9	
N18	Lot 44	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8530	1.3908	33.2	2.17	300	37.4	2.7	1.34		Retest of N12
5/01/2017 N19	Lot 64	Sility CLAY	188	151	154	157	163	1.7956	1.2963	38,5	1.95	300	54.4	2.7	1,54	-6.3	
N20	Road	SIITY CLAY	>194	>194	>194	148	>182	1.7699	1.2558	40.9	1.96	300	28.7	2.7	1.38	9.6	411-0
6/01/2017 N21	Lot 44	CLAY	UTP	UTP	UTP	UTP	UTP	1,8059	1.3199	36.8	2.44	300	44.8	2.7	1.24	-2.0	
N22	Lot 42	CLAY	>194	>194	191	>194	>193	1.8125	1.3097	38:7	1.09	300	32.9	2.7	1.36	4.6	
N23	Lot 64	CLAY	UTP	UTP	UTP	UTP	UTP	1.8148	1.3314	36.3	2.25	300	43.2	2.7	1,26		Retest of N19
10/01/2017 N24	Lot 43	CLAY	189	189	>189	>189	>189	1.7905	1.3118	36,5	3.44	300	36.3	2.7	1.32	3,6	
N25	Lot 19	CLAY	178	189	189	>189	>186	1.7855	1.2864	38.8	2.33	300	38.5	2.7	1.28	2,6	
	Lot 14	CLAY		>189	UTP			1.7964	1.2794	40.4	0.80	300	41.3	2.7	1.28	0.4	
	Lot 17	CLAY	>189	>189	UTP	UTP	>189	1.7357	1,2292	41.2	3.71	300	36.9	2.7	1.26	6.3	
This report should only b Created By: TG Checked By: TG Authorised Signatory:		Date: Date: Date:	8/12 25/01	/2016 /2017										4.17	110		Page: 1 of



	LF11 Rev 4 Soil Field Density NDM Direct Transmission with VSS Report													Auckland Laboratory CMW Geosciences (NZ) Limited Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752 Phone: +64 (09) 4144 632								
roject:		45 Station Road, Huapai										Test Method	is:		Notes:			-				
roject No:		AKL2016_0634										NZS 4402.2.1	Z5 4402.2.1:1986 Solid Density: Assumed									
ocation:		Ниараі										NZS 4407.4.2.2:2015 Testing Locations Selected By: CMW Field Staf										
Report No:		AKL2016_0634LAD Rev.0										NZGS:Augus										
eport Date:		15/02/2017													_		1					
lient		Cabra Developments Limited													indicated as	august die	Measurement	s marked " are not accredit				
Client Address												in of accredited are outside and are outside the scope of the						e the scope of the laborator				
Client Referen	ce:											ACCREDITED LABORATORY laboratory's accreditation accreditation										
				In-situ Vane Shear Strengths							Field and Laboratory Testing Data						-					
Nate Sampled	Sample No.	Test Location	Soil Description	Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m³)	Gauge Dry Density (t/m³)	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth	Oven Water Content (%)	Solid Density (t/m ³) *	Oven Dry Density (t/m ³)	Calculated Air Voids (%)	Comments				
12/01/2017	NZS	Lot 9	CLAY	97	124	138	135	124	1.7523	1.2313	42.3	2.18	300	35.5	2.7	1,30	5.2	Failed				
	N29	Lot 3	CLAY	167	189	>189	>189	>184	1.7401	1.2240	42.2	2.94	300	39.0	2.7	1.26						
	N30	Lot 20	CLAY	138	132	167	140	144										Falled				
13/01/2017	N31	Lot 9	CLAY	189	189	>189	>189	>189	1.7241	1.2262	40.6	4.68	300	41.6	2.7	1.22		Re-test of N28				
	N32	Lot 3	CLAY	>189	>189	189	189	>189	1,7342	1.2154	42.7	2.99	300	50.0	2.7	1.15						
	N33	Lot 20	CLAY	178	170	189	183	180	1.7505	1.2899	35.7	6.07	300	36.1	2.7	1.28		Re-test of N30				
16/01/2017	N34	Refer to site plan	CLAY	189	189	>189	>189	>189	1.8756	1.4034	33.6	0.70	300	32.9	2.7	1.42						
	N35	Refer to site plan	CLAY	178	175	189	189	183	1.7740	1.3549	30.9	7.82	300	32.3	2.7	1.34						
	N36	Lot 39	CLAY	UTP	UTP	UTP	UTP	UTP	1.7281	1.2764	35.4	7.46	300	30.7	2.7	1.32						
	N37	Lot 43	CLAY	UTP	UTP	UTP	UTP	UTP	1.7351	1.2784	35.7	6.88	300	37.1	2.7	1.26						
	N38	Lot 12	CLAY	UTP	UTP	UTP	UTP	UTP	1.8507	1.3499	37.1	-0.19	300	43.6	2.7	1.28	-3.9					
18/01/2017	N39	Lot 37	CLAY	UTP	UTP	UTP	UTP	UTP	1.8490	1.4097	31.2	3.76	300	31.1	2.7	1.42						
	N40	Lot 34	CLAY	183	189	189	>189	>188	1.7472	1.2828	36,2	5.95	300	34.9	2.7	1.30	6.9					
	N41	Lot 17	CLAY	127	140	119	121	127	1.6636	1.1636	44.0	5.64	300	35.0	2.7	1.24		Failed				
	N42	Lot 9	CLAY	>189	>189	>189	>189	>189	1.7471	1.2521	39.5	4.01	300	46.0	2.7	1.20	0.6	10.00				
25/01/2017	N43	Lot 40	CLAY	>189	>189	UTP	UTP	>189	1.7865	1,3862	28.9	8.54	300	26.7	2.7	1.40	10.0					
	N44	Lot 18	CLAY	186	189	>189	>189	>188	1.8318	1,3358	37.1	0.81	300	38.8	2.7	1.32		Re-test of N41				
	N45	Lot 44	CLAY	173	178	189	>189	>182	1.7411	1,2510	39,2	4.55	300	37.3	2.7	1.26	5.8					
27/01/2017	N46	Refer to site plan	CLAY	178	146	151	165	160	1.7280	1.2275	40,8	4.38	200	45.5	2.7	1.18	2.0					
31/01/2017	N47	Lot 12	CLAY	UTP	UTP	UTP	UTP	UTP	1.7523	1.2719	37.8	4.74	300	39,3	2.7	1.25	3.9					
	N48	Lot 44	CLAY	UTP	UTP	UTP	UTP	UTP	1.7986	1.4142	27.2	9.10	300	34.6	2.7	1.34	4.3					
	N49	Lot 40	CLAY	UTP	UTP	UTP	UTP	UTP	1.7970	1.3901	29.3	7.74	300	35.3	2.7	1.32	3.9					
7/02/2017	N50	Lot 9	CLAY	UTP	UTP	UTP	UTP	UTP	1.7613	1.2824	37,3	4.50	300	44,4	2.7	1.22	0.7					
	N51	Lot 12	CLAY	UTP	UTP	UTP	UTP	UTP	1.7743	1.2653	40.2	2.12	300	52.4	2.7	1.16	-4.1					
	N52	Lot 19	CLAY	UTP	UTP	UTP	UTP	UTP	1.7893	1.3263	34.9	4.48	300	33.2	2.7	1.34	5.6					
	N53	Refer to site plan	CLAY				UTP			1,2984	35.7	5.51	300	34,4	2.7	1.32	6.4					
13/02/2017		Lot 36	CLAY	189	189	181	>189	>187	1.7475	1.2313	41.9	2.66	300	46.5	2.7	1.20						
his report sh	nould only I	be reproduced in full.									And 10 100 100											
reated By:	TG		Date:	13/01	/2017																	
hecked By:			Date:		/2017																	
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	NGeoscier on Woodward	LF11 Rev 4 SOIl	Field Density NDN	1 Dire	ect Tr	ansn	nissio	on w	ith VSS	Report		σ,	nces (NZ) Lim iermark Drive 6, Albany, Aud	ited , Rosedale, NZ :kland, NZ 0752					
Project:		45 Station Road, Huapai										Test Metho	ls:		Notes:				
Project No:		AKL2016_0634										NZS 4402.2.3	L:1986		Solid Density:			Assumed	
Location:		Huapai							NZS 4407.4.2.2:2015 Testing Locations Selected By:						ed By:	CMW Field Staff			
Report No:		AKL2016_0634LAE Rev.0										NZGS:Augus	t 2001						
Report Date:		7/08/2017													le la la col	1.1			
Client:		Cabra Developments Limited													s indicated as ccredited are o	outside		marked * are not accredited	
Client Address	:													lab as	cope of the atory's accredi	tation		the scope of the laboratories accreditation	
Client Referer	ce:											ACCRE	DITED LABOR	ATORY	atory's accredi	tation			
				Ir	n-situ Vai	ne Shear	Strength	IS	Field and Laboratory Testing Data										
Date Sampled	Sample No.	Test Location	Soil Description	Test 1 (kPa)	Test 2 (kPa)		Test 4 (kPa)	Ave.	Gauge Wet Density (t/m ³)	Gauge Dry Density (t/m ³)	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth	Oven Water Content (%)	Solid Density (t/m ³) *	Oven Dry Density (t/m ³)	Calculated Air Voids (%) *	Comments	
13/02/2017	N55	Lot 43	CLAY	189	181	189	181	185	1.7503	1.2697	37.8	4.81	300	39.3	2.7	1.26	4.1		
	N56	Lot 19	CLAY	>189	>189	>189	>189	>189	1.7807	1.3258	34.3	5.30	300	36.0	2.7	1.30	4.3		
	N57	Lot 15	CLAY	189	189	>189	>189	>189	1.7570	1.3029	34.9	6.23	300	37.9	2.7	1.28	4.5		
	N58	Lot 9	CLAY	154	178	183	189	176	1.7871	1.2880	38.7	2.28	300	35.9	2.7	1.32	4.1		
3/03/2017	N59	Lot 47	CLAY	>185	>185	>185	>185	>185	1.7501	1.3173	32.8	7.84	300	34.5	2.7	1.30	6.9		
	N60	Lot 51	CLAY	>185	>185	>185	>185	>185	1.8485	1.4333	29.0	5.30	300	30.2	2.7	1.42	4.5		
6/03/2017	N61	Lot 15	CLAY	>185	>185	>185	>185	>185	1.7496	1.2828	36.4	5.73	300	41.9	2.7	1.24	2.6		
	N62	Lot 18	CLAY	>185	>185	>185	>185	>185	1.7843	1.3322	33.9	5.35	300	31.2	2.7	1.36	7.2		
22/03/2017	N63	Lot 14	CLAY	>185	UTP	UTP	>185	>185	1.7893	1.4001	27.8	9.14	300	28.3	2.7	1.40	8.9		
	N64	Lot 12	CLAY	>185	>185	>185	>185	>185	1.8275	1.3958	30.9	5.04	300	26.4	2.7	1.44	8.3		
This report s	nould only b	be reproduced in full.																	
Created By:	TG		Date:	14/02	2/2017														
Checked By:	TG		Date:	7/08	8/2017														
Authorised S	gnatory:	GS	Date:	8/08	3/2017													Page: 1 of 2	

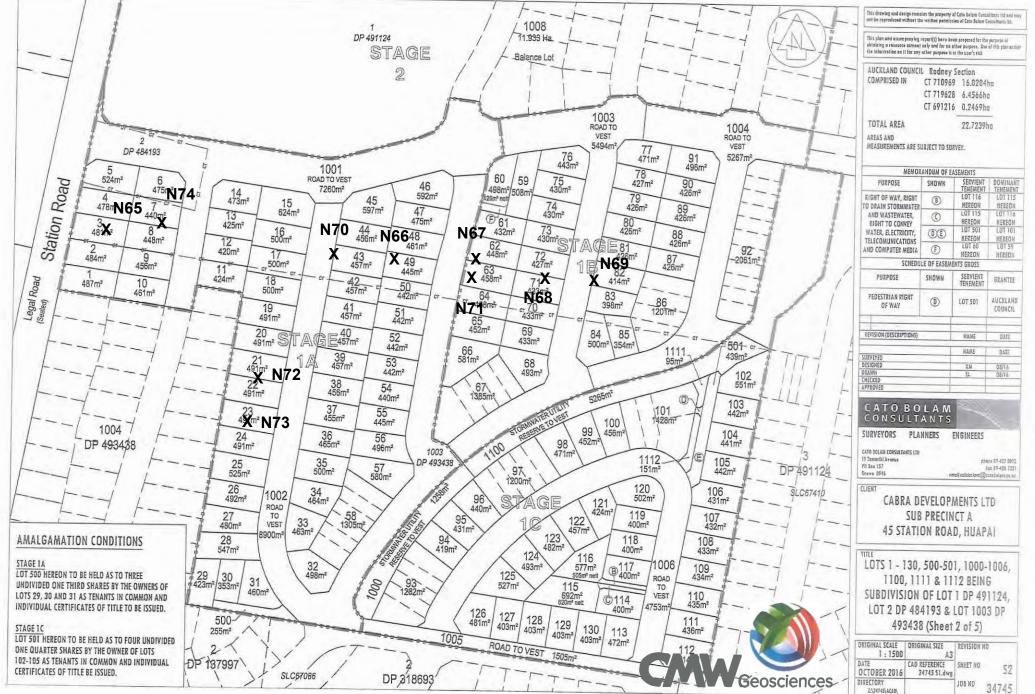


												Phone: +64 (0	-							
oject:		45 Station Road, Huapai										Test Metho			Notes:					
oject No:		AKL2016_0634									NZS 4402.2.1:1986				Solid Densit	,	Assumed			
cation:		Huapai										NZS 4407.4.			Testing Loca	tions Selecte	d By:	CMW Field Staff		
port No:		AKL2016_0634LAF Rev.0						NZGS:August 2001												
port Date:		17/07/2018												Tr	ests indicated	26				
ent:		Cabra Developments Limited												n	ot accredited a	re outside		is marked * are not accred e the scope of the laborate		
ent Address:	:														e scope of the boratory's accr			accreditation		
ent Referend	ce:											ACC	REDITED LAB	ORATORY						
				h	n-situ Va	ne Shear	Strength	ns				Field and	Laboratory To	esting Data						-
te Sampled	Sample No.	Test Location	Test Location Soil Description					Ave.	Gauge Wet Density (t/m ³) **	Gauge Dry Density (t/m ³)	Gauge Water Content (%)	er Voids (%) Don		Oven Water Content (%)	Density Density		Calculated Air Voids (%) *	Comments		
8/02/2018	N65	Refer to site plan	CLAY	155	155	UTP	UTP	155+	1.8720	1.3980	30.0	7.5	300	28.9	2.7	1.46	4.2			
26/02/2018	N66	Refer to site plan	CLAY	UTP	UTP	UTP	120	120+	1.8590	1.3508	34.0	1.0	300	31.8	2.7	1.42	2.9			
	N67	Refer to site plan	CLAY	UTP	180	140	180	167+	1.8680	1.3681	36.0	2.6	300	31.3	2.7	1.42	2.8			
	N68	Refer to site plan	CLAY	90	109	180	168	137	1.8010	1.2510	38.0	9.0	300					No Sample taken		
	N69	Refer to site plan	CLAY	180	140	180	190	173	1.8460	1.3480	36.0	3.4	300	20.9	2.7	1.52	12	Retest of N68		
5/03/2018	N70	Refer to site plan	CLAY	137	171	155	218+	170+	1.8410	1.3420	35.7	2.3	300	32.5	2.7	1.38	3.4			
1	N71	Refer to site plan	CLAY	171	218	218	155	191	1.8520	1.3501	35.1	2.1	300	30.4	2.7	1.42	4.2			
6/03/2018	N72	Refer to site plan	CLAY	UTP	UTP	UTP	UTP	UTP	1.8920	1.4590	29.1	2.8	300	28.3	2.7	1.48	3.6			
	N73	Refer to site plan	CLAY	UTP	UTP	215	UTP	215+	1.8290	1.3750	32.0	3.0	300	32.4	2.7	1.38	4.1			
15/03/2018	N74	Refer to site plan	CLAY	UTP	UTP	UTP	UTP	UTP	1.8755	1.4444	29.8	3.3	300	27.6	2.7	1.48	5.0			
•••••••		be reproduced in full.							** Gauge Wet I	Densities outside	of the calibrate	ed range of 1.72	8 to 2.756 t/m ³	are not accredite	d and are outsi	de the laborator	ies scope of accr	editation.		

AKL2016_0634LAF Rev.A

45 Station Road

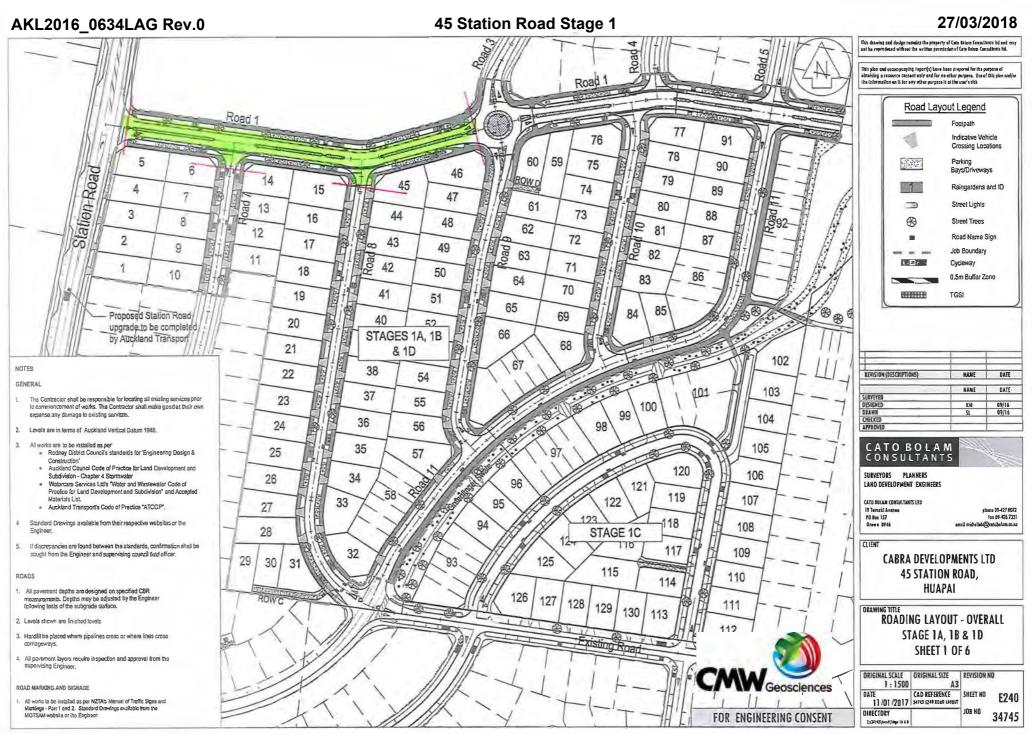
17/07/2018



CMWGeos	sciences	LF14 Rev.8 Dynamic Cone Penetration (DCP) Test Report NZS 4402: Test 6.5.2: 1988										
Report No:		AKL2016_0634LAG	Rev.0									
Project Name:		45 Station Road - S	tage 1			Auckland Labor CMW Geoscien	ces (NZ) Limited					
Project Location:		Ниарі				-	ermark Drive, Rosedale, NZ 0632 , Albany, Auckland, NZ 0752					
Project Number:		AKL2016_0634				Phone: +64 (09		10, 112 07 52				
Test Date:		23/03/2018				Testing Locatio	ns Selected By:		CMW Field Staf	Ť		
Tested By:		ММС/ЈМЈ										
Client:		Cabra				Ó	not acc the sco	idicated as redited are outside pe of the		es are not accredited the scope of the		
Client Address:		19 Tamariki Ave, O	rewa 0931			ACCREDITED LA	inhouse in the second	ory's accreditation	laboratory's	accreditation		
Client Reference:												
Test No.		1		2		3		4		5		
Test Location	Ro	ad 1	Ro	ad 1	Ro	ad 1	Road 1 Roa			ad 1		
Chainage & Offset	CH 20) Right	СН 3	0 Left	CH 40) Right	CH 5	0 Left	CH 60) Right		
Material & Layer:	CLA	Y / SG	CLAY	r / SG	CLA	Y / SG	CLAY	(/ SG	CLAY	(/ SG		
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*		
0 - 100	2	4	4	8	4	8	2	4	3	6		
100 - 200	4	8	2	4	3	6	3	6	3	6		
200 - 300	3	6	3	6	3	6	2	4	5	10		
300 - 400	3	6	3	6	3	6	2	4	5	10		
400 - 500	3	6	4	8	4	8	3	6	7	15		
500 - 600	5	10	5	10	3	6	4	8	9	20		
600 - 700	7	15	5	10	2	4	4	8	9	20		
700 - 800	15+	20+	6	13	3	6	5	10	9	20		
800 - 900												
900 - 1000												
Test No.		6		7		8		9		10		
Test Location	-	ad 1		ad 1		ad 1		ad 1		ad 1		
Chainage & Offset Material & Layer:		Ο Left Y / SG) Right (/ SG		0 Left Y / SG		0 Right (/ SG		10 Left / / SG		
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*		
0 - 100	4	8	2	4	5	10	5	10	7	15		
100 - 200	2	4	2	4	8	18	6	13	6	13		
200 - 300	1	2	3	6	5	10	4	8	7	15		
300 - 400	2	4	3	6	4	8	3	6	7	15		
400 - 500	4	8	2	4	5	10	4	8	7	15		
500 - 600	4	8	4	8	8	18	5	10	6	13		
600 - 700	4	8	2	4	6	13	4	8	4	8		
700 - 800	4	8	4	8	7	15	4	8	5	10		
800 - 900												
900 - 1000												
Prepared by:	JLM			Date:	26/03/2018		*Equivalent Cl	3R values calculated	ly be reproduced i using AUSTROADS (3, For Fine Grained	2010) Guide to		
Checked by:	JLM			Date:	27/03/2018				ined cohesive soils o			
Authorised Signatory:	IMI			Date:	27/03/2018				Page 1 of 4			

CMWGeos	ciences		LF14 Rev.8	Dynami		Penetrat 4402: Test 6.5.2:		P) Test R	est Report						
Report No:		AKL2016_0634LAG	Rev.0												
Project Name:		45 Station Road - S	tage 1			Auckland Labor CMW Geoscien	ces (NZ) Limited								
Project Location:		Ниарі				-	ermark Drive, Rosedale, NZ 0632 , Albany, Auckland, NZ 0752								
Project Number:		AKL2016_0634				Phone: +64 (09		10, 112 07 52							
-		_													
Test Date:		23/03/2018				Testing Locatio	ns Selected By:		CMW Field Staf	t					
Tested By:		MMC/JMJ													
Client:		Cabra					not acc	dicated as redited are outside		es are not accredited the scope of the					
Client Address:		19 Tamariki Ave, O	rewa 0931			ACCREDITED LA		pe of the ory's accreditation	laboratory's	accreditation					
Client Reference:															
Test No.	-	11	1	.2	:	13	1	.4	1	15					
Test Location	Ro	ad 1	Roa	ad 1	Ro	ad 1	Roa	ad 1	Roa	ad 1					
Chainage & Offset	CH 12	0 Right	CH 13	30 Left	CH 14	0 Right	CH 15	i0 Left	CH 16	0 Right					
Material & Layer:	CLA	Y / SG	CLAY	r / SG	CLA	Y / SG	CLAY	′ / SG	CLAY	(/ SG					
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*					
0 - 100	5	10	9	20	4	8	5	10	4	8					
100 - 200	5	10	4	8	4	8	4	8	4	8					
200 - 300	5	10	3	6	4	8	4	8	4	8					
300 - 400	6	13	4	8	5	10	4	8	3	6					
400 - 500	4	8	4	8	2	4	3	6	3	6					
500 - 600	6	13	6	13	5	10	4	8	4	8					
600 - 700	4	8	4	8	5	10	3	6	6	13					
700 - 800	4	8	4	8	5	10	5	10	6	13					
800 - 900															
900 - 1000															
Test No.	:	16	1	.7		18	1	.9	2	20					
Test Location	Ro	ad 1	Roa	ad 1	Ro	ad 1	Roa	ad 1	Roa	ad 1					
Chainage & Offset		0 Right		30 Left		0 Right		0 Right		0 Left					
Material & Layer:	CLA	Y / SG	CLAY	r / SG	CLA	Y / SG	CLAY	' / SG	CLAY	(/ SG					
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*					
0 - 100	4	8	4	8	5	10	6	13	4	8					
100 - 200	4	8	4	8	3	6	5	10	2	4					
200 - 300	4	8	3	6	4	8	4	8	2	4					
300 - 400 400 - 500	4	8	4	8	3	6	5	10	3	6 8					
400 - 500 500 - 600	3	8	4	8		8	4	8	6	8					
600 - 700	3	6	5	8	4	6	4 5	8	4	8					
700 - 800	4	8	10	20+	5	10	4	8	6	13					
800 - 900	· · ·														
900 - 1000															
		1	l	1	l	1	This	report should on	ly be reproduced i	n full					
Prepared by:	JLM			Date:	26/03/2018		*Equivalent CE Pavement Technol	BR values calculated ogy Part 2, Figure 5	l using AUSTROADS (.3, For Fine Grained	2010) Guide to Cohesive Soils, and					
Checked by:	JLM			Date:	27/03/2018		are	relevant to fine gra	ined cohesive soils o	only.					
Authorised Signatory:	IMI			Date:	27/03/2018				Page 2 of 4						

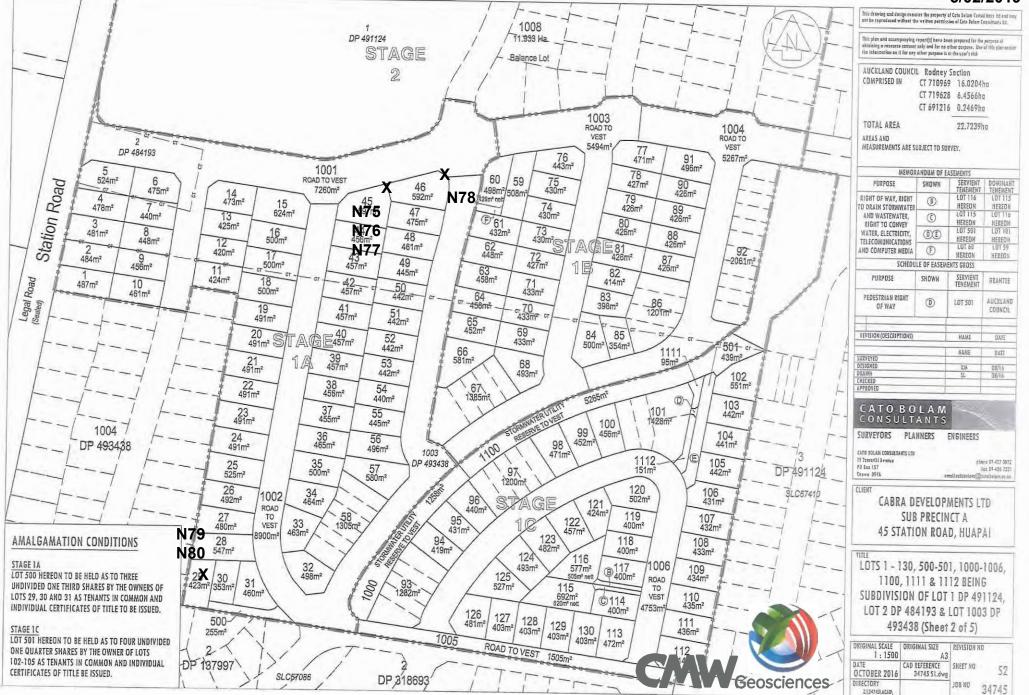
CMWGeos	ciences		LF14 Rev.8	Dynami		Cone Penetration (DCP) Test Report										
Report No:		AKL2016_0634LAG	Rev.0													
Project Name:		45 Station Road - St	tage 1			Auckland Labo CMW Geoscier	es (NZ) Limited									
Project Location:		Huapi				-	ermark Drive, Ro 5. Albany. Aucklai		!							
Project Number:		AKL2016_0634				Phone: +64 (09	i, Albany, Auckland, NZ 0752) 4144 632									
Test Date:		23/03/2018				Testing Lesstie	ons Selected By: CMW Field Staff									
							ins selected by.	CMW Field Staff								
Tested By:		MMC/JMJ					Tauta in									
Client:		Cabra					not acc the sco	dicated as redited are outside se of the		the scope of the						
Client Address:		19 Tamariki Ave, Or	rewa 0931			ACCREDITED LA	BORATORY laborato	ry's accreditation	laboratory's	accreditation						
Client Reference:																
Test No.	2	21	2	22	-	23										
Test Location	Road 1 In	tersection	Ro	ad 7	Ro	ad 8										
Chainage & Offset	CH 220) Centre	CH 15	Centre	CH15	Centre										
Material & Layer:	CLAY	(/ SG	CLAY	(/ SG	CLA	Y / SG										
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*						
0 - 100	2	4	3	6	8	18										
100 - 200	3	6	3	6	2	4										
200 - 300	3	6	3	6	3	6										
300 - 400	3	6	3	6	4	8										
400 - 500	4	8	7	15	3	6										
500 - 600	4	8	10+	20+	4	8										
600 - 700	4	8			3	6										
700 - 800	4	8			4	8										
800 - 900																
900 - 1000																
Test Loostian																
Test Location Chainage & Offset																
Material & Layer:																
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*						
0 - 100																
100 - 200																
200 - 300																
300 - 400																
400 - 500																
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900 - 1000	<u> </u>		l		<u> </u>		τ ι.,	report should	ly be reproduced.	n full						
Prepared by:	JLM			Date:	26/03/2018		*Equivalent Cl Pavement Techno	3R values calculate logy Part 2, Figure !	Ily be reproduced i d using AUSTROADS (5.3, For Fine Grained	2010) Guide to Cohesive Soils, and						
Checked by:	JLM			Date:	27/03/2018				ained cohesive soils o							
Authorised Signatory:	JMJ			Date:	27/03/2018				Page 3 of 4							



														Phone: +64 (0						
roject:		45 Station Road, Huapai												Test Metho			Notes:			
roject No:		AKL2016_0634												NZS 4402.2.			Solid Densit	y:		Assumed
ocation:		Ниараі												NZS 4407.4.	2.2:2015		0	ations Selecte	,	CMW Field Staff
eport No:		AKL2016_0634LAH Rev.0												NZGS:Augus	t 2001		 Blade siz 	ze of 19mm ι	sed.	
eport Date:		4/02/2019																		
lient:		Cabra Developments Limited													Ġ					s marked * are not accredite the scope of the laborator
lient Address	:																			accreditation
lient Referen	ce:														ACCREE	ITED LABO	RATORY			
			Van	e ID	In-situ Vane Shear Strengths						Field and	Laboratory To	esting Data							
ate Sampled	Sample No.	Test Location*	Soil Description*	Head #	Blade # ①	Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m ³) **	Gauge Dry Density (t/m³)	Gauge Water Content (%)	Gauge Air Voids (%)	•	Oven Water Content (%)	Solid Density (t/m ³) *	Oven Dry Density (t/m ³)	Calculated Air Voids (%) *	Comments
7/05/2018	N75	Refer to site plan	LS CLAY	1620	1620	UTP	UTP	UTP	133	133+	1.73	1.27	36.2	6.6	300		2.70			No sample taken
	N76	Refer to site plan	LS CLAY	1620	1620	UTP	147	133	133	138+	1.70	1.26	34.9	9.1	300		2.70			No sample taken
	N77	Refer to site plan	LS CLAY	1620	1620	UTP	147	147	133	142	1.73	1.27	36.9	8.6	300		2.70			No sample taken
9/05/2018	N78	Refer to site plan	LS CLAY	1620	1620	UTP	UTP	UTP	UTP	UTP	1.86	1.36	36.0	4.2	300	39.2	2.70	1.34	-2.0	Retest of N75/76/77
25/01/2019	N79	Refer to site plan	CLAY	1589	1589	UTP	UTP	UTP	UTP	UTP	1.82	1.57	33.1	4.0	300	32.7	2.70	1.38	4.4	
	N80	Refer to site plan	CLAY	1589	1589	UTP	UTP	185	139	162+	1.83	1.42	28.6	6.9	300	28.1	2.70	1.42	6.8	
		e reproduced in full.									** Gauge Wet	Donsitios outsid	of the calibrat	ed range of 1.72	28 to 2 756 t/m ³	are not accredit	ed and are out	ide the laborate	ries scope of ac	reditation

45 Station Road

5/02/2019



Page 2 of 2

CONVIGEosciences			ACT HAMMER 1 ANDARD: ASTM D5874:2016 FI				
Project:	45 Station Road, Huapai			Auckland Labo	oratory		
Project No:	AKL2016_0634			CMW Geoscie	nces (NZ) Ltd Pa liermark Drive, I		527
Location:	Huapai			PO Box 30020	6, Albany, Auckl		152
Report No:	AKL2016_0634LAI Rev.0			Phone: +64 (09	9) 4144 632		
Report Date:	25/06/2019			Testing Location	ons Selected By	:	CMW Field Staff
Client:	Cabra Developments Limited					ndicated as credited are outside	Measurements marked * are not
Client Address:				ACCREDITED LAB	the sco	credited are outside ope of the cory's accreditation	accredited and are outside of the scope of the laboratory's accreditation
Client Reference:		1	1				
Test No.:	Date Tested:	Test Location*	Impact Hammer No	Material**	Impact Value (IV)**	Inferred * CBR %	Notes
1	26/02/2019	Wall 27 + 800m	СІНО4	GAP65	32	72	
2	26/02/2019	Wall 27 + 800m	СІНО4	GAP65	35	86	
3	26/02/2019	Wall 27 + 800m	СІНО4	GAP65	30	63	
4	26/02/2019	Wall 3 + 800m	СІНО4	GAP65	28	55	
5	26/02/2019	Wall 3 + 800m	СІНО4	GAP65	29	59	
6	26/02/2019	Wall 3 + 800m	СІНО4	GAP65	30	63	
7	26/02/2019	Wall 3 + 800m	СІНО4	GAP65	32	72	
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** Measurements with a m	aximum particle size > 37.5m	m are outside of the scope of ASTM D5 scope of the labo	874:2016 Field Procedure A oratory's accreditation.	. Impact Values	on such mater	ials are not acc	redited and are outside of the
Created By:	JLM	Date:	1/03/2019		This re	eport should or	nly be reproduced in full
Checked By:	JLM	Date:	25/06/2019		* Inf	erred CBR Calc	ulation: CBR= IV ² x0.07
Authorised Signatory:	JLM	Date:	27/06/2019			Page	e 1 of 2

AKL2016-0634LAI Rev.0

45 Station Road

25/06/2019



Page 2 of 2

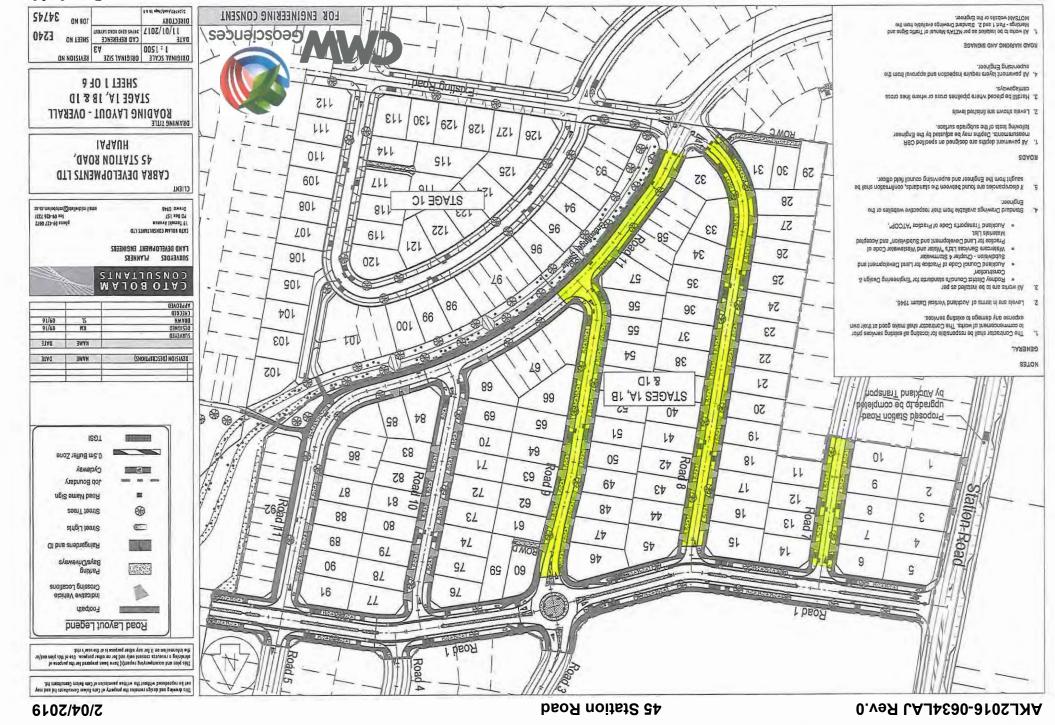
CMWGees	sciences		LF14 Rev.10	Dynam		Penetra 4402: Test 6.5.2:	tion (DC 1988	P) Test F	Report	
Report No:		AKL2016-0634LAJ F	Rev.0							
Project Name:		45 Station Road Sta	age 1			Auckland Labor CMW Geoscien	atory Ices (NZ) Ltd Part	nership		
Project Location:		Huapai				-	ermark Drive, Ro Albany, Aucklar			
-		AKL2016-0634				Phone: +64 (09	5, Albany, Auckland, NZ 0752 1) 4144 632			
Project Number:										
Test Date:		2/04/2019				Testing Locatio	ns Selected By:		CMW Field Staf	f
Tested By:		ммс					All tes	its reported		
Client:		Cabra				Ó	perfo	n have been med in accordance he laboratory's		R Values are not outside the scope of
Client Address:						ACCREDITED LA		of accreditation		's accreditation
Client Reference:										
Test No.		1		2		3		4		5
Test Location	Ro	ad 9	Roa	ad 9	Ro	ad 9	Roa	ad 9	Ro	ad 9
Chainage & Offset	СН	50R	СН	60L	СН	170R	СН	80L	СН	90R
Material & Layer:	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	8	18	1	2	1	2	3	6	2	4
100 - 200	3	6	3	6	3	6	3	6	2	4
200 - 300	4	8	2	4	3	6	3	6	3	6
300 - 400	4	8	3	6	4	8	3	6	5	10
400 - 500	5	10	5	10	5	10	5	10	5	10
500 - 600	4	8	5	10	5	10	5	10	4	8
600 - 700	5	10	5	10	4	8	4	8	5	10
700 - 800	4	8	4	8	4	8	4	8	4	8
800 - 900	5	10	4	8	4	8	4	8	4	8
900 - 1000										
Test No.	_	6		7		8		9		10
Test Location		ad 9 100L		ad 9 110R		ad 9 120L		ad 9 L30R		ad 9 140L
Chainage & Offset Material & Layer:		Y/SG		Y/SG		120L .Y/SG		Y/SG	-	Y/SG
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	1	2	2	4	4	8	2	4	4	8
100 - 200	1	2	3	6	3	6	2	4	3	6
200 - 300	3	6	2	4	3	6	3	6	4	8
300 - 400	4	8	2	4	2	4	4	8	4	8
400 - 500	4	8	3	6	4	8	3	6	2	4
500 - 600	4	8	4	8	1	2	5	10	2	4
600 - 700	5	10	7	15	3	6	4	8	2	4
700 - 800	4	8	4	8	5	10	4	8	2	4
800 - 900	5	10	6	13	5	10	5	10	6	13
900 - 1000				<u> </u>						
Prepared by: Checked by:	JLM			Date: Date:	4/04/2019 10/04/2019		*Equivalent Cl Pavement Techno	BR values calculated logy Part 2, Figure 5	ly be reproduced i lusing AUSTROADS (.3, For Fine Grained ined cohesive soils c	2010) Guide to Cohesive Soils, and
Authorised Signatory:	JLM			Date:	27/06/2019				Page 1 of 6	

CMWGeo	sciences		LF14 Rev.10	Dynami		Penetra 4402: Test 6.5.2:	tion (DC	P) Test F	Report	
Report No:		AKL2016-0634LAJ F	tev.0							
Project Name:		45 Station Road Sta	ige 1			Auckland Labor CMW Geoscien	atory ices (NZ) Ltd Part	nership		
Project Location:		Huapai				-	ermark Drive, Ro , Albany, Aucklar			
Project Number:		AKL2016-0634				Phone: +64 (09		10, 112 07 52		
-		2/04/2019				Testing Losstia				
Test Date:						Testing Locatio	ns Selected By:		CMW Field Staf	T
Tested By:		ммс					Allton	ats reported		
Client:		Cabra				á	herei	n have been med in accordance		R Values are not outside the scope of
Client Address:						ACCREDITED LA		he laboratory's of accreditation	the laboratory	's accreditation
Client Reference:										
Test No.	1	1	1	.2	-	13	1	14	1	15
Test Location	Ro	ad 9	Roa	ad 9	Ro	ad 9	Ro	ad 9	Roa	id 11
Chainage & Offset	CH:	150R	CH:	160L	CH	170R	CH:	180L	CH:	100R
Material & Layer:	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	2	4	2	4	1	2	3	6	1	2
100 - 200	4	8	3	6	2	4	4	8	4	8
200 - 300	2	4	2	4	3	6	4	8	10+	20+
300 - 400	5	10	4	8	3	6	4	8		
400 - 500	2	4	2	4	3	6	3	6		
500 - 600	4	8	3	6	4	8	4	8		
600 - 700	4	8	4	8	5	10	4	8		
700 - 800	3	6	4	8	4	8	4	8		
800 - 900	5	10	4	8	4	8	4	8		
900 - 1000										
Test No.		.6		.7		18		19		20
Test Location		d 11		d 11 80R		id 11		id 11		ad 8 250R
Chainage & Offset Material & Layer:		90L Y/SG	-	Y/SG		170L Y/SG		60R Y/SG	-	Y/SG
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	4	8	0	0	4	8	2	4	2	4
100 - 200	5	10	2	4	4	8	2	4	16	20+
200 - 300	6	13	3	6	5	10	2	4		
300 - 400	5	10	4	8	3	6	2	4		
400 - 500	6	13	2	4	3	6	3	6		
500 - 600	7	15	1	2	3	6	4	8		
600 - 700	4	8	3	6	3	6	4	8		
700 - 800	4	8	3	6	3	6	9	20		
800 - 900	5	10	4	8	3	6	8	18		
900 - 1000										
Prepared by: Checked by:	JLM			Date: Date:	4/04/2019		*Equivalent Cl Pavement Techno	3R values calculated logy Part 2, Figure 5	ly be reproduced i lusing AUSTROADS (.3, For Fine Grained ined cohesive soils c	2010) Guide to Cohesive Soils, and
Authorised Signatory:	JLM JLM			Date:	10/04/2019 27/06/2019		ale	. Leven to me gld	Page 2 of 6	

CMWGeos	sciences		LF14 Rev.10	Dynam		Penetra 4402: Test 6.5.2:	tion (DC	P) Test F	Report	
Report No:		AKL2016-0634LAJ F	lev.0							
Project Name:		45 Station Road Sta	ige 1			Auckland Labor CMW Geoscier	ratory Ices (NZ) Ltd Part	nership		
Project Location:		Huapai				-	ermark Drive, Ro , Albany, Aucklar			
Project Number:		AKL2016-0634				Phone: +64 (09				
Test Date:		2/04/2019				Tosting Locatio	ns Salastad By:		CMW Field Staf	f
							ns Selected By:			1
Tested By:		ММС						sts reported		
Client:		Cabra				Ó	perfo	n have been rmed in accordance the laboratory's	accredited and are	R Values are not outside the scope of
Client Address:						ACCREDITED LA		of accreditation	the laboratory	's accreditation
Client Reference:			-		-					
Test No.	2	!1	2	22	:	23	2	24	2	25
Test Location	Ro	ad 8	Roa	ad 8	Ro	ad 8	Ro	ad 8	Ro	ad 8
Chainage & Offset	CH	240L	CH2	230R	СН	220L	CH2	210R	CH	200L
Material & Layer:	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	3	6	3	6	2	4	2	4	3	6
100 - 200	4	8	10+	20+	4	8	3	6	3	6
200 - 300	3	6			3	6	4	8	3	6
300 - 400	3	6			3	6	1	2	3	6
400 - 500	3	6			2	4	3	6	3	6
500 - 600	9	20			3	6	3	6	2	4
600 - 700	2	4			3	6	1	2	3	6
700 - 800	1	2			3	6	4	8	2	4
800 - 900	2	4			3	6	3	6	3	6
900 - 1000										
Test No.		26		27		28		29		80
Test Location	-	ad 8		ad 8 180L	-	ad 8 170R		ad 8 160L		ad 8
Chainage & Offset Material & Layer:	-	Y/SG		Y/SG		I/UR IY/SG		Y/SG		L50R Y/SG
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	2	4	3	6	4	8	3	6	2	4
100 - 200	4	8	4	8	4	8	4	8	2	4
200 - 300	2	4	4	8	3	6	3	6	3	6
300 - 400	4	8	3	6	3	6	4	8	3	6
400 - 500	4	8	4	8	3	6	3	6	3	6
500 - 600	3	6	3	6	3	6	4	8	3	6
600 - 700	3	6	3	6	3	6	5	10	3	6
700 - 800	3	6	3	6	3	6	3	6	3	6
800 - 900	3	6	4	8	3	6	3	6	3	6
900 - 1000										
Prepared by: Checked by:	JLM			Date: Date:	4/04/2019 10/04/2019		*Equivalent Cl Pavement Techno	BR values calculated logy Part 2, Figure 5	ly be reproduced i l using AUSTROADS (.3, For Fine Grained ined cohesive soils c	2010) Guide to Cohesive Soils, and
Authorised Signatory:	JLM			Date:	27/06/2019				Page 3 of 6	

CMWGeos	sciences		LF14 Rev.10	Dynam		Penetra 4402: Test 6.5.2:	tion (DC 1988	P) Test F	Report	
Report No:		AKL2016-0634LAJ F	lev.0							
Project Name:		45 Station Road Sta	ige 1			Auckland Labor CMW Geoscien	atory ces (NZ) Ltd Part	nership		
Project Location:		Huapai				-	ermark Drive, Ro , Albany, Aucklar			
Project Number:		AKL2016-0634				Phone: +64 (09				
Test Date:		2/04/2019				Testing Locatio	nc Coloctod Dur		CMW Field Staf	÷
							ns Selected By:			I
Tested By:		ММС					All tes	s reported		
Client:		Cabra				ó N	herein	have been ned in accordance	accredited and are	
Client Address:						ACCREDITED LA		e laboratory's of accreditation	the laboratory	's accreditation
Client Reference:			-		-					
Test No.	3	31	3	32	3	33	3	34	3	35
Test Location	Ro	ad 8	Roa	ad 8	Ro	ad 8	Ro	ad 8	Ro	ad 8
Chainage & Offset	CH	140L	CH1	130R	CH	120L	CH	L10R	CH	100L
Material & Layer:	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	2	4	1	2	2	4	2	4	3	6
100 - 200	2	4	4	8	1	2	2	4	2	4
200 - 300	2	4	2	4	1	2	2	4	4	8
300 - 400	2	4	1	2	2	4	3	6	3	6
400 - 500	1	2	2	4	2	4	1	2	4	8
500 - 600	2	4	2	4	1	2	2	4	4	8
600 - 700	2	4	1	2	1	2	2	4	4	8
700 - 800	1	2	1	2	1	2	2	4	4	8
800 - 900	3	6	1	2	1	2	3	6	4	8
900 - 1000										
Test No.		16 		37		38		89 		10
Test Location		ad 8 90R		ad 8 80L		ad 8 70R		ad 8 60L		ad 8 50R
Chainage & Offset Material & Layer:		y/sg		Y/SG		Y/SG		Y/SG		Y/SG
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	4	8	2	4	3	6	5	10	4	8
100 - 200	4	8	2	4	3	6	4	8	3	6
200 - 300	4	8	2	4	3	6	3	6	3	6
300 - 400	4	8	3	6	3	6	3	6	3	6
400 - 500	4	8	3	6	3	6	3	6	3	6
500 - 600	4	8	3	6	3	6	6	13	3	6
600 - 700	6	13	3	6	3	6	5	10	3	6
700 - 800	5	10	3	6	3	6	4	8	2	4
800 - 900	5	10	3	6	3	6	6	13	3	6
900 - 1000										
Prepared by: Checked by:	JLM JLM			Date: Date:	4/04/2019 10/04/2019		*Equivalent Cl Pavement Techno	BR values calculated logy Part 2, Figure 5	ly be reproduced i lusing AUSTROADS (.3, For Fine Grained ined cohesive soils c	2010) Guide to Cohesive Soils, and
Authorised Signatory:	JLM			Date:	27/06/2019				Page 4 of 6	

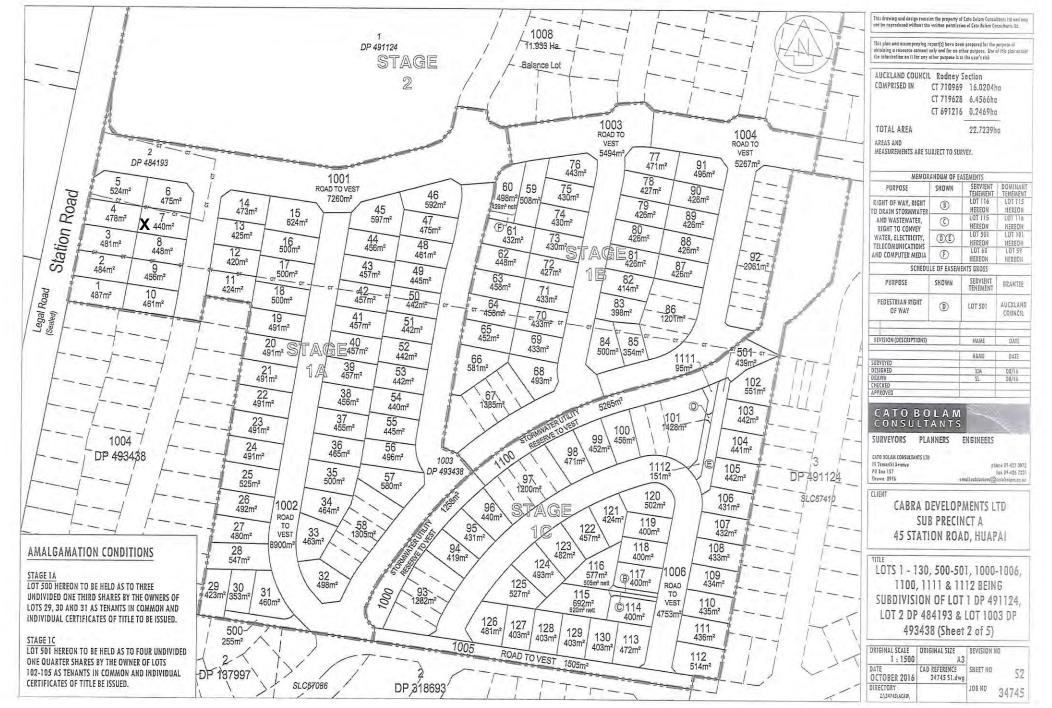
CMWGeos	clences		LF14 Rev.10	Dynam		Penetra 4402: Test 6.5.2:	tion (DC 1988	P) Test F	Report	
Report No:		AKL2016-0634LAJ F	Rev.0							
Project Name:		45 Station Road Sta	age 1			Auckland Labor CMW Geoscien	atory ices (NZ) Ltd Part	nership		
Project Location:		Huapai				-	ermark Drive, Ro , Albany, Aucklar			
Project Number:		AKL2016-0634				Phone: +64 (09				
Test Date:		2/04/2019				Testing Locatio	nc Coloctod Dur		CMW Field Staf	÷
							ns Selected By:			I
Tested By:		ММС					All tes	ts reported		
Client:		Cabra				Ó	herein	have been med in accordance	accredited and are	R Values are not outside the scope of
Client Address:						ACCREDITED LA		ne laboratory's of accreditation	the laboratory	's accreditation
Client Reference:			-		-					
Test No.	2	11	4	12	4	13	2	14	2	15
Test Location	Roa	ad 8	Roa	ad 8	Roa	ad 7	Roa	ad 7	Ro	ad 7
Chainage & Offset	CH	40L	СН	30R	СН	10R	CH	20L	СН	30R
Material & Layer:	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	4	8	15	20+	4	8	2	4	2	4
100 - 200	3	6			4	8	3	6	3	6
200 - 300	3	6			5	10	3	6	5	10
300 - 400	3	6			2	4	2	4	5	10
400 - 500	4	8			1	2	3	6	5	10
500 - 600	3	6			1	2	3	6	7	15
600 - 700	3	6			2	4	4	8	5	10
700 - 800	4	8			2	4	3	6	5	10
800 - 900	4	8			4	8	3	6	4	8
900 - 1000										
Test No.		16		17		48		19		50
Test Location	-	ad 7		ad 7		ad 7		ad 7		ad 7
Chainage & Offset Material & Layer:		40L Y/SG		50R Y/SG		160L .Y/SG		70R Y/SG		Y/SG
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	3	6	3	6	3	6	2	4	2	4
100 - 200	3	6	4	8	3	6	3	6	3	6
200 - 300	3	6	4	8	3	6	3	6	3	6
300 - 400	3	6	4	8	5	10	3	6	3	6
400 - 500	3	6	4	8	4	8	3	6	4	8
500 - 600	5	10	4	8	6	13	4	8	4	8
600 - 700	4	8	4	8	5	10	4	8	5	10
700 - 800	3	6	4	8	4	8	4	8	4	8
800 - 900	4	8	5	10	4	8	4	8	5	10
900 - 1000										
Prepared by: Checked by:	JLM JLM			Date: Date:	4/04/2019 10/04/2019		*Equivalent Cl Pavement Techno	BR values calculated logy Part 2, Figure 5	ly be reproduced i lusing AUSTROADS (.3, For Fine Grained ined cohesive soils c	2010) Guide to Cohesive Soils, an
Authorised Signatory:	JLM			Date:	27/06/2019			-	Page 5 of 6	



9 to 9 9969

CM	WGeosci	LF11 Rev.12 Soil Fi	eld Density NDM D	irect Tra	insmis	sion w	vith V	SS Re	eport	(Cohe	esive S	Soils)		Auckland Labo CMW Geoscie Building C, 9 F PO Box 30020 Phone: +64 (0	ences (NZ) Ltd Piermark Drive, 06, Albany, Auc	, Rosedale, NZ				
Project:		45 Station Road												Test Metho	ds:	Notes:	Solid Densit	ty:		Assumed
Project No:		AKL2016-0634												NZS 4402 19	986 Test 2.1		Solid Densit	ty Data Sourc	e:	N/A
Location:		Huapai												NZS 4407 20)15 Test 3.1		Testing Loca	ations Select	ed By:	CMW Field Staff
Report No:		AKL2016-0634LAK Rev.0												NZS 4407 20)15 Test 4.2					
Report Date:		11/02/2020												NZS 4407 20)15 Test 4.3					
Client:		Cabra Developments Limited												NZGS:Augus	t 2001		1 Blade size	e of 19mm use	d.	
					Var	ne ID		n-situ Va	ane Shear	Strengt	ns				EDITED LABO	RATORY lab	sts indicated a t accredited ar e scope of the poratory's accre Data	re outside		s marked * are not accredited e the scope of the laboratories accreditation
Date Sampled	Sample No.	Test Location*	Soil Description*	Solid Density (t/m ³) *	Head #	Blade #		Test 2 (kPa)		Test 4 (kPa)	Ave.	Gauge Wet Density (t/m ³) **	Gauge Dry Density (t/m³)		Gauge Air Voids (%)	Gauge Probe Depth (mm)	e Oven Water	Oven Dry Density (t/m ³)	Oven Calculated Air Voids (%) *	Comments
11/12/2019	N81	Batter extension	CLAY minor Gravel	2.70	2080	2080	UTP	174	222+	177	191+	1.90	1.46	5 29.9	2	2 300	25.9	9 1.51	. 5	
This report sh	nould only b	be reproduced in full.	1	1				1	1	1	I	** Gauge Wet	l Densities outsic	l de of the calibrate	l ed range of 1.72	1 28 to 2.756 t/m³	l are not accredit	ted and are outs	l ide the laborato	ies scope of accreditation.
Created By:			Date:					2/2019												
Checked By:			Date:					2/2020												
Authorised Si	ignatory:	WL	Date:				11/02	2/2020												Page: 1 of 2

11/02/2020



Appendix E: Producer Statements



17 March 2020

Document Ref: AKL2016_0634AL Rev.0

Cabra Developments Limited PO Box 197 Orewa 0946

Attention: Duncan Unsworth

Dear Sir

RE: CONSTRUCTION REVIEW FOR TIMBER POLE CANTILEVER AND KEYSTONE RETAINING WALLS- CONSENT BCO10092414-1-B

45 STATION ROAD, HUAPAI (53 STATION ROAD, HUAPAI)

CMW Geosciences (CMW) has visited the site at 45 Station Road, Huapai now legally described as 53 Station Road, Huapai Lot 2 DP 533552 on several occasions between April 2017 and February 2020 to observe the site works for the construction of timber pole cantilever retaining walls 4A (beyond chainage 170m), 7A, 7B, 7C, 9, 10, 11A and keystone retaining walls 3, 8 and 27.

Our work has included review of the following documents and drawings:

- Conditions of Auckland Council Building Consent referenced BCO10092414-1-B issued 5 May 2017;
- Consented construction drawings, prepared by CMW Geosciences, referenced AKL2016_0634 Stage 1A & 1B Keystone Wall figures 201 to 203 and Stage 1A & 1B Timber Pole Wall figures 204-206 dated 23 March 2017;
- Geotechnical Report for Stage 1A & 1B Retaining Wall Designs 45 Station Road, Huapai prepared by CMW Geosciences, referenced AKL2016_0634AD Rev.0, dated 23 March 2017.

The site works observed and/or tested by CMW staff for the Keystone Retaining Walls incorporated:

- Assessment of soil strengths at subgrade and retained ground;
- Drainage placement and outlets;
- Backfill aggregate quality and placement;
- Geogrid type, orientation and placement.

Our testing demonstrated vane shear strengths in the subgrade and retained ground that met design specifications. Drainage placement and outlets were observed by CMW.

During February 2020 a few layers of wall 27 were removed and sleeves for fence posts were installed. Grids and compacted hardfill was then re-installed. The orientation and placement of grids were inspected by CMW

and met design specifications. Hardfill compaction was inspected using impact hammer tests; CIV values met design specifications ranging between 228 and 35.

The site works observed and/or tested by CMW staff for the <u>Timber Pole Cantilever Retaining Walls</u> incorporated:

- Measurement of pile hole depth, spacing and diameter;
- Assessment of soil strengths in the pile holes;
- Measurement of timber pole and rail sizes;
- Drainage placement and outlets;
- Confirmation of timber treatment levels.

Between April 2017 and February 2020 CMW inspected the construction of the timber pole cantilever retaining walls. Vane shear strengths in the bases and sides of the pile holes met design specifications, exceeding 70kPa in the 2 metre crust and in excess of 50kPa below 2 metres depth. Groundwater was encountered in some of the piles holes during construction and the contractors were advised to pump this out before concrete was poured.

During December 2019 it was observed that the bridging detail piles at the wall 11A and 11B connections had been drilled too close together. Two piles were subsequently pulled out and redrilled to the correct spacing and depth. Bridging details across the remaining pipe crossings were inspected and met design specifications.

The measurements, dimensions and drainage placement and outlets across the timber pole cantilever retaining walls were observed and met design specifications.

On the basis of our observations and testing, we consider that the site works have been undertaken in accordance with the approved Building Consent and related approved documentation described above, are in accordance with the requirements and/or recommendations of the geotechnical design report and provide the basis for our attached PS4 Construction Review producer statement.

CMW's site presence during construction for this project included periodic observations of specific elements of work as described herein. As we were not on site at all times during construction, we have relied on the Contractor's diligence, construction observations and their attached PS3 certification to ensure that the works have been carried out in accordance with:

- a) The approved Contract drawings and design details;
- b) The approved Contract specifications;
- c) Authorised Variations to (a) and (b) during the execution of the works;
- d) The conditions of Resource and Building Consents where applicable;
- e) The relevant Geotechnical Investigation reports, recommendations and site instructions;

and that all as-built information and other details provided to the Client and/or CMW are accurate and correct in all respects.

For and on behalf of CMW Geosciences

Knowles

Richard Knowles Principal Geotechnical Engineer

- Distribution: 1 electronic copy to Cabra Developments Limited via email Original held at CMW Geosciences
- Attachments: Producer Statement Construction Review





PRODUCER STATEMENT – PS4 – CC (Guidance on use of Producer Statements (formerly page 2) is a	
CMW Geosciences (NZ) Limited Partnership	
(Construction Review Firm)	
TO: Cabra Developments Limited	
(Owner/Developer)	
TO BE SUPPLIED TO: Auckland Council (Building Consent Authority)	
IN RESPECT OF: The Construction of Timber Cantilever Retaining Walls	14 (part) 74 78 70 0 10 11 and
(Description of Building Work)
AT: Segmental Block Retaining Walls 3, 8 and 27 at 45 Station Road, Huap (Address)	
Town/City: Auckland LOT.2	DP 533552 SO
(Address)	
We CMW Geosciences (NZ) Limited Partnership have been engaged by (Construction Review Firm)	Cabra Developments Limited
To provide CM1 CM2 CM3 CM4 CM5 (Engineering Categor	ies) or 🗌 observation as per agreement with
owner/developer. Cabra Developments Limited	
orotheras described in CMW letter referenced AKL2016_0634AL Re (Extent of Engagement)	
in respect of clause(s) .B1 of the B	uilding Code for the building work described in
documents relating to Building Consent No. BCO10092414-1	
Building Consent Amendment(s) Nos. BCO10092414-1-B course of the works. We have sighted these Building Consents and the conc	itions of attached to them.
Authorised instructions/variations(s) Noor by the attached Schedule have been issued during the course of the w	orks.
On the basis of this review these review(s) and information supplied and on behalf of the firm undertaking this Construction Review, I believe o All or Part only of the building works have been completed in accord	n reasonable grounds that
Building Consent and Building Consent Amendments identified above, with r of the Building Code. I also believe on reasonable grounds that the persons the necessary competency to do so.	espect to Clause(s)B1 who have undertaken this construction review have
I, Richard Knowles (AC Author #2342) (Name of Construction Review Professional)	ng .160049 # 🔲 Reg Arch #
I am a member of: Bengineering New Zealand NZIA and hold the follow The Construction Review Firm issuing this statement holds a current policy of F \$200,000*.	ving qualifications BE (civil), CMEngNZ, CPEng Professional Indemnity Insurance no less than
The Construction Review Firm is a member of ACENZ:	A 4
	111
SIGNED BY Richard Knowles (AC Author #2342) (Name of Construction Review Professional)	(Signature)
CMW/ Goossionoos (NZ) Limited Destroyabin	Date 18/3/20
Note: This statement shall only be relied upon by the Building Consent Authority name Design Firm only. The total maximum amount of damages payable arising from this sta	tement and all other statements provided to the Building
Consent Authority in relation to this building work, whether in contract, tort or otherwise	(including negligence), is limited to the sum of \$200,000*.

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PRODUCER STATEMENT PS4

Building Code Clause(s). B1



17 March 2020

Document Ref: AKL2016_0634AN Rev.0

Cabra Developments Limited PO Box 197 Orewa 0946

Attention: Duncan Unsworth

Dear Sir

RE: CONSTRUCTION REVIEW FOR TIMBER POLE CANTILEVER AND SEGMENTAL BLOCK RETAINING WALLS- CONSENT BCO10285560

45 STATION ROAD, HUAPAI (53 STATION ROAD, HUAPAI)

CMW Geosciences (CMW) has visited the site at 45 Station Road, Huapai now legally described as 53 Station Road, Huapai Lot 200 DP 513781 and Lot 201 DP 513781 on several occasions between December 2019 and January 2020 to observe the site works for the construction of timber pole cantilever retaining walls 4A (approximate chainage 0m to 170m), 5 and 6 and segmental block retaining wall 28.

Our work has included review of the following documents and drawings:

- Conditions of Auckland Council Building Consent referenced BCO10285560 issued 10 June 2019;
- Consented construction drawings, prepared by CMW Geosciences, referenced AKL2016_0634 Timber Pole Retaining Wall Design and Segmental Block Wall with No-Fines Concrete figures 01-04 dated 23 April 2019;
- Geotechnical Report for Remaining Scope of Stage 1A & 1B Retaining Wall Designs 45 Station Road, Huapai prepared by CMW Geosciences, referenced AKL2016_0634AH Rev.1, dated 19 March 2019.

The site works observed and/or tested by CMW staff for the <u>Segmental Block Retaining Wall</u> incorporated:

- Assessment of soil strengths at subgrade and retained ground;
- Drainage placement and outlets;
- Foundation preparation for segmental block walls;
- Backfill No-Fines concrete placement
- Subsoil drainage installation;
- Compaction testing of engineered backfill during placement.

Our testing demonstrated vane shear strengths in the footing and retained soil of wall 28 averaged over 100kPa. No-fines concrete placement met design specifications. Drainage placement and outlets were observed.

The site works observed and/or tested by CMW staff for the <u>Timber Pole Cantilever Retaining Walls</u> incorporated:

- Measurement of pile hole depth, spacing and diameter;
- Assessment of soil strengths in the pile holes;
- Measurement of timber pole and rail sizes;
- Drainage placement and outlets;
- Confirmation of timber treatment levels.

Between early December 2019 and late January 2020 CMW inspected the construction of the 3 timber pole retaining walls 5, 6 and a section of 4a (approximately CH 0m to CH 170m). Vane shear strengths in the base and sides of all pile holes met design specifications with all exceeding 70kPa. Where groundwater was encountered in the pile holes, contractors were asked to pump it out before concrete was poured. Drainage outlets were observed to have been installed in every lot.

The measurements, dimensions and drainage placement across the timber pole cantilever retaining walls were observed and met design specifications.

On the basis of our observations and testing, we consider that the site works have been undertaken in accordance with the approved Building Consent and related approved documentation described above, are in accordance with the requirements and/or recommendations of the geotechnical design report and provide the basis for our attached PS4 Construction Review producer statement.

CMW's site presence during construction for this project included periodic observations of specific elements of work as described herein. As we were not on site at all times during construction, we have relied on the Contractor's diligence, construction observations and their attached PS3 certification to ensure that the works have been carried out in accordance with:

- a) The approved Contract drawings and design details;
- b) The approved Contract specifications;
- c) Authorised Variations to (a) and (b) during the execution of the works;
- d) The conditions of Resource and Building Consents where applicable;
- e) The relevant Geotechnical Investigation reports, recommendations and site instructions;

and that all as-built information and other details provided to the Client and/or CMW are accurate and correct in all respects.

For and on behalf of CMW Geosciences

Ry Knowles

Richard Knowles Principal Geotechnical Engineer

Distribution: 1 electronic copy to Cabra Developments Limited via email Original held at CMW Geosciences

Attachments: Producer Statement - Construction Review





PRODUCER STATEMENT – PS4 – CONSTRUCTION REVIEW

ISCUED DV. CMW Geosciences (NZ) Limited Partnership	() is available at <u>www.engineeringnz.org</u>)
(Construction Review TO: Cabra Developments Limited	
(Owner/Developer TO BE SUPPLIED TO: Auckland Council	
(Building Consent Auth IN RESPECT OF: The Construction of Timber Cantilever Retaining Wa (Description of Building)	^{onty)} ills 4A (part), 5 & 6 & Segmental Block Wall 28
AT: 45 Station Road, Huapai (53 Station Road, Huapai) (Address)	
Town/City: Auckland (Address)	DP ⁵¹³⁷⁸¹ SO
We CMW Geosciences (NZ) Limited Partnership have been engage (Construction Review Firm)	by Cabra Developments Limited
	egories) or observation as per agreement with
owner/developer. Cabra Developments Limited	
or otheras described in CMW letter referenced AKL2016_0634A	nt)
in respect of clause(s) .B1 of th	e Building Code for the building work described in
documents relating to Building Consent No. BCO10285560	and those relating to
Building Consent Amendment(s) Nos course of the works. We have sighted these Building Consents and the o	onditions of attached to them.
Authorised instructions/variations(s) Noor by the attached Schedule Thave been issued during the course of t	e works,
On the basis of this review these review(s) and information supp and on behalf of the firm undertaking this Construction Review, I believ All or Part only of the building works have been completed in acc	a on reasonable grounds that
Building Consent and Building Consent Amendments identified above, w of the Building Code. I also believe on reasonable grounds that the perso the necessary competency to do so.	th respect to Clauso(s) B1
I, Richard Knowles (AC Author #2342) (Name of Construction Review Professional)	PEng .160049 # Reg Arch #
I am a member of: Engineering New Zealand NZIA and hold the for The Construction Review Firm issuing this statement holds a current policy \$200,000*.	of Professional Indemnity Insurance no less than
The Construction Review Firm is a member of ACENZ:	n i
SIGNED BY Richard Knowles (AC Author #2342) (Name of Construction Review Professional)	(Signature)
ON BEHALF OF CMW Geosciences (NZ) Limited Partnership (Construction Review Fir	m) Date 18/3/20
Note: This statement shall only be relied upon by the Building Consent Authority na Design Firm only. The total maximum amount of damages payable arising from thi Consent Authority in relation to this building work, whether in contract, tort or other	amed above. Liability under this statement accrues to the

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PRODUCER STATEMENT PS4

The Country Club Huapai - Station Road, Huapai

Stage 1A As Builts



LOCATION DIAGRAM Scale 1:10,000

Plan No	Rev	Plan Title
		Asbuilts
9000	2	Final Contour Asbuilt Plan
9010	2	Cut And Fill Asbuilt Plan
9100	3	Road Asbuilt Plan Sheet 1 of 4
9101	2	Road Asbuilt Plan Sheet 2 of 4
9102	3	Road Asbuilt Plan Sheet 3 of 4
9103	3	Road Asbuilt Plan Sheet 4 of 4
9200	2	Waste Water Reticulation Asbuilt Plan Sheet 1 of 3
9201	2	Waste Water Reticulation Asbuilt Plan Sheet 2 of 3
9202	2	Waste Water Reticulation Asbuilt Plan Sheet 3 of 3
9300	2	Stormwater Asbuilt Plan Sheet 1 of 6
9301	2	Stormwater Asbuilt Plan Sheet 2 of 6
9302	2	Stormwater Asbuilt Plan Sheet 3 of 6
9303	2	Stormwater Asbuilt Plan Sheet 4 of 6
9304	2	Stormwater Asbuilt Plan Sheet 5 of 6
9305	2	Stormwater Asbuilt Plan Sheet 6 of 6
9400	2	Water Reticulation Asbuilt Plan Sheet 1 of 2
9401	2	Water Reticulation Asbuilt Plan Sheet 2 of 2





40353-DR-SU-0001-3

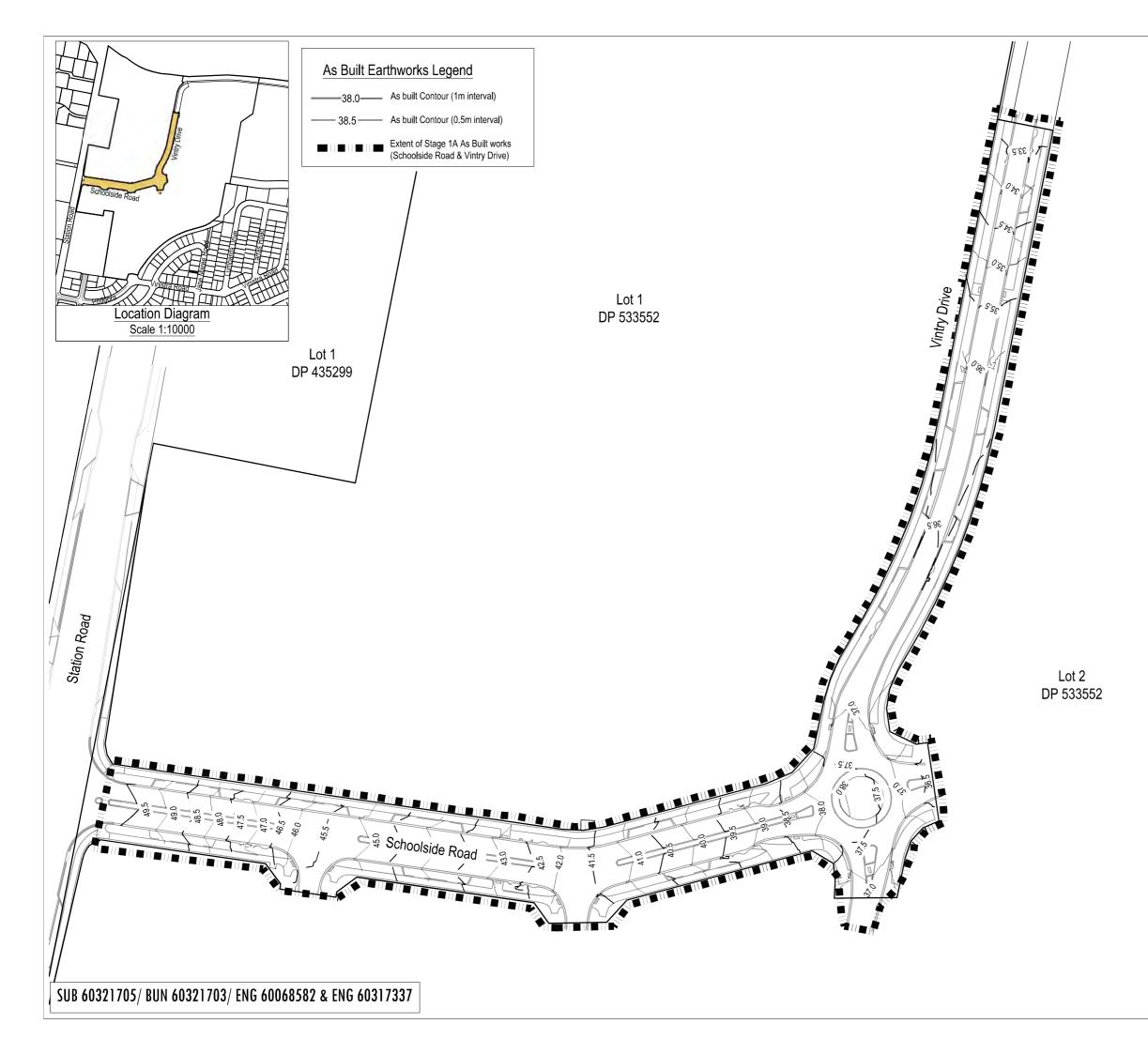
FEBRUARY 2019 DRAWING NO.

DATE

ORIGINAL SIZE

A3







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NOTES

- Contours are finished ground levels as surveyed at 21/02/2019. Contour interval of 0.5m 1. 2.
- Levels are in terms of LINZ Datum 1946. Coordinates are in terms of NZTM.
- 3.

I certify that these As-Built Plans are an accurate record of the works undertaken and that:

- * The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within ± 50mm.
- * The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.

Signed:

Registered Professional Surveyor

Date:	28/02/2019
Name:	Tom Lemon
	(09) 427 0072 catobolam@catobolam.co.nz



CATO BOLAM CONSULTANT PLANNERS | SURVEYORS | ENGINEERS | ENVIRONMENTAL

PROJECT

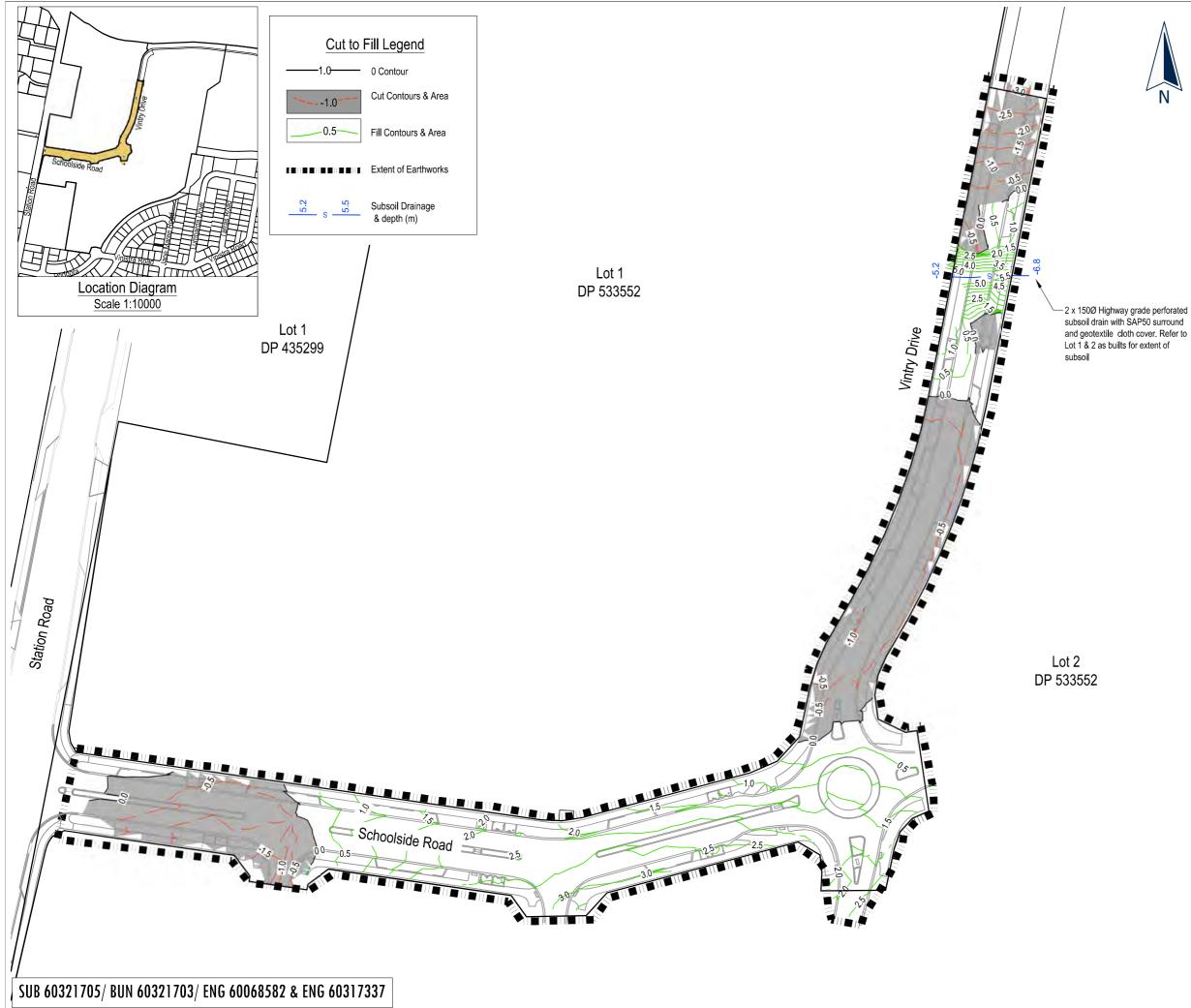
The Country Club Huapai Station Road Huapai

DRAWING TITLE

Stage 1A Final Contours As Built Plan

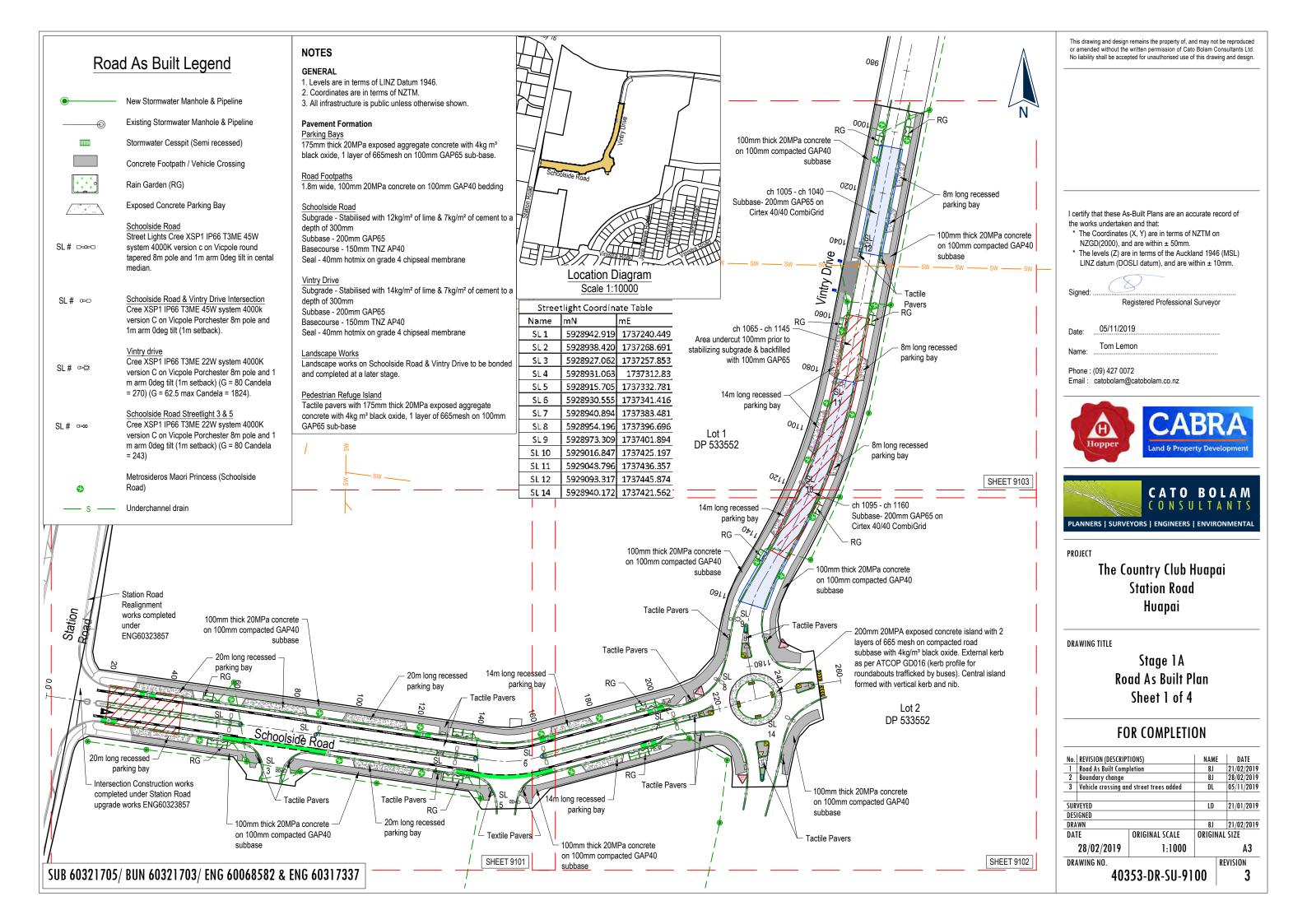
FOR COMPLETION

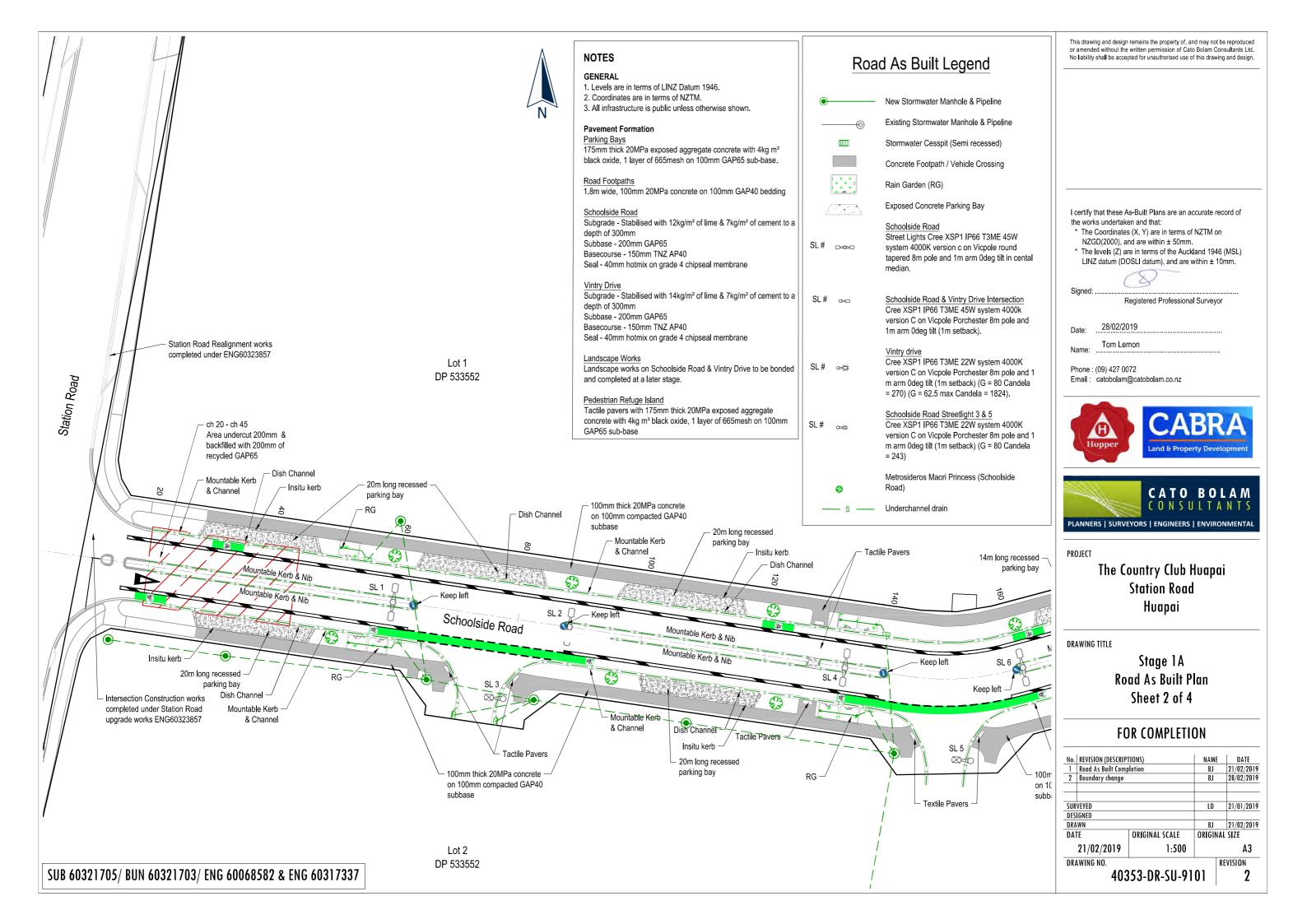
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DD	28/02/2019	1:1000		A3
DA	TE	ORIGINAL SCALE	ORIGI	NAL SIZE
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DES	GNED			
SUR	VEYED		LD	21/01/2019
2	Boundary change	BJ	28/02/2019	
1	Final Contours As B	suilt Completion	BJ	25/02/2019
Α	Final Contours As B	BJ	21/02/2019	
No.	REVISION (DESCRIPT	FIONS)	NAM	E DATE

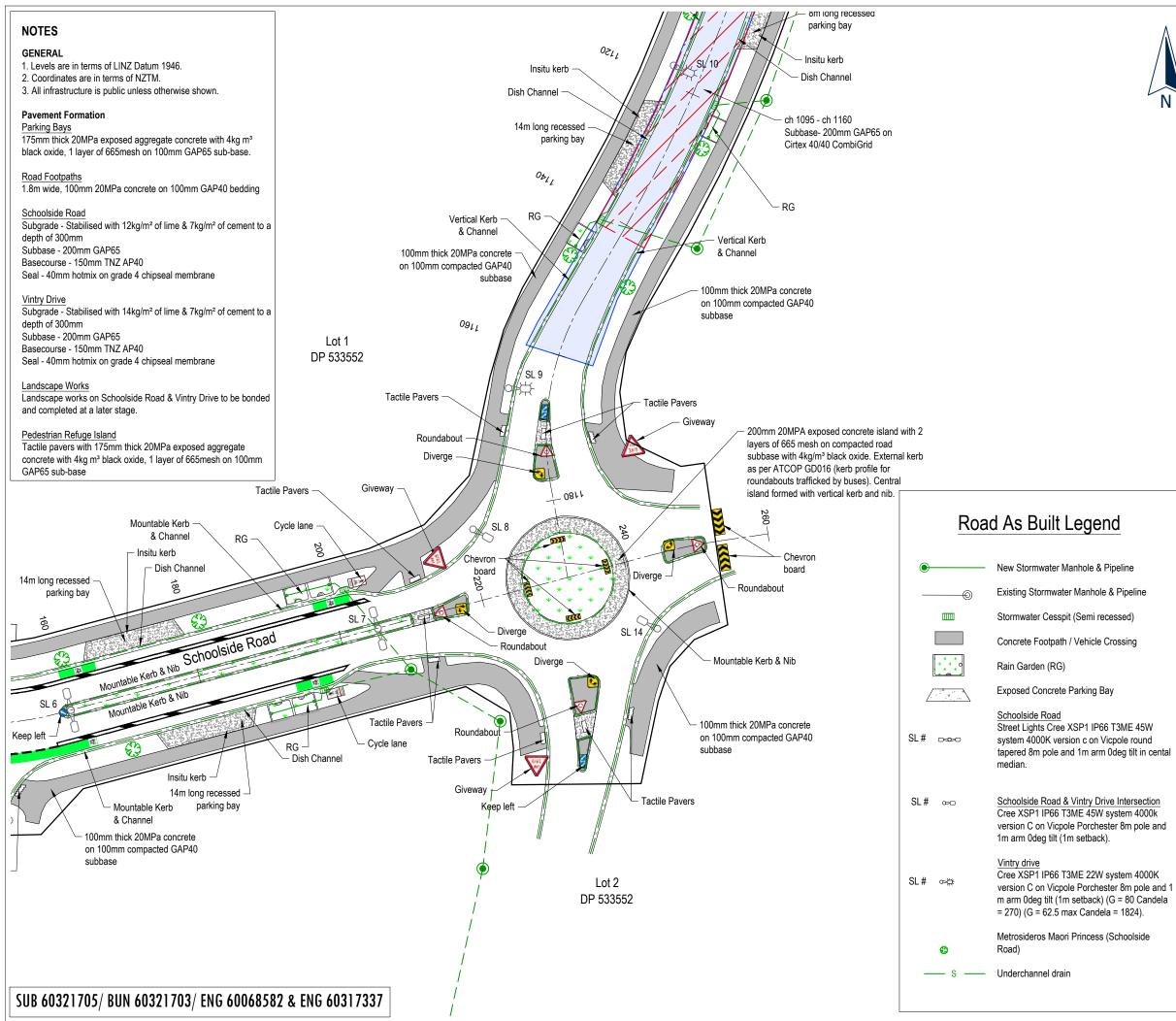




and geotextile cloth cover. Refer to







/	
L	

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I certify that these As-Built Plans are an accurate record of the works undertaken and that: * The Coordinates (X, Y) are in terms of NZTM on
NZGD(2000), and are within ± 50mm. * The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.
Signed:
Registered Professional Surveyor
Date:05/11/2019
Name:
Phone : (09) 427 0072 Email : catobolam@catobolam.co.nz
CABRA Land & Property Development
CATO BOLAM CONSULTANTS PLANNERS SURVEYORS ENGINEERS ENVIRONMENTAL
PROJECT
The Country Club Huapai
Station Road

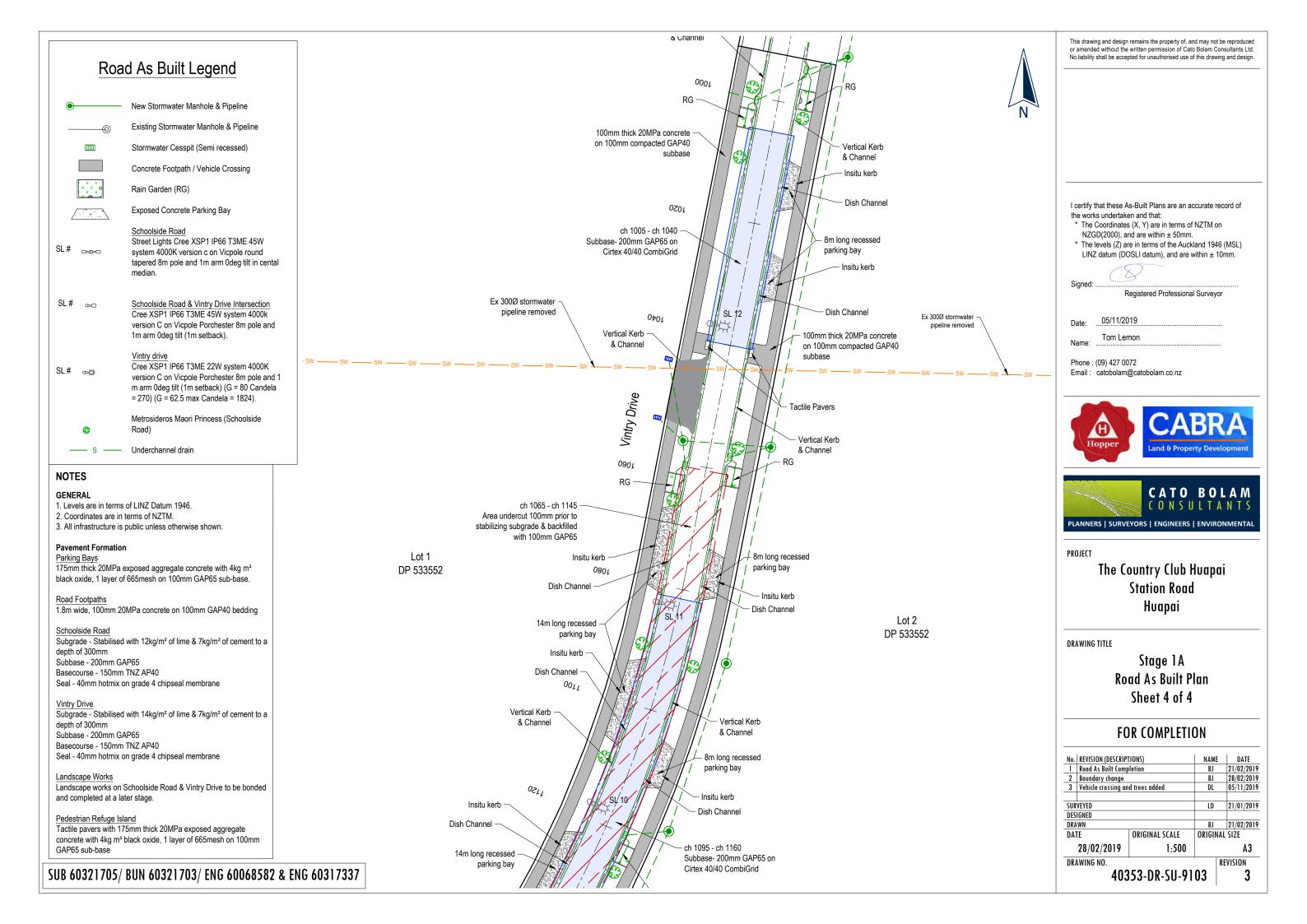
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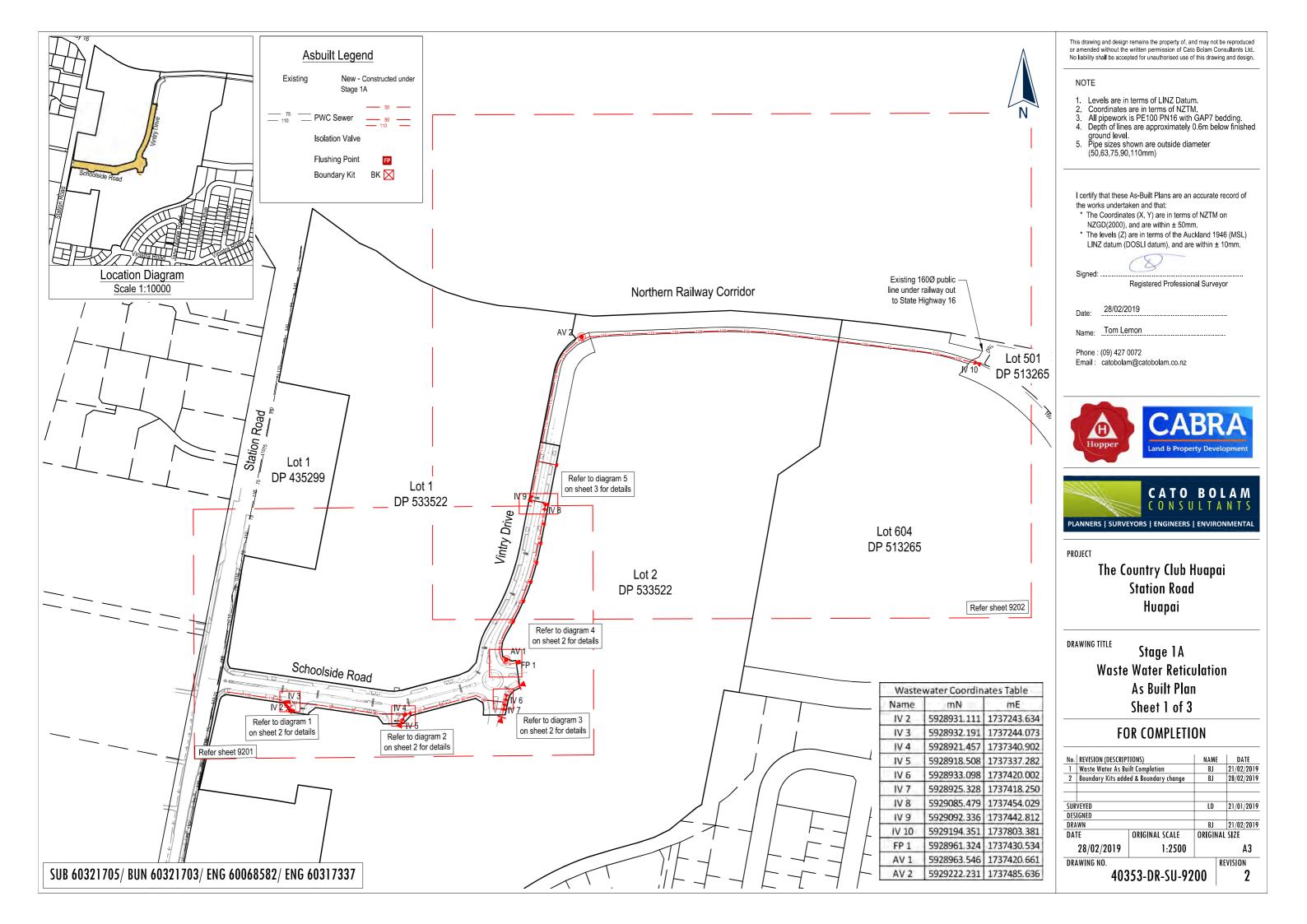
Stage 1A Road As Built Plan Sheet 3 of 4

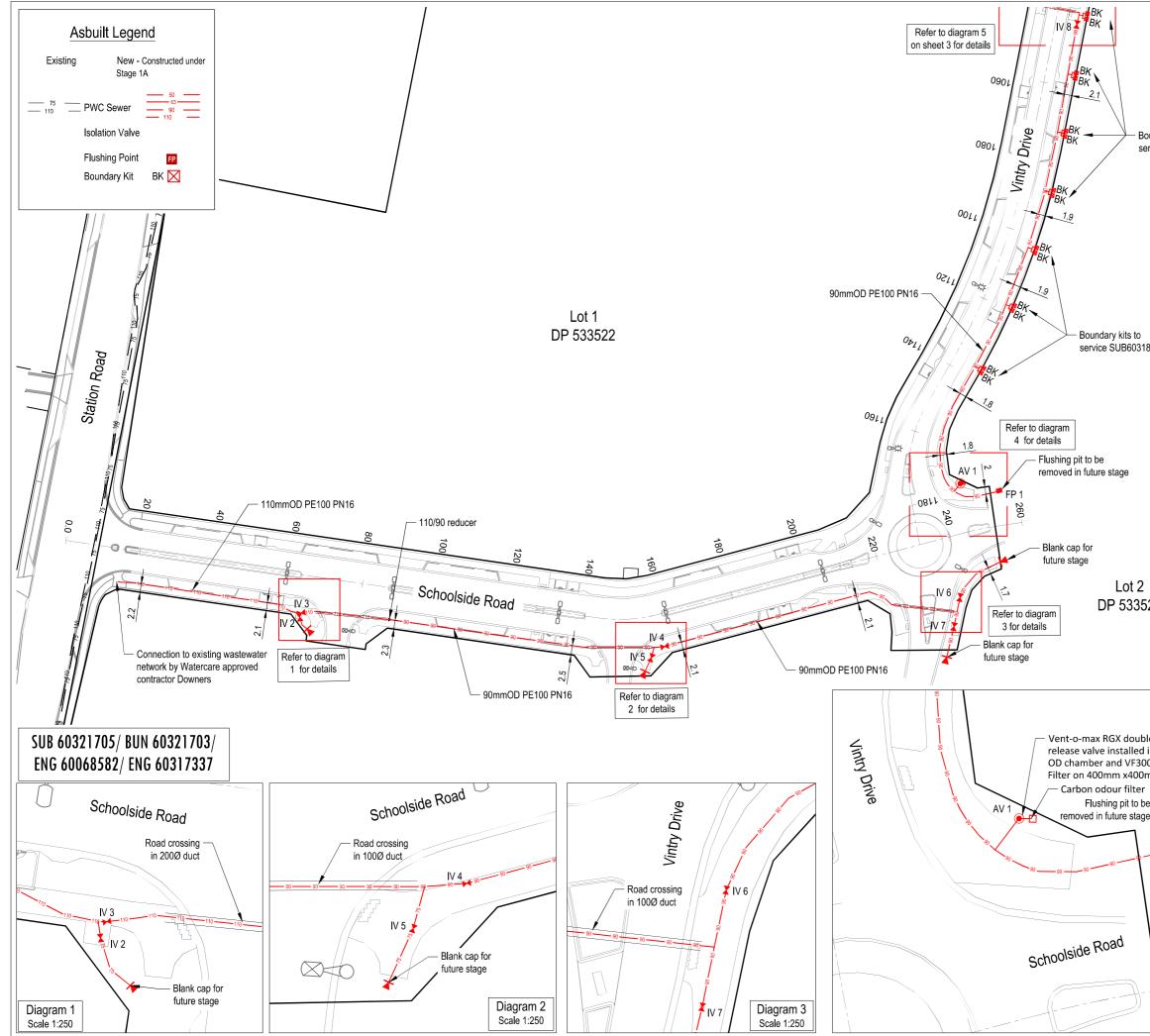
Huapai

FOR COMPLETION

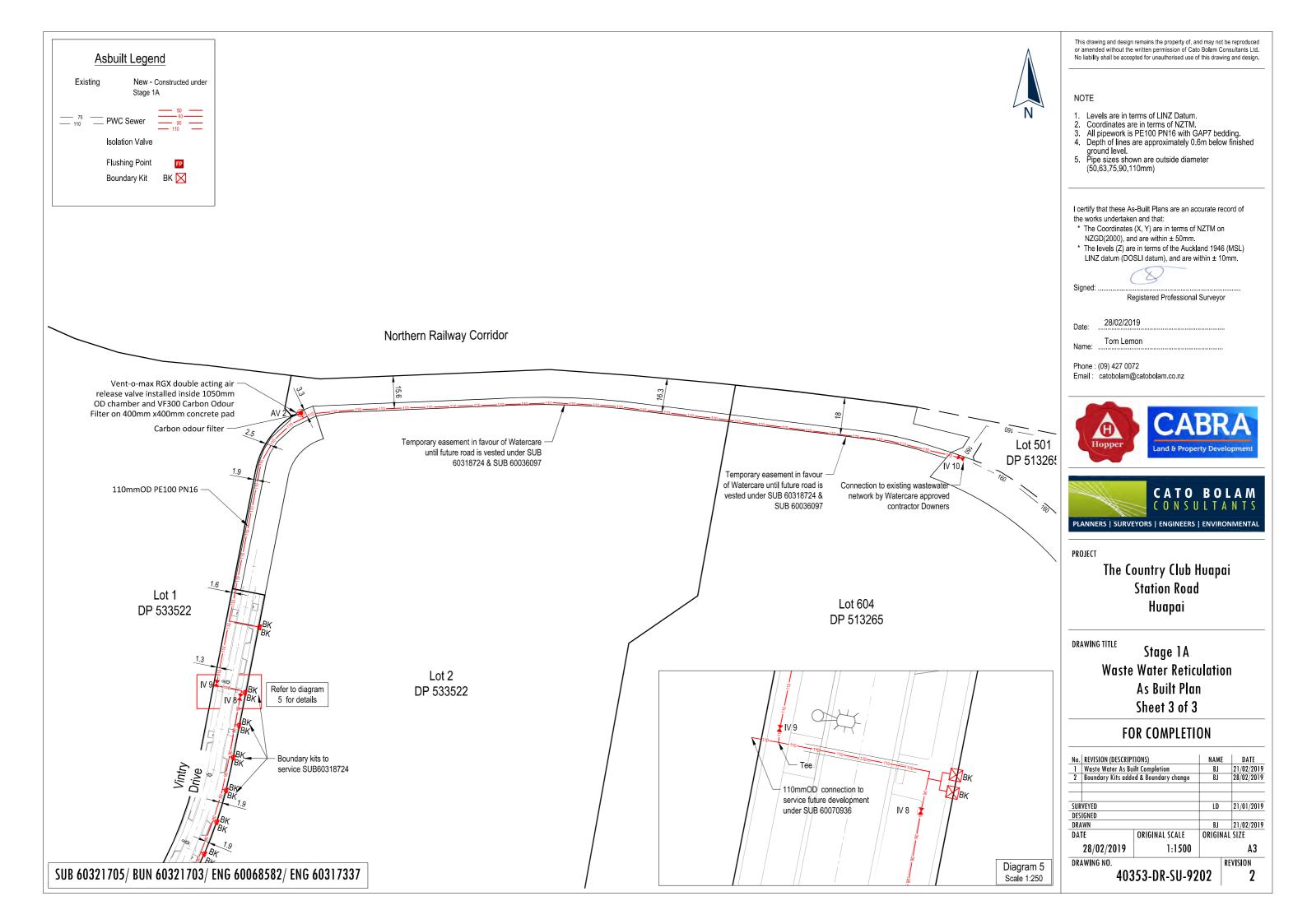
No.	REVISION (DESCRIPTIONS)		NAME		DATE
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2	Boundary change		BJ		28/02/2019
3	Street Trees added		DL		05/11/2019
SUR	VEYED		LD		21/01/2019
DES	IGNED				
DRA	WN		BJ		21/02/2019
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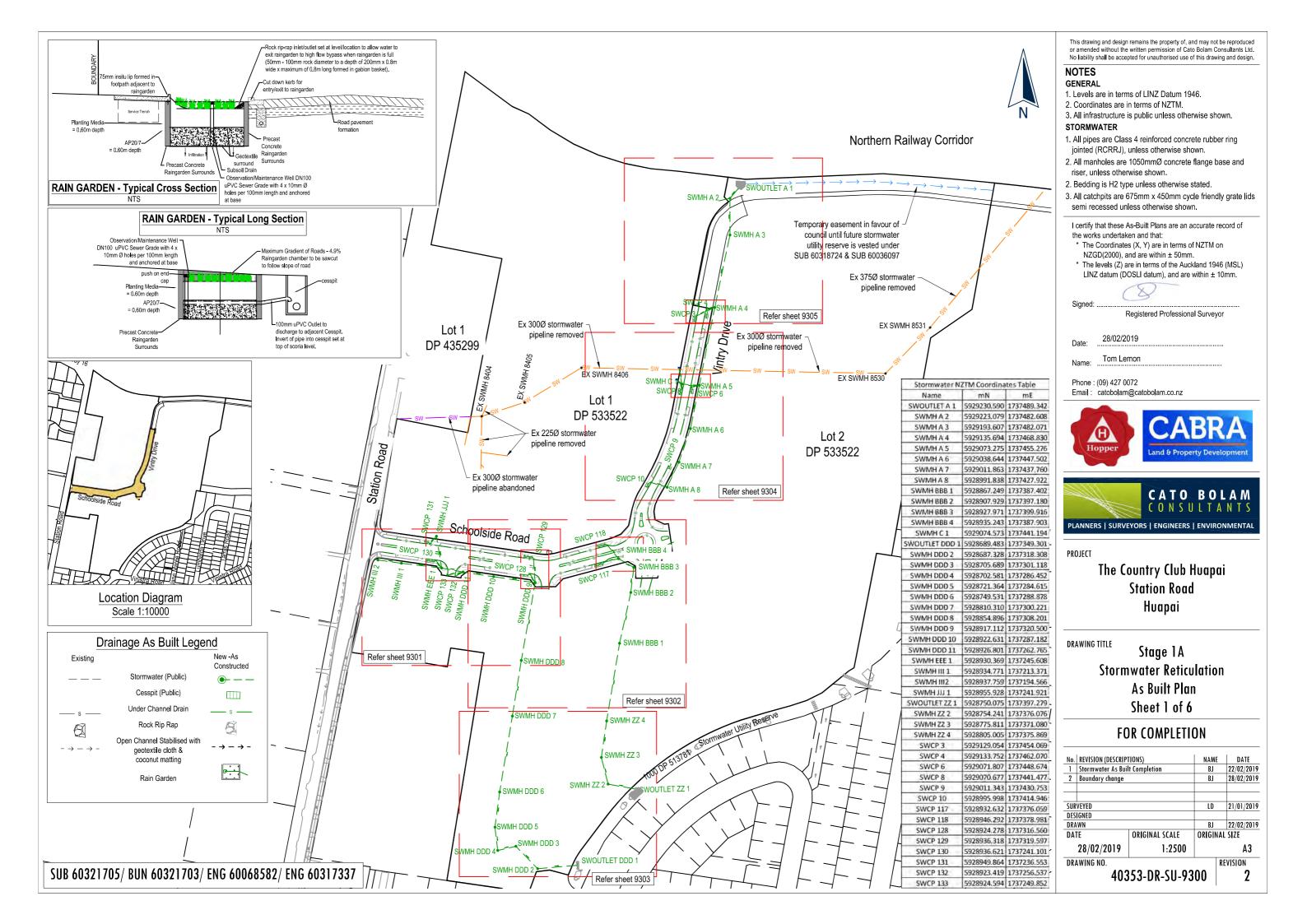


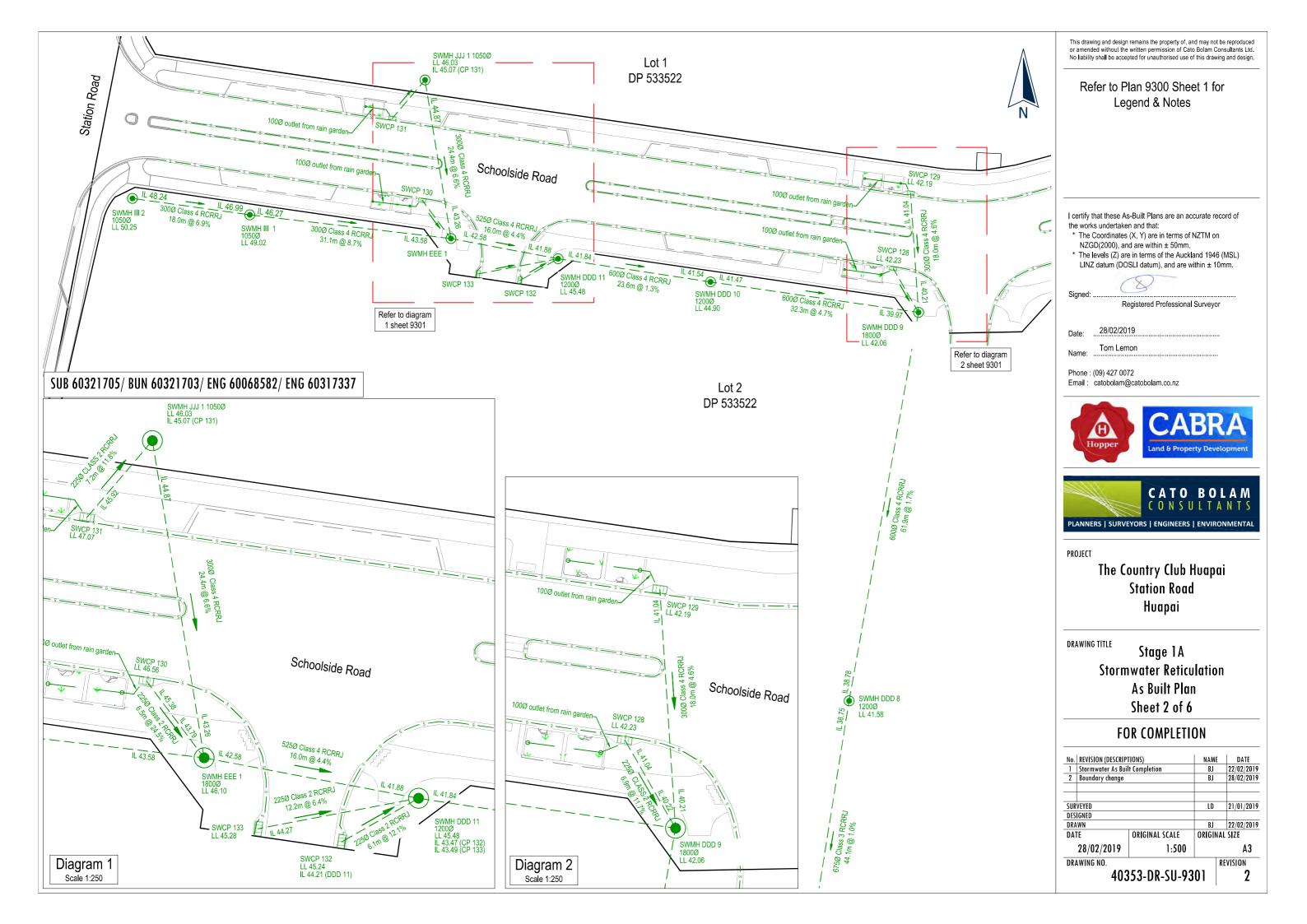


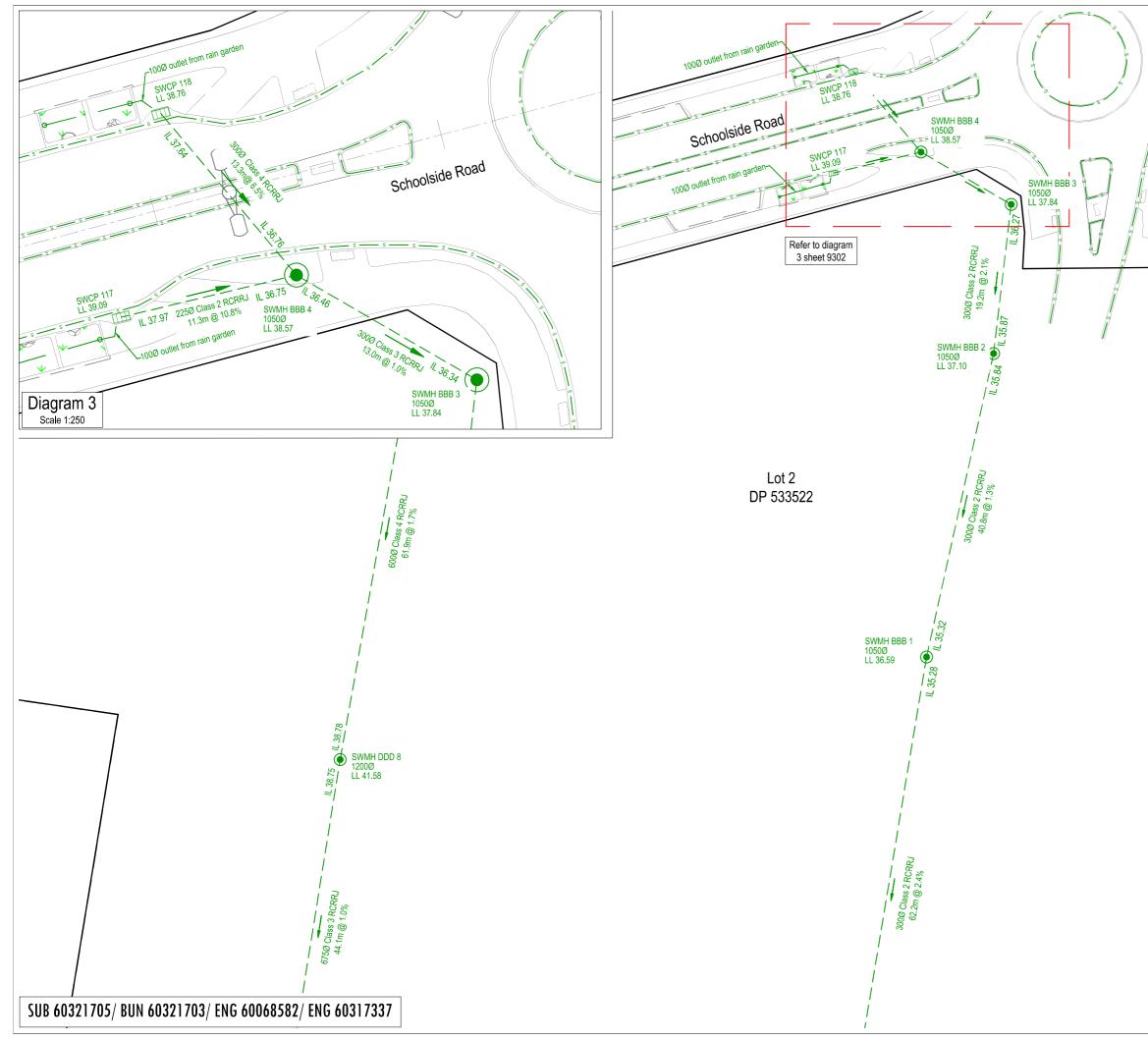


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pundary kits to ervice SUB60318724	 NOTE Levels are in terms of LINZ Datum. Coordinates are in terms of NZTM. All pipework is PE100 PN16 with GAP7 bedding. Depth of lines are approximately 0.6m below finished ground level. Pipe sizes shown are outside diameter (50,63,75,90,110mm)
	I certify that these As-Built Plans are an accurate record of the works undertaken and that: * The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within ± 50mm. * The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm. Signed: Registered Professional Surveyor
8724	Date: 28/02/2019
	Name: Tom Lemon
	Phone : (09) 427 0072 Email : catobolam@catobolam.co.nz
22	CARBRAG Land & Property Development CATO BOLAM CONSULTANTS PLANNERS SURVEYORS ENGINEERS ENVIRONMENTAL
	PROJECT The Country Club Huapai Station Road Huapai
le acting air inside 1050mm 0 Carbon Odour mm concrete pad e e	DRAWING TITLE Stage 1A Waste Water Reticulation As Built Plan Sheet 2 of 3
FP 1	FOR COMPLETION
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Diagram 4 Scale 1:250	drawing no. 40353-DR-SU-9201 2

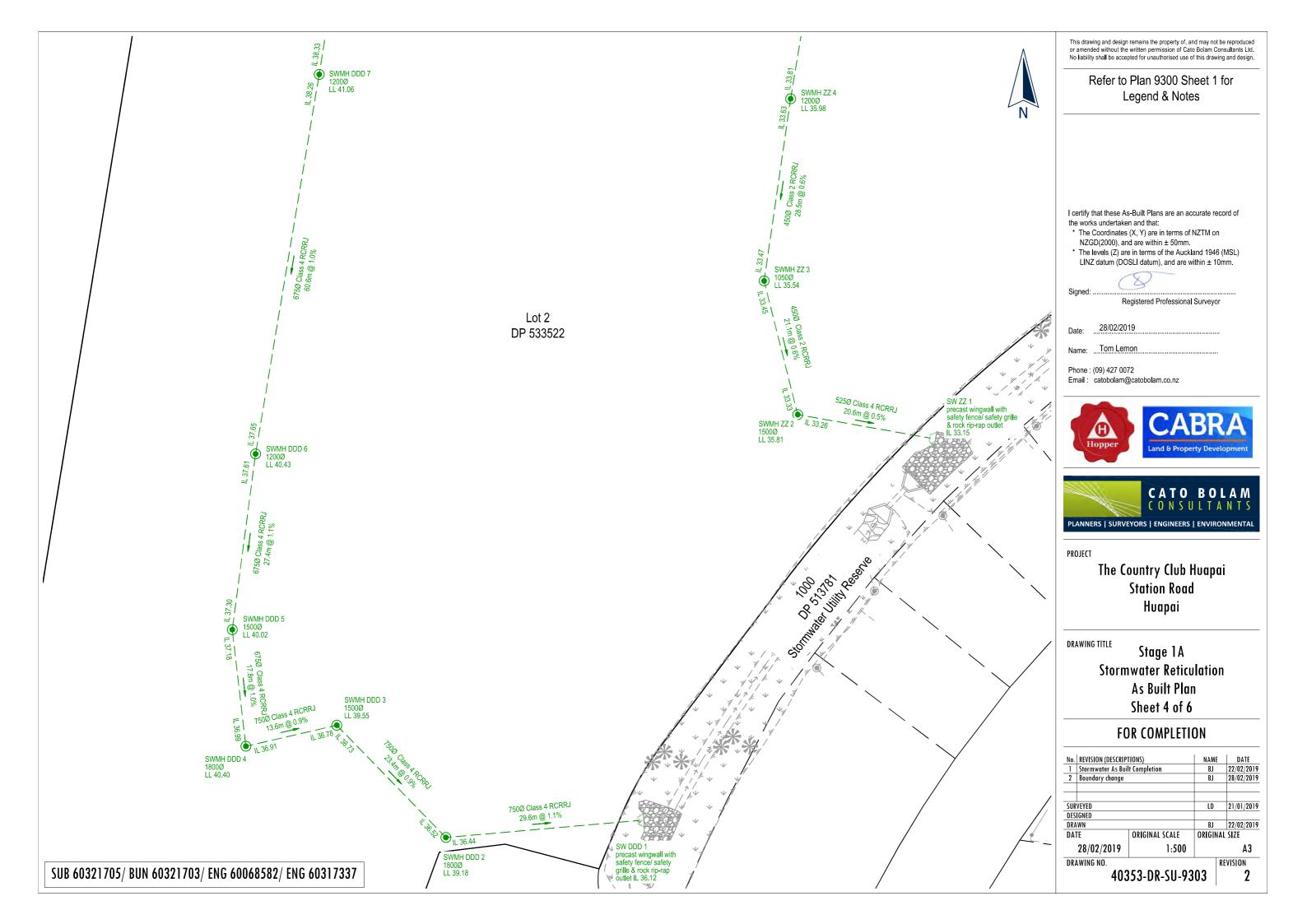


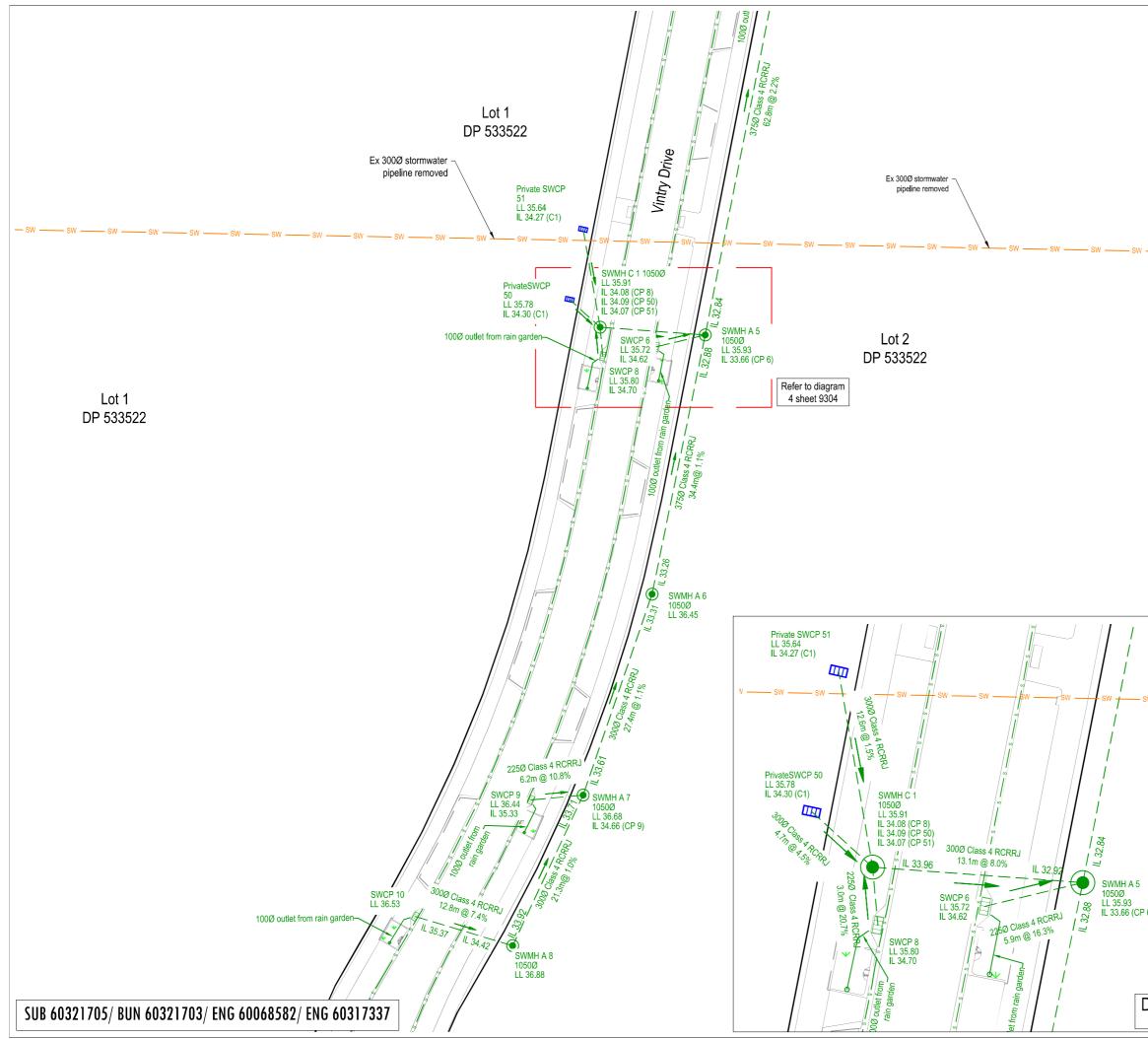




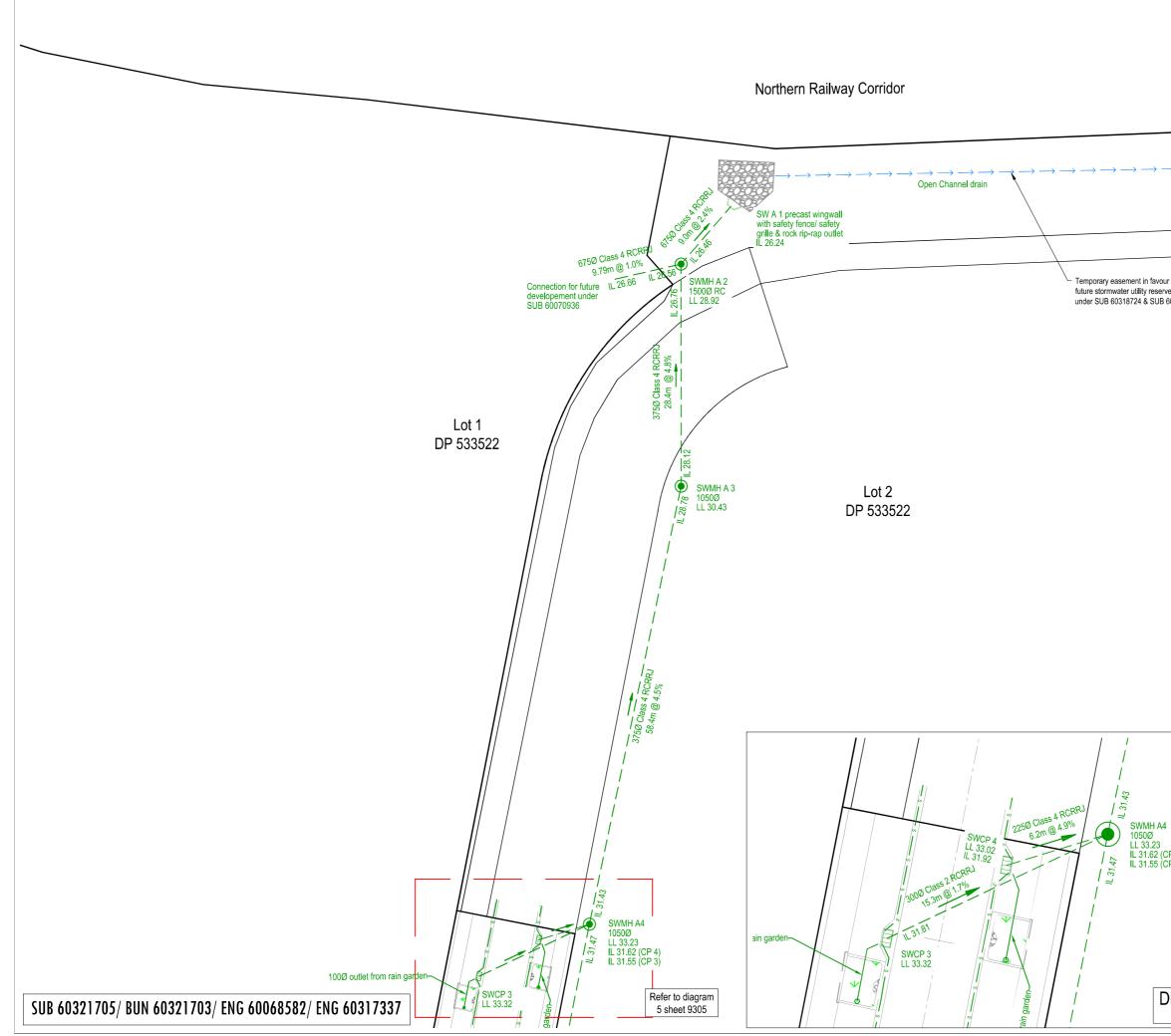


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Refer to	Plan 9300 She	eet 1 fo	r
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I certify that these As	s-Built Plans are an ac	curate reco	rd of
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	8		
Signed:Ri	egistered Professional	Surveyor	
Date:			
Name:	non		
Phone : (09) 427 00 Email : catobolam@			
		-	1.11
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PROJECT The C DRAWING TITLE Storm FC <u>No. REVISION (DESCRIP</u> <u>1 Stormwater As Bui</u> <u>2 Boundary change</u> <u>SURVEYED</u> DESIGNED	CATO CONSU EVORS ENGINEERS OUNTRY Club I Station Road Huapai Stage 1A Water Reticu As Built Plan Sheet 3 of 6 DR COMPLETI	BOL JLTA JENVIRON HUapai J J J J J J A A B J B J B J L D	DATE 22/02/2019 21/01/2019 22/02/2019

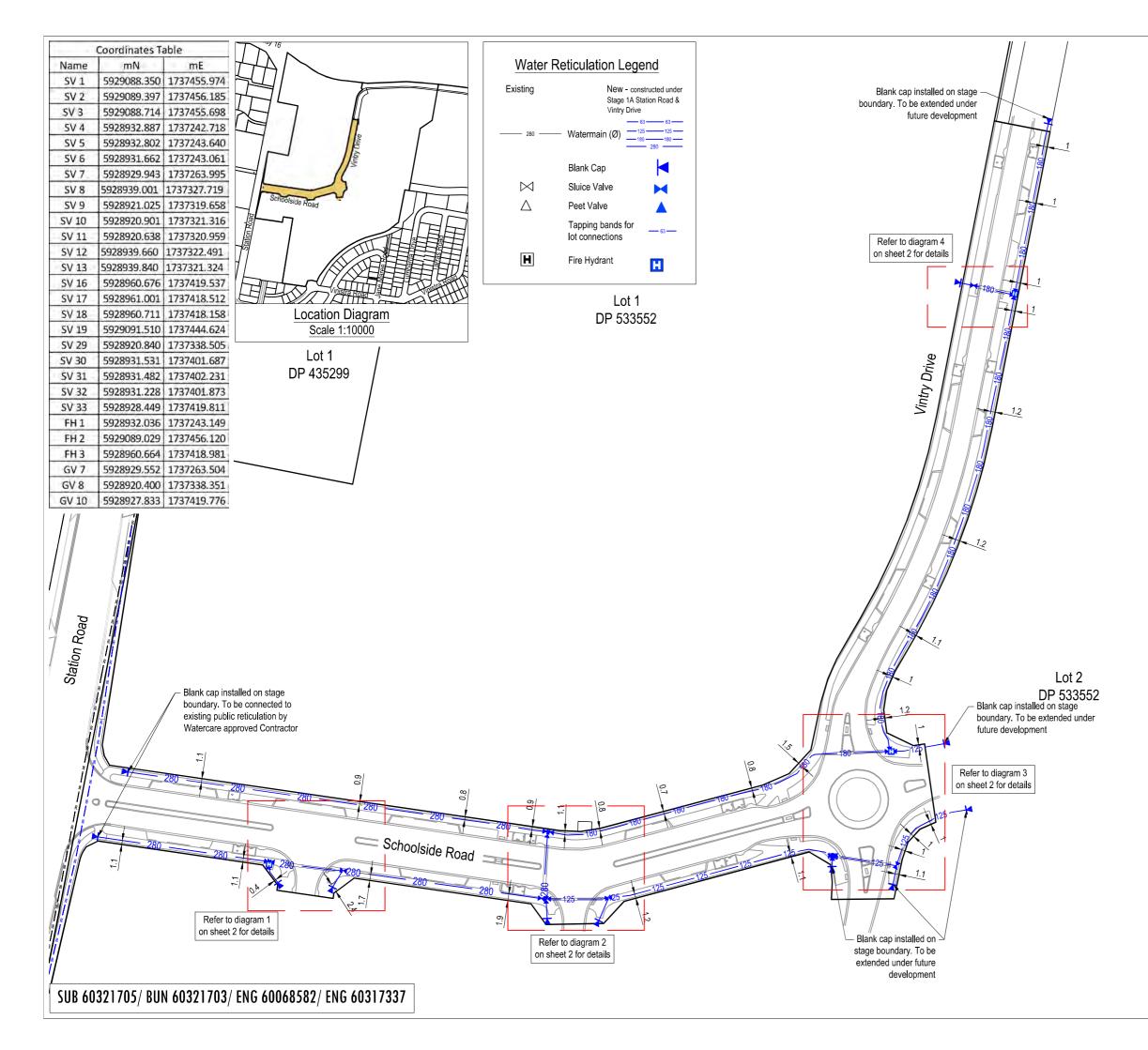




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SW SN	I certify that these As-Built Plans are an accurate record of the works undertaken and that: * The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within ± 50mm. * The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm. Signed: Registered Professional Surveyor Date: 28/02/2019 Tom Lemon Name: Phone : (09) 427 0072
	Email : catobolam@catobolam.co.nz
5W SW	PLANNERS SURVEYORS ENGINEERS ENVIRONMENTAL PROJECT The Country Club Huapai Station Road Huapai
	DRAWING TITLE Stormwater Reticulation As Built Plan Sheet 5 of 6
6)	FOR COMPLETION No. REVISION (DESCRIPTIONS) NAME DATE 1 Stormwater As Built Completion BJ 22/02/2019 2 Boundary change BJ 28/02/2019 3 Built Completion BJ 28/02/2019 3 Built Completion BJ 28/02/2019 Built Completion BJ 22/02/2019 Built Completion BJ 22/02/2019 DESIGNED DRAWN BJ 22/02/2019 DATE ORIGINAL SCALE ORIGINAL SIZE 28/02/2019 1:500 A3
Diagram 4 Scale 1:250	DRAWING NO. 40353-DR-SU-9304 2



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	Refer to Plan 9300 Sheet 1 for Legend & Notes
N	Legend & Notes
$\rightarrow \rightarrow \rightarrow \rightarrow$	
	 I certify that these As-Built Plans are an accurate record of the works undertaken and that: * The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within ± 50mm. * The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.
our of council until rve is vested 3 60036097	Signed:
	Date:
	Phone : (09) 427 0072 Email : catobolam@catobolam.co.nz
	Hopper CABRA Land & Property Development
	CATO BOLAM CONSULTANTS PLANNERS SURVEYORS ENGINEERS ENVIRONMENTAL
	PROJECT The Country Club Huapai
	Station Road Huapai
	DRAWING TITLE Stage 1A
	Stormwater Reticulation As Built Plan Sheet 6 of 6
4	FOR COMPLETION
(CP 4) (CP 3)	No. REVISION (DESCRIPTIONS) NAME DATE 1 Stormwater As Built Completion BJ 22/02/2019 2 Boundary change BJ 28/02/2019
	SURVEYED LD 21/01/2019 DESIGNED DRAWN BJ 22/02/2019
Diagram 5	DATE ORIGINAL SCALE ORIGINAL SIZE 28/02/2019 1:500 A3 DRAWING NO. REVISION
Scale 1:250	40353-DR-SU-9305 2





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NOTES

GENERAL

1. Coordinates are in terms of NZTM 2000.

WATER RETICULATION

- Cover for water reticulation.
- Mains under grass berms and footpaths 600mm
- b) Mains under road carriageway 900mm
 2. Watermain 125, 180 & 280 OD PE100 PN12.5
- 3. All ridermains 63 OD - PE80 PN12.5 4. Stainless Steel bolts and nuts used for flanged

connections. 5. Metallic Detector Tape provided above all watermains and ridermains

I certify that these As-Built Plans are an accurate record of the works undertaken and that:

- $^{\ast}\,$ The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within ± 50mm.
- * The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.

Signed: Registered Professional Surveyor

28/02/2019 Date Tom Lemon Name

Phone : (09) 427 0072 Email: catobolam@catobolam.co.nz

CABR and & Property Deve

CATO BOLAM CONSULTANT PLANNERS | SURVEYORS | ENGINEERS | ENVIRONMENTAL

PROJECT

The Country Club Huapai Station Road Huapai

DRAWING TITLE

Stage 1A Water Reticulation As Built Plan Sheet 1 of 2

FOR COMPLETION

drawing no. 40353-DR-SU-9400			REVISION 7	
	28/02/2019	1:1000		A3
DA	TE	ORIGINAL SCALE	ORIGINAL SIZE	
DR/	WN	-	BJ	21/02/2019
DESIGNED				
SUR	VEYED		LD	21/01/2019
2	Boundary change		BJ	28/02/2019
1	Water As Built Completion		BJ	21/02/2019
No.	REVISION (DESCRIPTIONS)		NAM	E DATE

